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# A Study of Dredging Effects in Hampton Roads, Virginia

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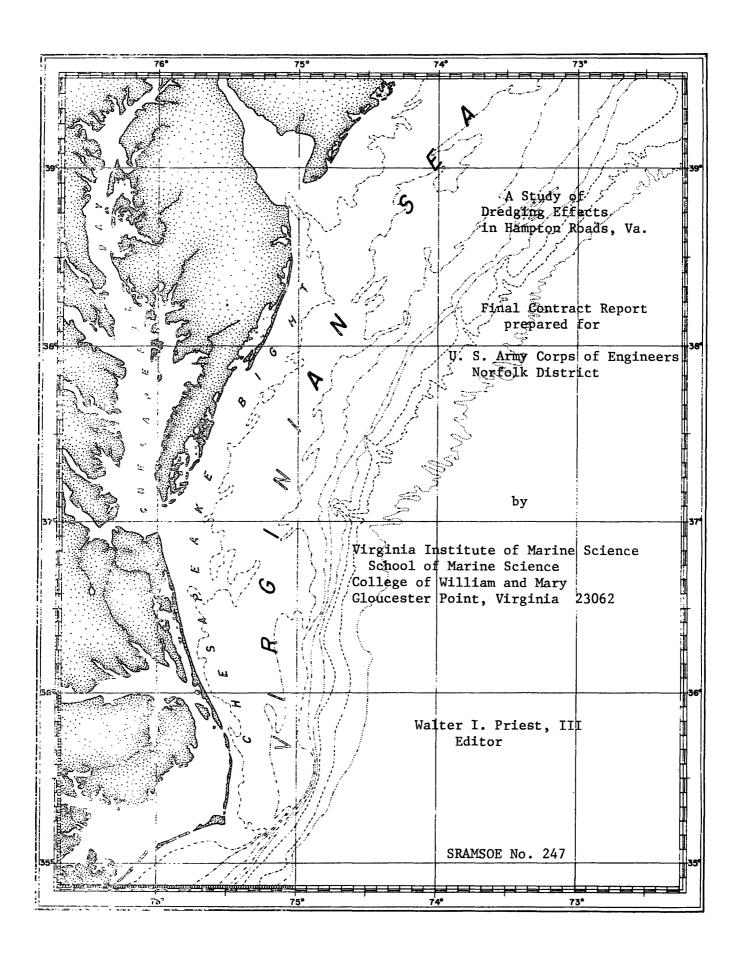


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### A STUDY OF DREDGING EFFECTS IN HAMPTON ROADS, VIRGINIA

Final Contract Report

prepared for:

U. S. Army Corps of Engineers Norfolk District 803 Front Street Norfolk, Virginia 23510

under Contract No. DACW65-78-C-0029

bу

Virginia Institute of Marine Science School of Marine Science College of William and Mary Gloucester Point, Virginia 23062

Walter I. Priest, III, Editor
March, 1981

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### Introduction

The environmental consequences of dredging and spoil disposal are among the most extensively studied of all of the impacts associated with construction activities performed in aquatic ecosystems. Because the dredged material must be disposed of, the operations are often considered synonymous. This can present problems when assessing the environmental impacts of a project because the majority of adverse impacts are associated with the disposal operations in open-water rather than the dredging per se. This synonymy is unfortunate when the dredged material is being placed in a confined upland site whereby a major portion of the adverse impacts to the environment are being eliminated or greatly reduced.

The intent of this report is to identify and quantify, in part, the adverse effects associated with the dredging operation itself and those segments of the ecological community which might be adversely affected by the levels of suspended solids and sedimentation attributable to the dredge. It consists of three sections including: a comprehensive review of the major marine resources, their location in and utilization of the Hampton Roads Harbor and vicinity; the turbidity model and physical environment, describing the levels and distribution of suspended sediment and sedimentation and local current patterns; and a review of the effects of increased suspended sediment loads on estuarine organisms and water quality.

The first section on marine resources contains chapters of finfish, shellfish and ichthyoplankton. The finfish report summarizes the results of comprehensive trawl surveys performed during 1978 and 1979. These data were analyzed for the seasonal distribution of both resident and migratory species and nursery areas utilized by juveniles.

The shellfish report details the distribution of the oyster, <u>Crassostrea virginica</u>, and the hard clam, <u>Mercenaria mercenaria</u>, in Hampton Roads and the lower James River. The oyster data are based on the different densities of oysters associated with three types of substrate, oyster rock, mud and shell and sand and shell, which represent the areas where oyster populations are densest. Also included are data on oyster spatfall for the years 1976-1979 at selected stations in the study area. The hard clam data depict their distribution and abundance in the Hampton Roads area.

The ichthyoplankton chapter reports the seasonal distribution of fish eggs and larvae in and near the study area based on recent research.

The data from the lower Chesapeake Bay can be extrapolated to a limited extent to include Hampton Roads and that from the Southern Branch of the Elizabeth River is directly applicable to other Hampton Roads tributaries.

The first two chapters of the second section of this report describe the model and the field calibration experiments developed to predict the distribution of dredge-induced suspended solids and sedimentation and the various facets of the dredging operation which influence their generation and distribution. Also included in this section are detailed descriptions of the surface and near bottom currents in the study area which also affect the distribution of the suspended solids.

The final section of this report presents a review of the literature concerning the environmental impacts of increased suspended solids levels created by dredging. These impacts include: increased turbidity levels, changes in dissolved oxygen, sedimentation and their effects on various estuarine organisms.

This report is intended to provide an effective scheme for the evaluation of the impacts of dredging in the Hampton Roads area. By providing detailed quantified distributional data on the important resources

of the area, an accurate means of predicting the distribution of increased suspended solids levels and a means of approximating which organisms are going to be affected by the predicted increase, it is hoped that well informed decisions can be made regarding dredging activities in Hampton Roads.

MARINE RESOURCE DESCRIPTIONS

Nekton Utilization of Aquatic
Resources in the Elizabeth River
and the Lower James River

by

Marion Y. Hedgepeth, John V. Merriner and Frank Wojcik

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March, 1981

Nekton Utilization of Aquatic Resources in the Elizabeth River and the Lower James River

by

Marion Y. Hedgepeth, John V. Merriner and Frank Wojcik

#### Introduction

The Chesapeake Bay and its tributaries provide the state of Virginia with some if its' greatest natural resources. Our blue crab, oyster and finfish industries are three of the largest commercial fisheries on the east coast of the United States.

Although a major portion of one of Virginia's largest tributary systems (the James River) has been closed to most shellfishing and finfishing since 1976 due to Kepone contamination, it still provides seasonal and permanent residence for large populations of shellfish and finfish. The lower James River area (Hampton Roads) and the Elizabeth River provide an estuarine habitat for many commercially and recreationally important species. For example, the Elizabeth River and the lower James River are important nursery grounds for spot, Atlantic croaker, Atlantic menhaden, weakfish, striped bass, black seabass, and summer flounder. Furthermore, they are important as feeding grounds for adult bluefish, weakfish, spot, and Atlantic croaker. Anadromous species such as striped bass, American shad, blueback herring and alewife travel through these areas to reach their freshwater spawning grounds.

The purpose of this study was to investigate nekton utilization of the Elizabeth River and the lower James River and to establish specific uses. Subsequently, this information would be used by the Army Corps of Engineers for scheduling dredging projects at times and locations for least impact on the nekton community.

Studies by Virginia Institute of Marine Science (VIMS, Musick et al., 1972 and Rooney-Char, and Ayres, 1978), and the U.S. Army Corps of Engineers (1977) addressed several problems associated with dredging operations and pipeline landfall sites in our present study areas. They concluded that the two major impacts would be the removal of benthic organisms which serve as fish food and the resuspension of sediments. The latter would affect fish by increasing turbidity, altering respiration rates and predator-prey behavior, and by resuspending heavy metals or other toxic substances present. In a report on water quality in the Elizabeth River, Nielson et al. (1978) cited high levels of heavy metals in bottom sediments and high levels of fecal coliforms in water samples. suggest that environmental impacts in the Hampton Roads and Elizabeth River must be examined in detail before dredging permits are issued.

# Study Area and Methods

The areas included in the nekton resource survey were the eastern, southern, and western branches of the Elizabeth River and the lower James River from the Hampton Roads Bridge Tunnel to the James River Bridge (approximately mile ten).

Bottom trawl surveys utilizing lined 16-foot (5-meter) semiballoon trawls were conducted on the Elizabeth River during 1978 and 1979. During the month of August, 1978, 22 random stations (Fig. 1) were made in the southern branch of the Elizabeth River. A 42-foot (13-meter) commercial boat, The Three Daughters, was sub-contracted for this survey. In all subsequent Elizabeth River surveys the R/V Restless, a 32-foot (10-meter) vessel, was used. During March, 1978, three fixed stations (Fig. 1) were made in the southern branch. These stations were approximately located at the upper, middle and lower portions of the river. Again, in February, 1979, 22 random stations were made in the southern branch, while 9 fixed stations (three in each branch) were made in August, 1979, (Fig. 2).

Thirty-foot, (9-meter), lined semi-balloon trawls were used on the surveys of the lower James River. Thirty random stations (Fig. 3) were made in this area during February, 1978 from the R/V Langley, an 80-foot (24-meter) steel ferryboat. Trawl data from July, 1978 (consisting of 34 random stations, Fig. 3) and January, 1979 (consisting of 30 random stations, Fig. 4) were taken in conjunction with a Kepone Biomass Study

of the James River. Trawl data (consisting of 2 stations) from July, 1979 were taken during a VIMS Crustaceology-Ichthyology Monitoring Survey conducted with the R/V <u>Pathfinder</u>, a 55-foot (17-meter) vessel.

After each five minute tow, fish were identified, counted and weighed by species. Whenever possible, 50 fish of a species were measured for total length in millimeters. Blue crabs were counted, and scored (tallied) by sex and stage of development.

Water quality observations were obtained from surface and bottom readings of dissolved oxygen (mg/l), salinity (ppt.) and temperature (°C). Secchi disk readings (in meters) were used to describe water clarity.

#### RESULTS

# Fish Distributions in the Elizabeth River

During the 1978 Winter Survey, only two fish were captured (a hogchoker and a juvenile blueback herring); therefore, no table was prepared. Water temperatures ranged from 2-7°C.

Many species which overwinter in the rivers probably migrated just outside of the mouth of the Chesapeake Bay or offshore.

The 1979 Winter Survey yielded 18 species and a total of 657 fish, (Table 1). The most abundant species were juvenile spot, Atlantic croaker, blueback herring and alewife. Spot, striped bass, American eel, hogchokers and river herring accounted for 90 percent of the total biomass.

Juvenile spot and striped bass were only collected upstream of Mains Creek, (Figs. 5 and 6). Spot ranged in total length from 73-151 millimeters, while striped bass ranged in total length from 117-197 millimeters. Water temperatures below Mains Creek were 8-9°C, while those around Craney Island were 4.3-5.3°C.

Atlantic croaker were collected throughout the river,

(Fig. 7). Most of these fish were less than 50 millimeters

in total length. Winter kills of Atlantic croaker were noted

in trawls made near New Mill Creek, Town Point and upriver from

Jones Creek.

Alosines (blueback herring, alewife and American shad) were also collected throughout the river, (Fig. 8). Blueback herring dominated most of the catch of alosines; however, at

Milldam Creek, alewife constituted 99 percent of the catch.

Alosines varied in length from 46-170 millimeters.

Summer surveys usually provided more species, more individuals and larger fish. Seventeen species and 3,912 fish were collected in August 1978 from the southern branch. Bay anchovy, spot and weakfish were the most abundant species, (Table 2). Biomass mainly consisted of spot, hogchoker, Atlantic croaker, summer flounder and weakfish. Water temperatures between 26.9 and 32°C were recorded. In the summer of 1979, only nine species were collected from each branch. Again, spot and Atlantic croaker were the dominant species, (Table 3).

Spot were more abundant at stations upstream of Milldam Creek in the southern branch, (Fig. 9), and upriver in the eastern and western branches. Adults as well as juveniles were collected in the waters around Craney Island. Adult summer flounder were also quite abundant near Craney Island.

Atlantic croaker were more abundant at stations in the western and eastern branches, (Fig. 10). Juveniles (22-137 millimeters in total length) were found at stations below Jones Creek on the southern branch while adults (215-355 millimeters in total length) were found near the mouth of the river.

Adult and very small juvenile (18-23 millimeters in total length) weakfish were collected from the mouth of the river to Town Point (Fig. 11). Larger juveniles were collected at upriver stations where temperatures were warmer and salinities were slightly less saline.

# Fish Distributions in the Lower James River

Fifteen species and a total of 349 fish were collected in the lower James River during the 1978 Winter Survey. Blueback herring and Atlantic silversides were the dominant species (Table 4). The winter survey of 1979 yielded twenty-three species and a total of 16,405 fish were collected. Atlantic croaker was by far the most abundant species followed by bay anchovy, Atlantic silversides and blueback herring. During the 1978 Winter Survey, water temperatures ranged from 1.0-2.1°C while water temperatures during the 1979 Winter Survey ranged from 5.0-6.0°C.

Atlantic croaker ranged in total length from 32-115 millimeters. Atlantic croaker and spot appeared to be more abundant in waters with depths greater than 13 meters (40 feet). On the otherhand, bay anchovy, Atlantic silversides, blueback herring and Atlantic menhaden appeared to prefer waters with depths of less than 6 meters (20 feet). Furthermore, the Atlantic croaker, herring, and shad appeared to be more abundant on the Norfolk-side of the river, (Figs. 12 and 13).

The 1978 Summer Survey yielded 18 species and a total of 2,470 fish. Striped and bay anchovies were the most abundant species followed by spot, weakfish and hogchokers, (Table 4). In the 1979 Summer Survey, 16 species and a total of 989 fish were collected (Table 5). Bay anchovy was the dominant species, although, weakfish and several other species contributed larger amounts to the total biomass. Water temperatures ranged

between 24-28°C in 1978 and between 21-23°C in 1979. The distributions of important species of these surveys were not plotted due to insufficient data.

### DISCUSSION AND CONCLUSIONS

The seasonal distributions of finfishes were important in considering specific uses of the study areas; however, much of the discussion was limited to demersal fish (Table 6). Since only bottom trawls were utilized, the distributions and abundances of fishes such as gobies, blennies, killifish, and other finfish species of the beach zone communities and tidal creeks were not examined. Also, data were not available for large predator species such as bluefish which avoid the net.

The location and time of spawning were important in considering the distribution of fishes. Spot spawn at sea during late fall to early spring, while Atlantic croaker spawn at sea during late summer to early winter. Therefore, juvenile Atlantic croaker are found earlier in the Chesapeake Bay than spot. Weakfish spawn during the months of May, June and July at the entrance to the Chesapeake Bay. Later, young-of-the-year migrate into the Chesapeake Bay and its tributaries. Young spot, Atlantic croaker and weakfish remain in inshore nursery grounds for a period of a year or more before making their first migration to sea.

Alosines and striped bass migrate through the Chesapeake
Bay and spawn in the freshwater reaches of the Chesapeake Bay's
tributaries. Sexually mature alewife and striped bass enter
the Chesapeake Bay during the month of February followed
approximately four weeks later by blueback herring and

American shad (Hildebrand et al., 1928). Some striped bass are found in the Chesapeake Bay and its tributaries all year. Most young alosines leave the Chesapeake Bay upon the approach of cold weather; therefore populations of these species that remain to overwinter are small.

Small forage fish species such as bay anchovy, Atlantic silverside and naked goby which are permanent residents of the study areas spawn generally during the spring. Merriner et al. (1979) captured bay anchovy eggs, larvae and post-larvae from late spring through early fall in ichthyoplankton samples taken around Hog Island on the James River. Naked goby larvae and post-larvae were captured from May through October, while silverside eggs, larvae and juveniles were captured throughout the spring and summer.

In the U.S. Army Engineering Study (1977), it was suggested that the Elizabeth River was utilized as a nursery ground by Atlantic menhaden, spot and Atlantic croaker. In our study, winter distributions of spot and striped bass indicated that the upper reaches of the southern branch of the Elizabeth River serve as an overwintering ground and/or nursery ground for juveniles of these species. Juvenile Atlantic croaker and alosines were captured evenly throughout the Elizabeth and lower James River. Therefore, these species utilized both river systems as an overwintering-nursery ground. Juvenile Atlantic menhaden and small forage fish species such

as bay anchovy and Atlantic silverside preferred the waters of the lower James River as an overwintering nursery ground.

Permanent residents of both study areas included bay anchovy, Atlantic silverside, skilletfish, oyster toadfish, blackcheek tonguefish, and hogchoker. White perch and yellow perch were only captured in the Great Bridge area of the southern branch of the Elizabeth River. Other finfish species that were captured were considered as incidental species; because, only a few individuals of these species were captured in trawls during a survey.

During summer, the Elizabeth River and lower James River continued to be utilized as nursery grounds for juvenile spot, Atlantic croaker and weakfish. Juvenile spot preferred the upper reaches of these tributaries. In fact, during mid-summer juvenile spot were found as far up the James River as Hopewell, Virginia (approximately river mile 65). Adult spot, Atlantic croaker and weakfish preferred the Chesapeake Bay and the lower portions of its tributaries. The Craney Island-Lamberts Point area was a popular feeding area for adult spot, Atlantic croaker and summer flounder. They were rarely captured beyond this area on the Elizabeth River.

Temperature was the major factor in the winter distribution of fishes, while the availability of food was the major factor in the summer distribution of fishes. Principal finfish uses of the Elizabeth River and lower James River areas were (1) the nursery grounds for juvenile spot, Atlantic croaker,

alewife, blueback herring, American shad, striped bass and weakfish; (2) the adult feeding grounds for spot, Atlantic croaker, weakfish, summer flounder, etc. and (3) the spawning grounds for important forage species such as bay anchovy and Atlantic silverside. Only minor occurrences of striped bass and alosine spawning were observed in the upper reaches of the Elizabeth River.

Dredging operations in the study areas will have a greater affect on the juvenile fishes of the nursery ground and forage fishes, than on the adult fishes of the summer feeding grounds. Adult fishes are normally more efficient in their daily search for food, and are less subject to capture by prey species than juvenile fishes. Consequently, adult individuals will have a greater chance of finding other food resources beyond the area of a dredging project. The impact of dredging operations would be critical during winter and spring when water temperature and food availability restrict the distribution of permanent residents and fishes of the nursery ground and during summer and fall when many larval and juvenile fishes are abundant in the study areas. Winter dredging projects may increase the frequency of winter fish kills by forcing fish to migrate into colder waters. During spring, several finfish species such as bay anchovy and Atlantic silverside spawn in the study areas. Eggs and larvae of these species may be affected by dredging operations.

Other environmental factors to consider in scheduling dredge operations would be those mentioned in the Portsmouth

Refinery Study (1977). They include: the removal of benthic organisms (prey for fishes); respiratory problems; and the uptake of heavy metal and/or other toxic substances. The effect of these factors on fishes would be best observed during an actual dredging operation.

### **ACKNOWLEDGEMENTS**

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### LITERATURE CITED

- Hildebrand, S. F. and W. C. Schroeder. 1928. <u>Fishes of</u>

  <u>Chesapeake Bay</u>. Fish Bull. 52. T. F. H. Publications Inc.

  (1972 reprint) Neptune, New Jersey.
- Merriner, J. V., A. D. Estes and R. K. Dias. 1979.

  Ichthyoplankton Entrainment Studies at VEPCO Nuclear

  Power Station, 1978 Final Report. VIMS, Gloucester

  Point, Virginia.
- Musick, J. A. et al. 1972. VIMS 3-C Report, (Draft) Fish

  Section. VIMS, Gloucester Point, Virginia (unpublished).
- Nielson, B. J. and S. C. Sturm. 1978. Elizabeth River Water

  Quality Report VIMS Special Report in Applied Marine

  Science and Ocean Engineering No. 134, Gloucester Point,

  Virginia.
- Rooney-Char, Ann and R. P. Ayres. 1978. Offshore pipeline corridors and Landfalls in Coastal Virginia. VIMS

  Special Report in Applied Marine Science and Ocean

  Engineering. No. 190.
- United States Army Engineering District, Norfolk, Virginia.

  1977. Final Environmental Impact Statement, Hampton
  Roads Energy Company's Portsmouth Refinery and Terminal,
  Portsmouth, Virginia.

Table 1. Elizabeth River Winter Trawl Survey 1979 (22 stations in the southern branch).

Species	Total Number	Total Weight (grams)
American eel	6	850
Blueback	6 4	252
Alewife	79	634
American shad	5	86
Atlantic menhaden	12	66
Bay anchovy	37	35
Banded killifish	1	1
Striped killifish	1	5
Atlantic silverside	66	235
White perch	2	53
Striped bass	3 7	1,830
Yellow perch	1	6
Spot	178	2,572
Atlantic croaker	99	57
White mullet	3	204
Naked goby	1	1
Blackcheek tonguefish	5	24
Hogchoker	_60	806
	<u>657</u>	7,717
Blue Crabs Male Female - (mature) Female - (immature)	27 (4 soft) 2 25 54	

Table 2. Elizabeth River Summer Trawl Survey 1978 (22 stations in the southern branch).

Species	Total Number	Total Weight (grams)
American eel Cusk eel Atlantic menhaden Bay anchovy Oyster toadfish Spotted hake White perch Yellow perch Weakfish Black seabass Spot Atlantic croaker Naked goby Butterfish Northern searobin Summer flounder Hogchoker	9 2 9 1,097 9 8 11 1,860 57 1 2 1 24 386 3,912	822 37 173 919 950 830 414 34 2,072 30 18,822 3,940 0.5 3 5 2,841 6,676 38,568.5
Blue Crabs Male Female - (mature) Female - (immature)	87 15 61 163	

Table 3. Elizabeth River Summer Trawl Survey 1979 (3 stations in each branch).

		stern anch		thern anch		stern anch
Species	* TN	* TW	TN	$\mathbf{TW}$	TN	TW
American eel	1	308	2	150	10	950
Cusk eel	_	-	1	15	_	-
Atlantic menhaden	-	_	-	_	2	47
Gizzard shad	_	-	_	-	1	280
Bay anchovy	47	165	3	10	9	25
Oyster toadfish	1	185	6	410	5	1,020
Weakfish	4	220	17	100	19	277
Spot	431	4,953	160	3,230	175	1,590
Atlantic croaker	97	4,575	63	3 <b>,</b> 778	139	4,045
Summer flounder	4	377	1	220	_	-
Blackcheek tonguefish	1	10		-	-	-
Hogchoker	50	$\frac{1,380}{11,385}$	$\frac{64}{33.7}$	$\frac{1,295}{0.000}$	18	420
	636	11,895	317	9,208	378	8,654
Blue Crabs Male	16	soft)	31		37	
Female - (mature)	_ `-	SOLU	8		7	
Female - (immature)	19		13		16	
Mud crabs			1		_	
	35		<del>5</del> 3		<u>60</u>	

<sup>\*</sup> TN = Total Number

TW = Total Weight (grams)

Table 4. Lower James River Summer (34 stations) and Winter (30 stations) Trawl Surveys 1978.

	Total	Number		Weights ams)
Species	Summer	Winter	Summer	Winter
American eel	1		40	
Blueback herring		150		360
Alewife		7		74
Atlantic menhaden	1		15	
Gizzard shad		3		88
Striped anchovy	981		2,566	
Bay anchovy	581	50	279	39
Inshore lizardfish	2		7	
Oyster toadfish	7	8	205	30
Skilletfish		1		5
Spotted hake	3		230	
Striped cusk eel	18		450	
Atlantic silverside		109		326
Northern pipefish	2	6	2	15
White perch		2		11
Black seabass	5		234	
Weakfish	256		4,891	
Spot	310		7 <b>,</b> 570	
Atlantic croaker	16	1	3,007	1
Tautog		1		285
Striped blenny		5 2		40
Naked goby		2		1
Butterfish	17		77	
Norhern sea robin	4		35	
Summer flounder	39		2,417	
Windowpane flounder	3		160	
Hogchoker	224	3	4,973	150
Blackcheek tonguefish	2,470	$\frac{1}{349}$	27,158	$\frac{1}{1,426}$

Table 5. Lower James River Winter (30 stations) and Summer (July only; 2 stations/4 tows) Trawl Surveys 1979.

Total Number				Total Weight (grams)	
Species	Summer	Winter	Summer	Winter	
American eel		3		255	
Blueback herring		604		1,184	
Gizzard shad		1		21	
Alewife		102		896	
American shad		54		790	
Atlantic menhaden		116		1,816	
Bay anchovy	570	5,591	1,752	3,459	
Oyster toadfish	5	13	88	1,817	
Skilletfish	1	13	1	45	
Red hake		1		5	
Spotted hake	1	15	100	116	
Striped cusk eel	12		115		
Atlantic silverside		765		3,973	
Northern pipefish	_	20		34	
Black seabass	6		200		
Weakfish	. 102		12,070		
Spot	84	152	8,964	1,457	
Atlantic croaker	92	8,804	10,370	11,279	
Tautog	2		1,880	100	
Feather blenny		13		100	
Naked goby	,	9	1.0	5	
Butterfish	1	-	10		
Northern searobin	,	1	0.0	4	
Striped searobin	1	5	82	2.2	
Smallmouth flounder	13		2 262	22 3,353	
Summer flounder	2	49	2,262 95	3,333	
Windowpane flounder	2	1	95	670	
Winter flounder	96	33	1,830	1,029	
Hogchoker		40		•	
Blackcheek tonguefish	1	40	4	152	
	989	16,405	39,823	32,482	

Table 6. Summary of nekton utilization of aquatic resources in the Elizabeth River and lower James River.

Species	
Blueback herring	Winter nursery grounds, spring spawning probably in the upstream tidal creeks of the Elizabeth River
Alewife	11
American shad	11 11
Atlantic menhaden	Probably nursery ground
Bay anchovy	Permanent resident
Striped anchovy	Adult and juvenile summer feeding grounds in the lower James River
Oyster toadfish	Permanent resident
Clingfish	11 11
Banded killifish	Permanent resident of beach zone community
Striped killifish	Permanent resident of beach zone community
Atlantic silverside	Permanent resident
Striped bass	Winter nursery ground in the upper reaches of the Elizabeth River, probably some spawning in upstream tidal creeks of the Elizabeth River

Table 6. (continued)

Species	
Weakfish	Summer/fall nursery grounds, adult and juvenile summer, fall feeding ground at the mouth of the Elizabeth River and in the lower James River
Spot	Winter nursery grounds in the upper reaches of the Elizabeth River, adult and juvenile summer feeding grounds
Atlantic croaker	Winter/summer nursery grounds, adult summer feeding grounds at the mouth of the Elizabeth River and in the lower James River
Feather blenny	Permanent resident of oyster communities
Naked goby	n n
Summer flounder	Adult and juvenile summer feeding grounds at the mouth of the Elizabeth River and in the lower James River
Blackcheek tonguefish	Permanent resident
Hogchoker	Permanent resident

Figure 1

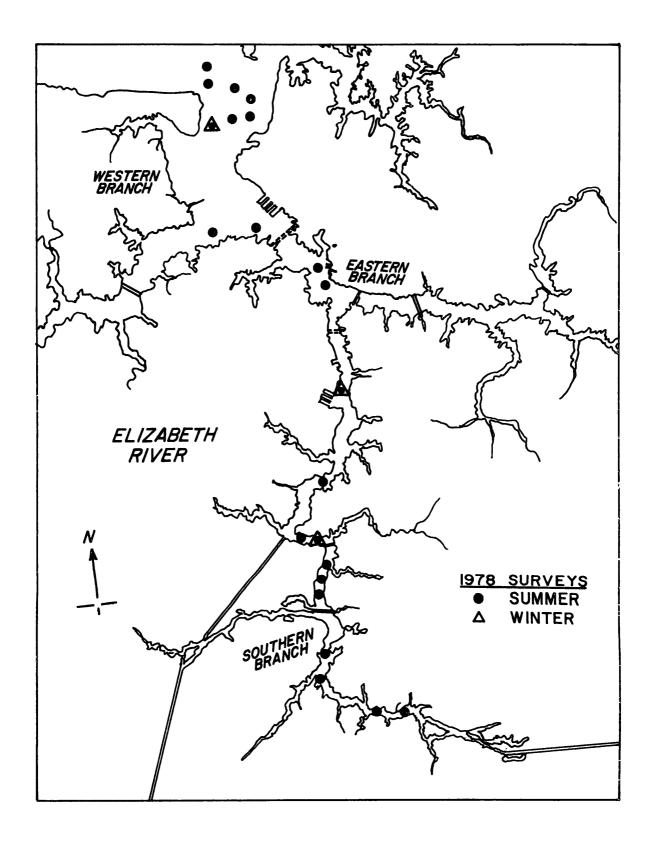
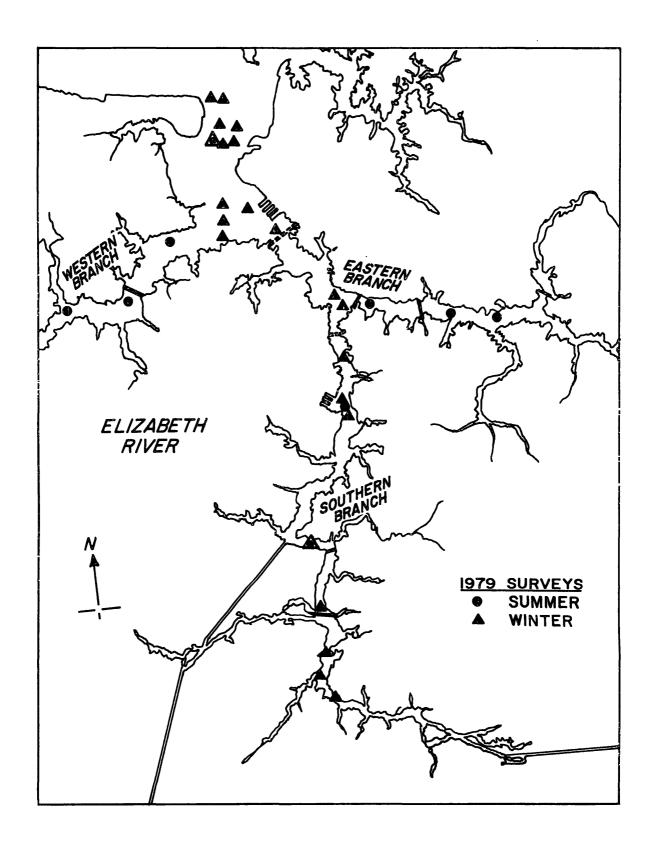
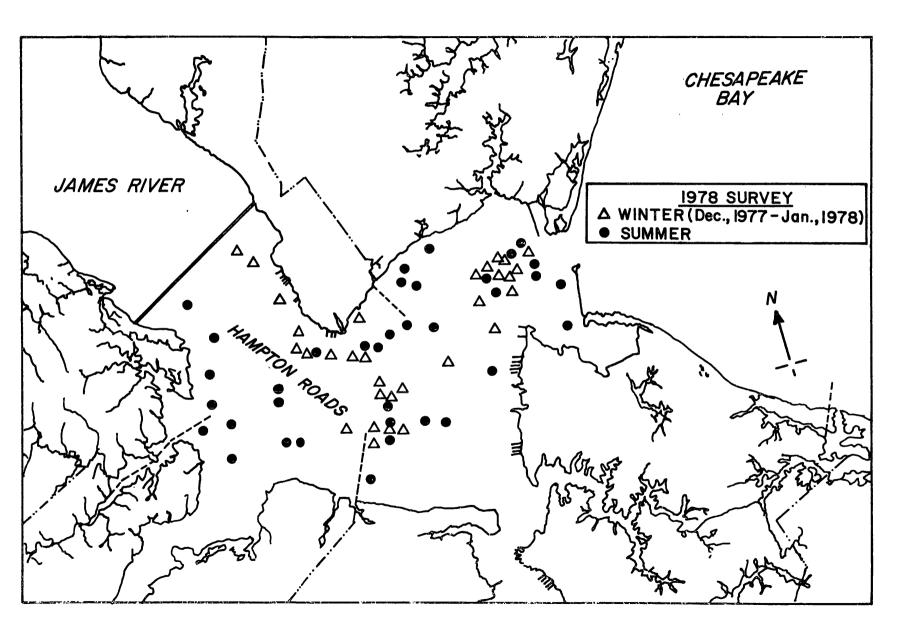


Figure 2





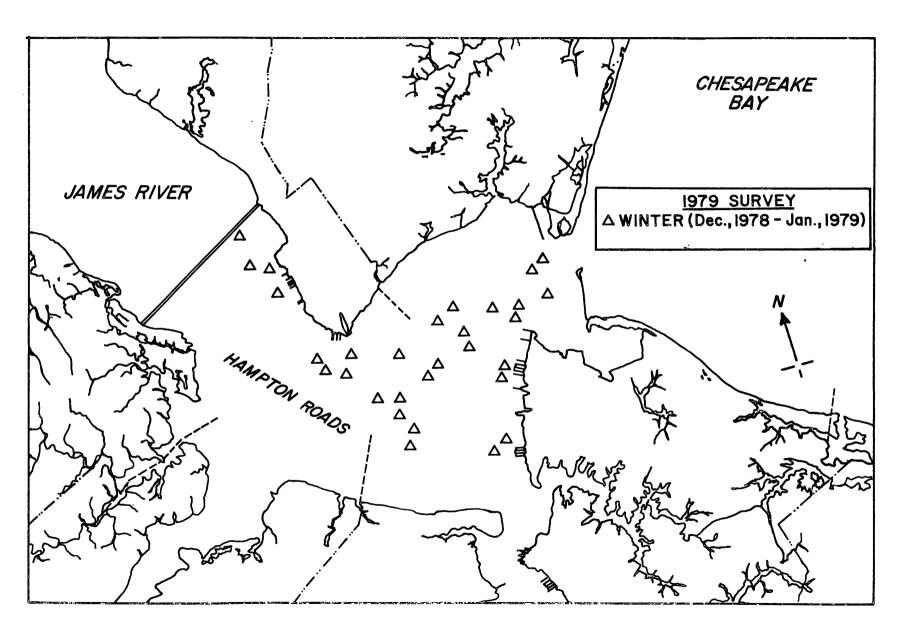


Figure 5

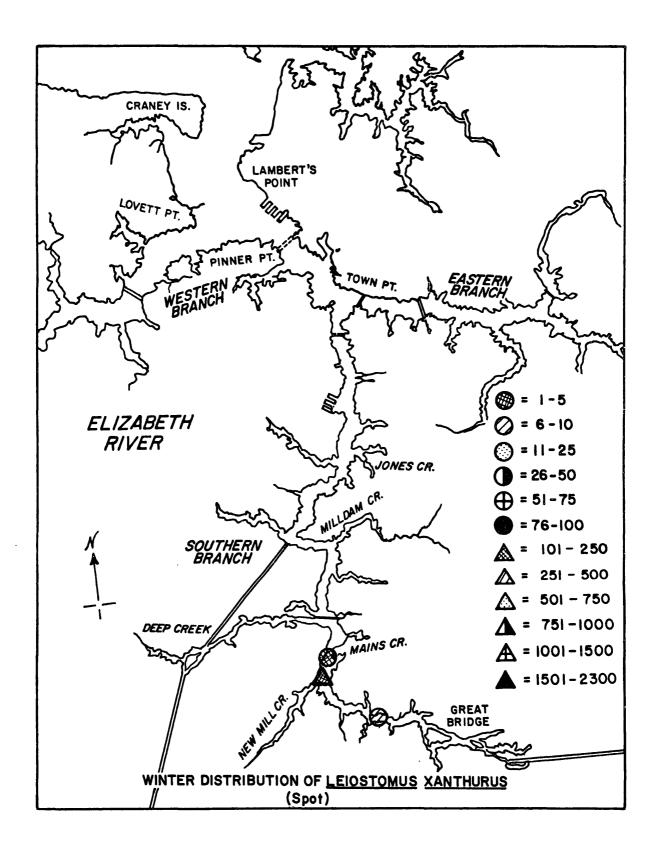


Figure 6

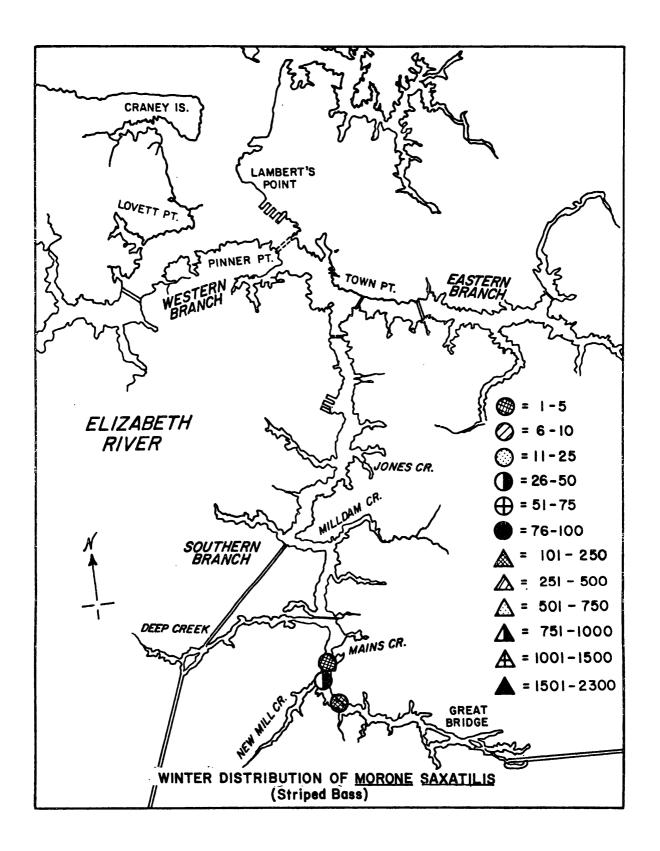


Figure 7

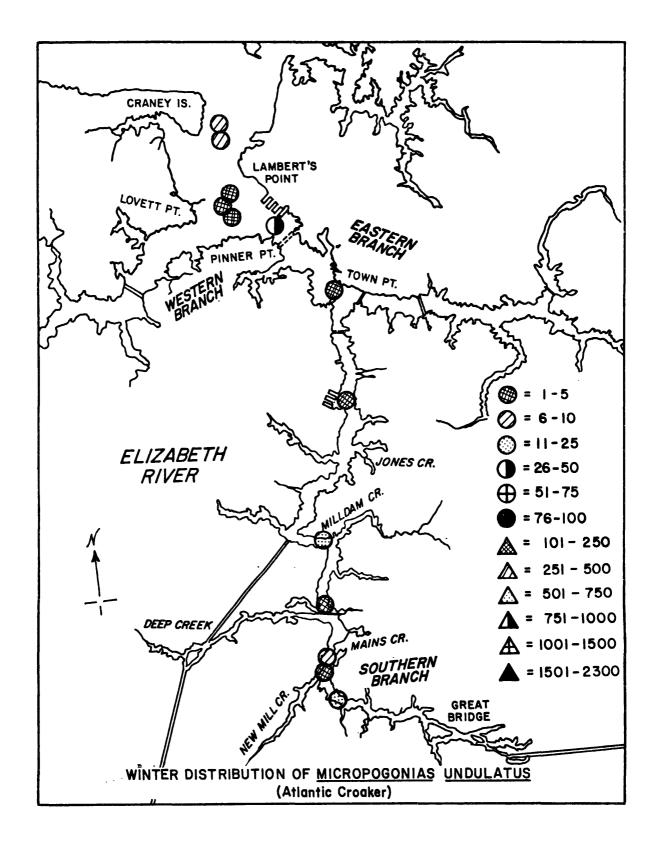


Figure 8

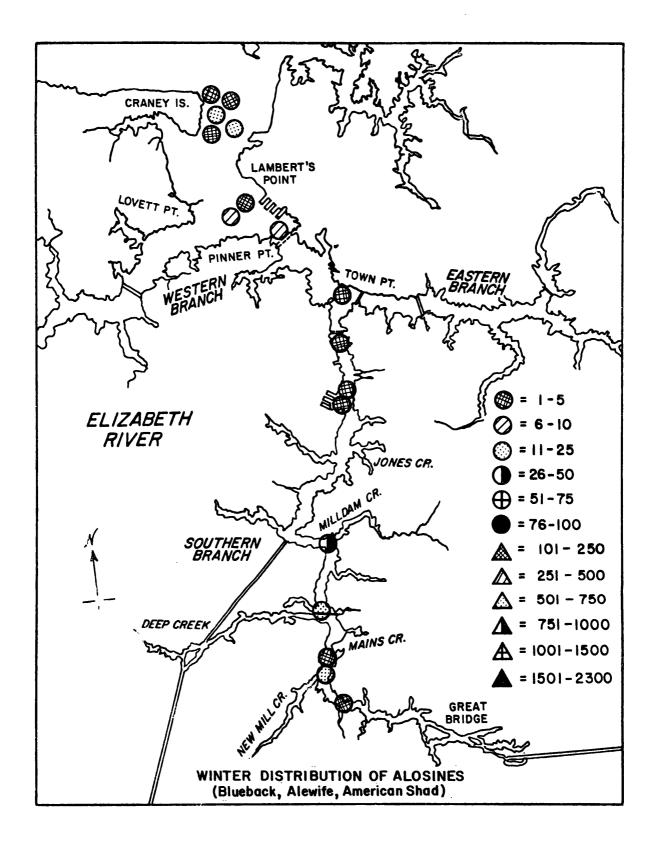


Figure 9

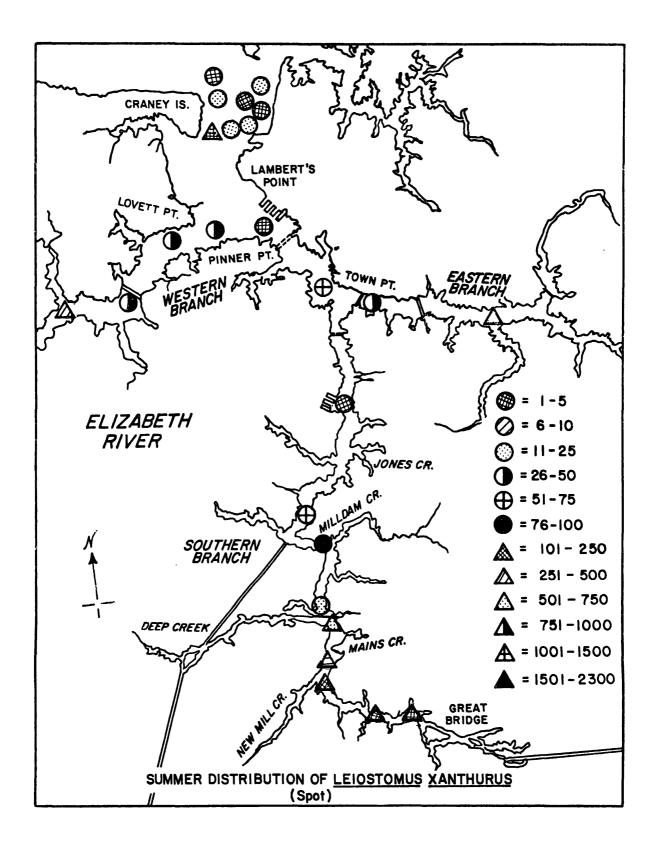


Figure 10

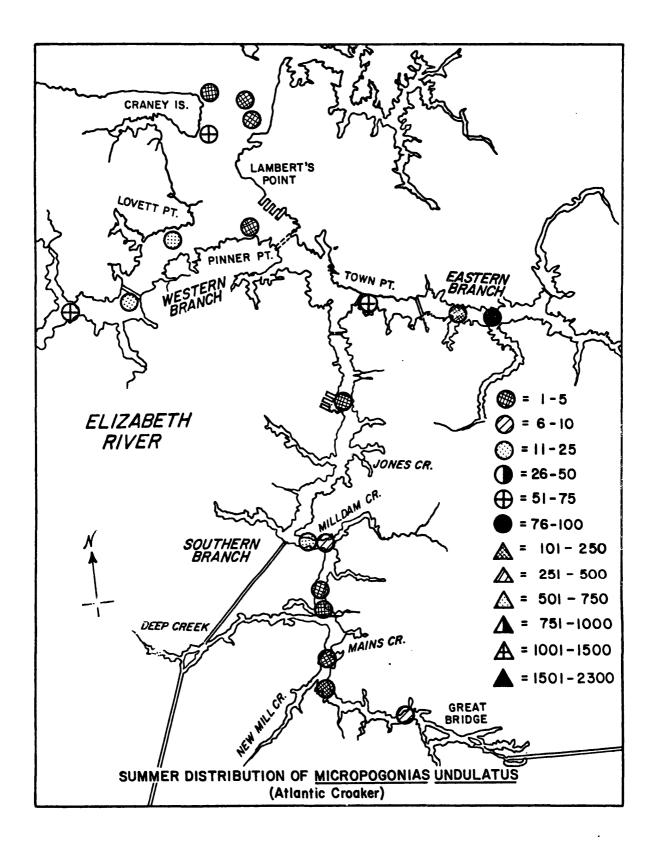


Figure 11

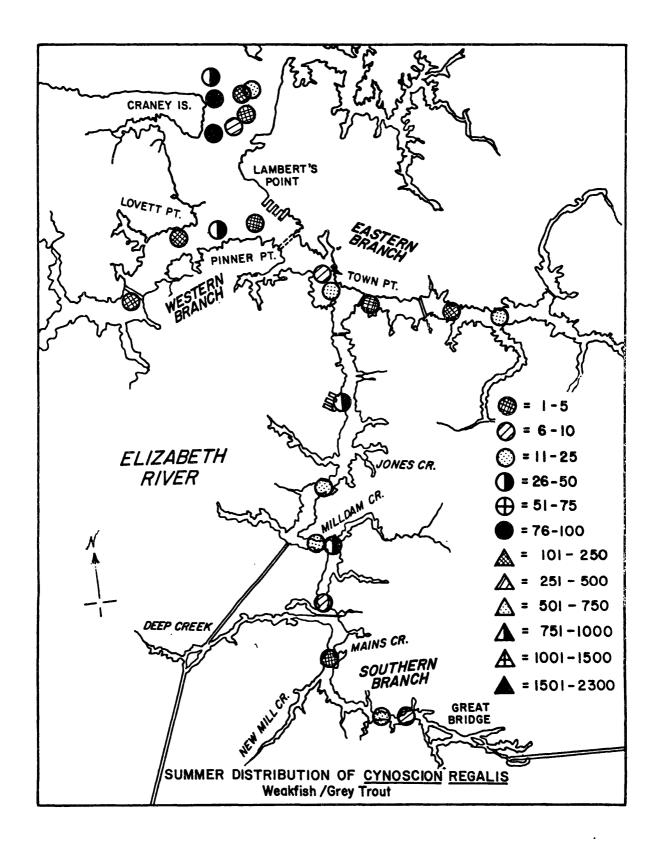


Figure 12. Winter Distribution of Herring, Shad, Blueback, Alewife, American shad and Hickory shad) in the Lower James River.

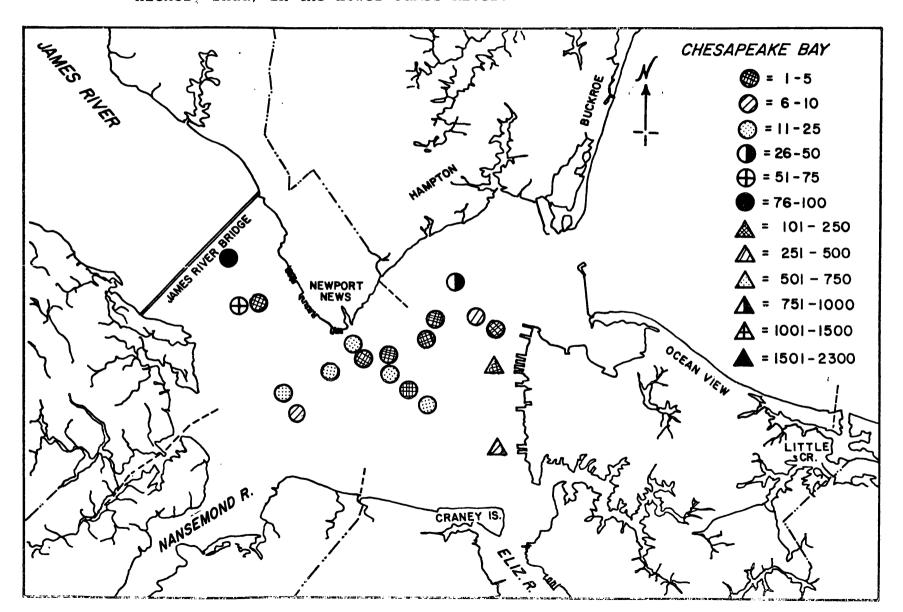
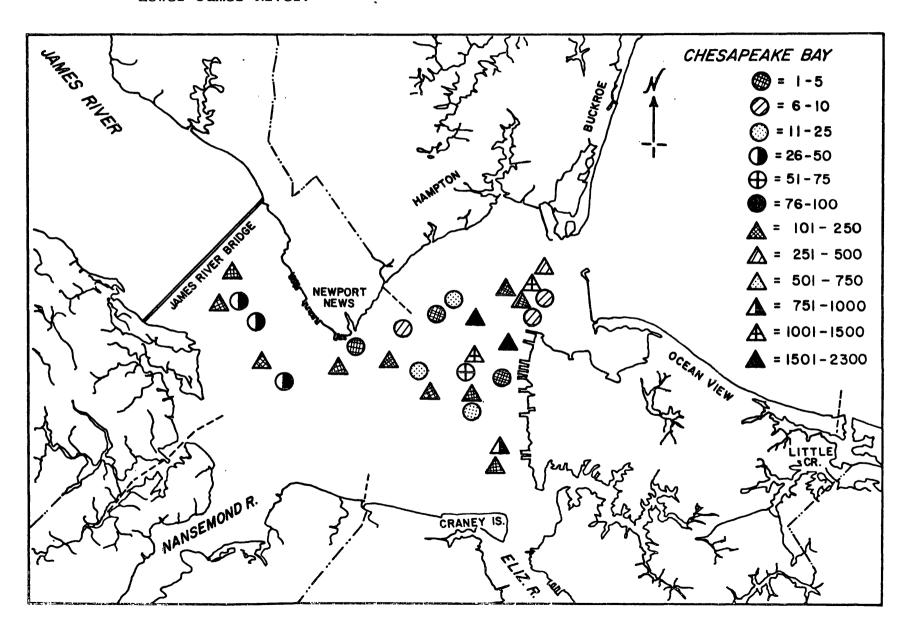


Figure 13. Winter Distribution of Micropogonias undulatus, Atlantic croaker in the Lower James River.



## OYSTER AND HARD CLAM DISTRIBUTION AND ABUNDANCE IN HAMPTON ROADS AND THE LOWER JAMES RIVER

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### Introduction

Hampton Roads and the Lower James River support large populations of oysters, <u>Crassostrea virginica</u>, and hard clams, <u>Mercenaria mercenaria</u> which are vitally important to the seafood industry as a source of seed oysters and hard clams. They are also the most vulnerable to the impacts of dredging activities because of their non-motile nature.

The most critical stage in the life cycle of the oyster are the egg, larval and setting stages where the free-swimming larvae develop, settle to the bottom and metamorphose into their adult form. The development of the egg to larvae has been shown to be affected by concentrations of suspended solids in the range of 100-200 mg/l (See the section on the effects of suspended solids in this report). These larvae also need a clean hard substrate upon which to strike and metamorphose (spatfall). In order to minimize the impacts on the oyster population it is important to avoid excessive concentrations of suspended solids and concomitant sedimentation during periods when these critical life stages are present in the estuary. Periods of peak spatfall at selected stations in Hampton Roads and the Lower James River are provided in Table 1 and Figure 1.

Adult oysters can withstand several days of elevated suspended solids levels by pumping at reduced rates or even closing their shells completely. However, rapid sedimentation in excess of .25 inch will have an adverse effect on adults and will probably kill newly settled spat.

Clam larvae are less susceptible to adverse effects from increased suspended solids. In fact, they spend most of their early sedentary life stages in the floc layer at the sediment-water interface where suspended solids levels are approximately 150 mg/l. Principal spawning times are June and early July.

Table 1. Spatfall records for the Hampton Roads and lower James River (VIMS data).

Spatfall on Shellstrings\* Annual Summary 1976-1979

### JAMES RIVER

			Hampto	n Flats			Nansemo	nd Ridge		New	port News	Tax Of	fice
Dates	Exposed**	1976	1977	1978	1979	1976	1977	1978	1979	1976	1977	1978	1979
Jun	19-25						0.0	7	0.0	0.0	0.0	70.0	0.0
Jun	25-Jul 2	0.0	0.0	0.0		<del></del>	0.0	{ 0.0	0.0	0.0	0.0	(0.0	7
Ju1			7	0.0				า์	0.0		0.0	0.0	{0.0
Ju1	9-16	0.2	1.0	7	0.7		0.3	(0.0	0.1	0.4		0.0	0.0
Ju1	16-23	0.3	1	<b>∤0.0</b>	7	0.0	0.0	(	0.1	~-	0.4	0.0	0.1
	23-30	0.3	0.2	0.0	{4.0	0.0		0.0 د		0.2	0.3	0.0	0.0
	30-Aug 6	0.1	0.6	0.1	0.4	0.0	0.1	0.0		0.2	0.7	0.0	0.0
	6-13	0.3	0.8	0.0	3.6		0.0	0.8	3.1	0.4		0.0	0.6
Aug	13-20	7	0.7	0.1	7.6		0.1	0.2	3.3	1.0	0.6	0.0	1.1
	20-27	} 1.7	0.5	0.3	1.7		7	0.9	0.4	0.0	0.4	7	
	27-Sep 3	1	2.5	1.3	0.0	0.4	{1.6	0.1	7	1.8	7	{0.1	
	3-10	1.1	0.2	0.7	1.3	0.4	0.2	0.9	0.1	2.5	1.3	0.2	0.8
	10-17	1.3	7	0.0	2.5		0.1	0.0	0.3	9.5	0.5	0.0	0.5
	17-24	7	2.7	٦	0.8	1.1	0.1	0.4		0.1	1.0	0.1	0.2
Sep	24-Oct 1	4.4	1	1.2						0.8	1.0		7
0ct	1- 8		<i>-</i> -	1.5				0.0					0.2
0ct	8-15												7
T	OTALS	9.7	9.2	5.2	22.6	1.9	2.5	3.3	7.4	16.9	6.2	0.7	3.5

### General Guide to Setting:

0.1 to 1.0 spat per shell = fair 1.1 to 10.0 spat per shell = moderate 10.1 to 100 spat per shell = heavy

<sup>\*</sup> Shows spat per shell (smooth side only).

<sup>\*\*</sup> Dates shown are for 1979. Dates in other years were approximately the same.

Not sampled in previous years.

			Brown	Shoal		]	Miles Wa	tch House	2		White	Shoal	
Dates	Exposed**	1976	1977	1978	1979	1976	1977	1978	1979	1976	1977	1978	1979
Jun	19-25		]0.0	0.0		0.0	] o.o	0.0		0.0		0.0	
	25-Jul 2	0.0	1	0.0		0.0	30.0	0.0		0.0	0.0	0.0	
			0.0	0.0	0.1			0.0	0.0	0.0		0.1	0.0
	9-16	0.9		0.0	0.3	0.0		0.0	0.5	0.0	0.1	0.0	0.0
	16-23	0.3	0.0	0.0	0.5	0.1	0.0	0.0	0.1	0.1		0.0	0.1
	23-30	0.2	0.1	0.0	0.4	0.0	0.0	0.0	0.7	0.0	0.0	0.0	0.7
	30-Aug 6	0.0	0.0	0.1	0.4	0.0	0.0	0.1	0.2	0.0	0.1	0.0	1.6
	6-13	0.0	0.5	. 0.2	0.2	0.0	0.5	0.3	0.4	0.1	3.0	_ 0.1	2.7
	13-20	0.8	0.7	0.2	2.0	0.2	0.5	0.1	2.9	1.3	0.6	0.0	2.0
_	20-27	0.5	0.3	0.1	0.0	0.1	0.5	0.2	0.0	0.2	6.0	١	0.5
	27-Sep 3	0.8	0.6	0.1	0.4	0.1	0.2		0.1	0.5	4.1	0.2	0.2
		3.9	0.5	0.0	0.5	0.1	{0.2	0.9	0.2	1.7	0.4	0.3	0.3
	10-17	6.7	0.4	0.5	1.5	0.4	٥٠. ٢	0.3	0.0	2.6	0.0	0.2	0.1
_	17-24	3.2	0.5	0.7	1.3	0.6		1.0	0.8	2.3	0.7	0.9	0.8
-	24-Oct 1	0.3	0.1	0.3	30.0	0.0	0.0	0.7	٥.0	0.0	0.8	0.6	0.0
	1- 8				{0.0			0.0	{0.0			0.0	0.0
0ct	8–15				] ***				3				}***
TO	OTALS	17.6	3.7	2.2	7.6	1.6	1.9	3.6	5.9	8.8	15.8	2.4	9.0
			Wreck	Shoal		W	arwick R	iver Mout	th		Point o	of Shoal	
Date	Exposed**	1976		Shoa1 1978	1979		arwick R			1976		of Shoal 1978	1979
Date	Exposed**	1976	Wreck 1977	Shoa1 1978	1979	W 1976	arwick R	iver Mou	1979	1976	Point o	of Shoal 1978	1979
	_			1978	1979 					1976 		1978	1979 
Jun	Exposed** 18-25 25-Jul 2	1976 0.0 0.0	1977			1976	1977	1978	1979	1976  0.0	1977		
Jun Jun	18-25	0.0	1977 0.0	1978		1976	1977	1978 0.0	1979		1977 	1978 0.0	
Jun Jun Ju1	18-25 25-Jul 2	0.0	1977 0.0 0.0	1978 0.0 0.0		1976  	1977 0.0 0.0	1978 0.0 0.0	1979  	 0.0	1977  0.0	1978 0.0 0.0	
Jun Jun Jul Jul	18-25 25-Jul 2 2- 9	0.0 0.0	1977 0.0 0.0 0.0	1978 0.0 0.0 0.0	  0.0	1976  	0.0 0.0 0.0	1978 0.0 0.0 0.0	1979   0.0	0.0	1977  0.0 0.0	1978 0.0 0.0 0.0	  0.0
Jun Jun Jul Jul Jul Jul	18-25 25-Jul 2 2- 9 9-16 16-23 23-30	0.0 0.0  0.0	1977 0.0 0.0 0.0 0.0 0.0 0.0	1978 0.0 0.0 0.0 0.0	 0.0 0.3  0.5	1976   	1977 0.0 0.0 0.0  0.0 0.0	1978 0.0 0.0 0.0 0.0	1979  0.0 0.0 0.0 0.0	0.0  0.0	1977  0.0 0.0 0.0	1978 0.0 0.0 0.0 0.0	 0.0 0.1
Jun Jun Jul Jul Jul Jul Jul	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6	0.0 0.0  0.0 0.0 0.0	1977 0.0 0.0 0.0 0.0 0.0	1978 0.0 0.0 0.0 0.0 0.0	 0.0 0.3  0.5	1976    0.0	1977 0.0 0.0 0.0  0.0 0.0	1978 0.0 0.0 0.0 0.0 0.0 0.0	1979  0.0 0.0 0.0 0.0 0.0	0.0  0.0 0.0	1977  0.0 0.0 0.0 0.0 0.0 0.1	1978 0.0 0.0 0.0 0.0 0.0	 0.0 0.1 0.0
Jun Jun Jul Jul Jul Jul Aug	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13	0.0 0.0  0.0 0.0	1977 0.0 0.0 0.0 0.0 0.0 0.0	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.2	 0.0 0.3  0.5	1976    0.0 0.0 0.2 0.1	1977 0.0 0.0 0.0  0.0 0.0 0.0 2.2	1978 0.0 0.0 0.0 0.0 0.0 0.0	1979  0.0 0.0 0.0 0.0	 0.0  0.0 0.0 0.0 0.0	1977  0.0 0.0 0.0 0.0 0.1 0.3 5.7	1978 0.0 0.0 0.0 0.0 0.0 0.0	 0.0 0.1 0.0 0.0
Jun Jul Jul Jul Jul Jul Aug Aug	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13 13-20	0.0 0.0 0.0 0.0 0.0 0.0 0.1	1977 0.0 0.0 0.0 0.0 0.0 0.0	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.6 0.0	 0.0 0.3  0.5 1.2	1976    0.0 0.0 0.2	1977 0.0 0.0 0.0  0.0 0.0 0.0 2.2 0.3	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0	1979  0.0 0.0 0.0 0.0 0.0	 0.0  0.0 0.0 0.0	1977  0.0 0.0 0.0 0.0 0.1 0.3 5.7 2.6	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0	 0.0 0.1 0.0 0.0
Jun Jul Jul Jul Jul Jul Aug Aug	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13	0.0 0.0  0.0 0.0 0.0 0.0	1977 0.0 0.0 0.0 0.0 0.0 0.0 0.0	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.2	 0.0 0.3  0.5 1.2 1.8	1976    0.0 0.0 0.2 0.1	1977 0.0 0.0 0.0  0.0 0.0 0.0 2.2 0.3 0.2	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	1979  0.0 0.0 0.0 0.0 0.0 0.0	 0.0  0.0 0.0 0.0 0.0	1977  0.0 0.0 0.0 0.0 0.1 0.3 5.7	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	 0.0 0.1 0.0 0.0 0.4 0.4
Jun Jul Jul Jul Jul Aug Aug Aug	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13 13-20 20-27 27-Sep 3	0.0 0.0 0.0 0.0 0.0 0.0 0.1 0.2 0.0	1977 0.0 0.0 0.0 0.0 0.0 0.0 0.4 0.7 0.5 1.1	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.6 0.0	 0.0 0.3  0.5 1.2 1.8 }	1976    0.0 0.0 0.2 0.1	1977 0.0 0.0 0.0  0.0 0.0 0.0 2.2 0.3	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0	1979  0.0 0.0 0.0 0.0 0.0 0.0	 0.0  0.0 0.0 0.0 0.0 0.2 0.0	1977 0.0 0.0 0.0 0.0 0.1 0.3 5.7 2.6 3.5 1.6	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0	 0.0 0.1 0.0 0.0 0.4 0.4 1.0 0.0
Jun Ju1 Ju1 Ju1 Ju1 Aug Aug Aug Aug	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13 13-20 20-27 27-Sep 3 3-10	0.0 0.0  0.0 0.0 0.0 0.0 0.1 0.2	1977 0.0 0.0 0.0 0.0 0.0 0.0 0.4 0.7 0.5	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.6 0.0	 0.0 0.3  0.5 1.2 1.8 }	1976    0.0 0.0 0.2 0.1 0.3 0.1	1977 0.0 0.0 0.0  0.0 0.0 0.0 2.2 0.3 0.2	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	1979  0.0 0.0 0.0 0.0 0.0 0.0	 0.0  0.0 0.0 0.0 0.0 0.2 0.0	1977  0.0 0.0 0.0 0.1 0.3 5.7 2.6 3.5	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	 0.0 0.1 0.0 0.0 0.4 0.4 1.0
Jun Ju1 Ju1 Ju1 Ju1 Aug Aug Aug Sep	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13 13-20 20-27 27-Sep 3 3-10 10-17	0.0 0.0 0.0 0.0 0.0 0.0 0.1 0.2 0.0 	1977  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.4 0.7 0.5 1.1 0.1	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.6 0.0 0.0 0.1 0.1	 0.0 0.3  0.5 1.2 1.8 1.5 0.2 0.1	1976 0.0 0.0 0.2 0.1 0.3 0.1 0.0 0.0	1977 0.0 0.0 0.0 0.0 0.0 0.0 2.2 0.3 0.2 0.2 0.0	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	1979 0.0 0.0 0.0 0.0 0.0 0.3 0.0	 0.0  0.0 0.0 0.0 0.0 0.2 0.0	1977 0.0 0.0 0.0 0.0 0.1 0.3 5.7 2.6 3.5 1.6 0.8 0.1	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	 0.0 0.1 0.0 0.0 0.4 0.4 1.0 0.0 0.1
Jun Jul Jul Jul Jul Aug Aug Aug Sep Sep	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13 13-20 20-27 27-Sep 3 3-10 10-17 17-24	0.0 0.0 0.0 0.0 0.0 0.1 0.2 0.0  0.0	1977  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.4 0.7 0.5 1.1 0.1 0.1	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.6 0.0 0.1 0.1 0.2 0.1	 0.0 0.3  0.5 1.2 1.8 1.5 0.2 0.1 0.5 0.7	1976 0.0 0.0 0.2 0.1 0.3 0.1 0.0 0.0	1977  0.0 0.0 0.0 0.0 0.0 0.0 0.0 2.2 0.3 0.2 0.2 0.0 0.0 0.0	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	1979 0.0 0.0 0.0 0.0 0.0 0.3 0.0 0.0	 0.0  0.0 0.0 0.0 0.2 0.0 0.1 0.0 0.0	1977 0.0 0.0 0.0 0.0 0.1 0.3 5.7 2.6 3.5 1.6 0.8 0.1 0.1	1978  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	 0.0 0.1 0.0 0.0 0.4 0.4 1.0 0.0 0.1 0.0
Jun Jul Jul Jul Jul Aug Aug Aug Sep Sep Sep	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13 13-20 20-27 27-Sep 3 3-10 10-17 17-24 24-Oct 1	0.0 0.0 0.0 0.0 0.0 0.1 0.2 0.0  0.0 0.7 1.1	1977  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.4 0.7 0.5 1.1 0.1 0.1 0.0 0.2	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.6 0.0 0.1 0.1 0.2 0.1	 0.0 0.3  0.5 1.2 1.8 1.5 0.2 0.1	1976 0.0 0.0 0.2 0.1 0.3 0.1 0.0 0.0	1977 0.0 0.0 0.0 0.0 0.0 0.0 2.2 0.3 0.2 0.2 0.0	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	1979 0.0 0.0 0.0 0.0 0.0 0.3 0.0	 0.0  0.0 0.0 0.0 0.2 0.0 0.1 0.0	1977 0.0 0.0 0.0 0.0 0.1 0.3 5.7 2.6 3.5 1.6 0.8 0.1	1978  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	 0.0 0.1 0.0 0.0 0.4 0.4 1.0 0.0 0.1 0.0
Jun Jul Jul Jul Jul Aug Aug Aug Sep Sep Sep Sep	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13 13-20 20-27 27-Sep 3 3-10 10-17 17-24 24-Oct 1 1- 8	0.0 0.0 0.0 0.0 0.0 0.1 0.2 0.0  0.0	1977  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.4 0.7 0.5 1.1 0.1 0.1	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.6 0.0 0.1 0.1 0.2 0.1	 0.0 0.3  0.5 1.2 1.8 1.5 0.2 0.1 0.5 0.7	1976 0.0 0.0 0.2 0.1 0.3 0.1 0.0 0.0	1977  0.0 0.0 0.0 0.0 0.0 0.0 0.0 2.2 0.3 0.2 0.2 0.0 0.0 0.0 0.0	1978  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1 0.1 0.	1979 0.0 0.0 0.0 0.0 0.0 0.3 0.0 0.0	 0.0  0.0 0.0 0.0 0.2 0.0 0.1 0.0 0.0	1977 0.0 0.0 0.0 0.0 0.1 0.3 5.7 2.6 3.5 1.6 0.8 0.1 0.1	1978  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	0.0 0.1 0.0 0.4 0.4 1.0 0.0 0.1 0.0 0.2 0.0
Jun Jul Jul Jul Jul Aug Aug Aug Sep Sep Sep Sep	18-25 25-Jul 2 2- 9 9-16 16-23 23-30 30-Aug 6 6-13 13-20 20-27 27-Sep 3 3-10 10-17 17-24 24-Oct 1	0.0 0.0 0.0 0.0 0.0 0.1 0.2 0.0  0.0 0.7 1.1	1977  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.4 0.7 0.5 1.1 0.1 0.1 0.0 0.2	1978 0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.6 0.0 0.1 0.1 0.2 0.1	 0.0 0.3  0.5 1.2 1.8 1.5 0.2 0.1 0.5 0.7	1976 0.0 0.0 0.2 0.1 0.3 0.1 0.0 0.0	1977  0.0 0.0 0.0 0.0 0.0 0.0 0.0 2.2 0.3 0.2 0.2 0.0 0.0 0.0	1978  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	1979 0.0 0.0 0.0 0.0 0.0 0.3 0.0 0.0	 0.0 0.0 0.0 0.0 0.0 0.2 0.0 0.1 0.0 0.0 0.2	1977 0.0 0.0 0.0 0.0 0.1 0.3 5.7 2.6 3.5 1.6 0.8 0.1 0.1 0.1	1978  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	 0.0 0.1 0.0 0.0 0.4 0.4 1.0 0.0 0.1 0.0

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			Mulberr	y Swash			Horsehe	ad Shoal			Deepwat	er Shoal	
Dates	Exposed**	1976	1977	1978	1979	1976	1977	1978	1979	1976	1977	1978	1979
Jun	18-25			0.0				0.0		<del></del> .		0.0	
Jun	25-Jul 2	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0	
Ju1	2- 9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		0.0	0.0	0.0	0.6	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0
Ju1	16-23	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
<b>Jul</b>	23-30	0.0	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0
Ju1	30-Aug 6	0.0	0.2	0.0	0.6	0.0	0.0	0.0	0.6	0.0	0.0	0.4	0.5
	6-13	0.0	4.0	0.1	0.7	0.3	1.1	0.1	1.4	0.0	0.6	0.0	0.3
	13-20	0.1	0.0	<b>.</b> .	1.2	0.1	0.5	0.0	0.7	0.0	0.4	0.0	0.7
Aug	20-27	0.0	1.7	0.4	0.5	0.0	2.3	0.1	0.0	0.0	1.5	0.6	0.3
Aug	27-Sep 3	0.0	2.4	0.3	0.3	0.0	0.4	0.0	0.2	0.0	0.4	0.0	0.4
Sep	3-10	0.4	0.1	1.1	0.0	0.1	0.2	0.5	0.0	1.1	0.2	0.4	0.4
	10-17	0.2	0.0	0.7	0.1	0.7	0.1		0.0	0.2	0.3	0.0	0.0
Sep	17-24	0.0	0.1	0.4	0.0	0.2	0.0	0.1	0.0	0.2	0.1	0.3	0.0
Sep	24-Oct 1	0.0	0.0	0.7	0.0	0.1	0.0	0.2	0.0	0.3	0.0	0.3	0.0
0ct	1- 8			0.2	}0.0				700			0.2	}o.o
0ct	8-15				ζο.υ				}o.o				} 0.0
TO	<b>DTALS</b>	0.7	8.5	3.9	4.0	1.5	4.6	1.0	3.0	0.8	3.5	2.2	2.6

### SHELLSTRING SURVEY STATIONS

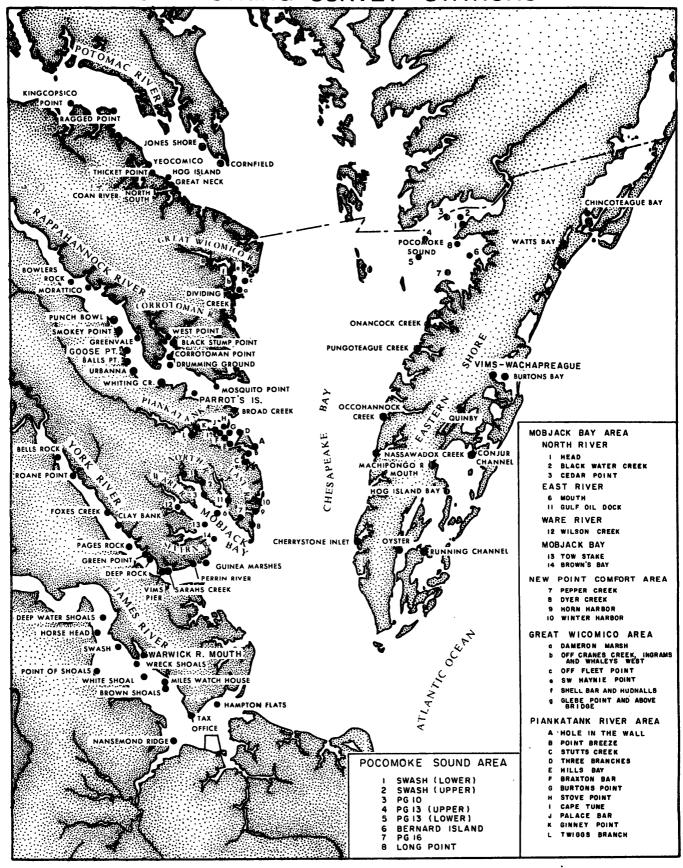
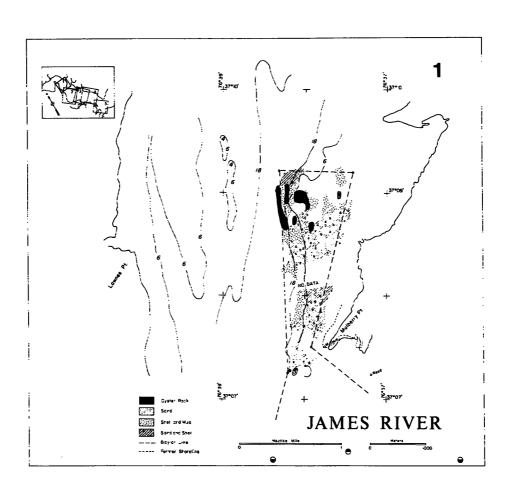
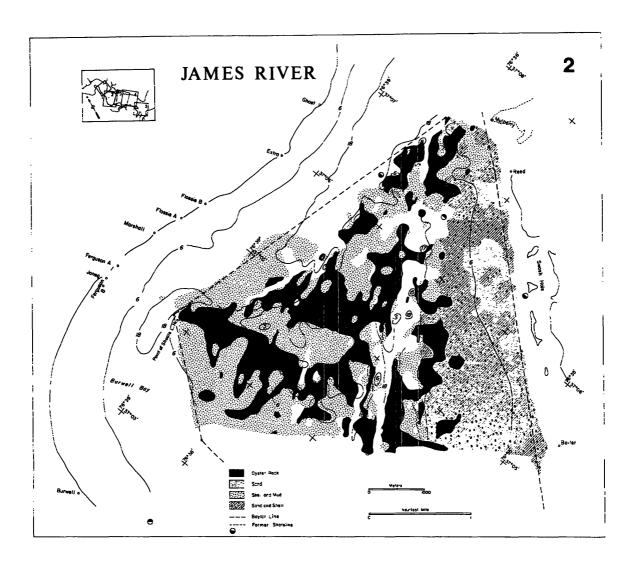


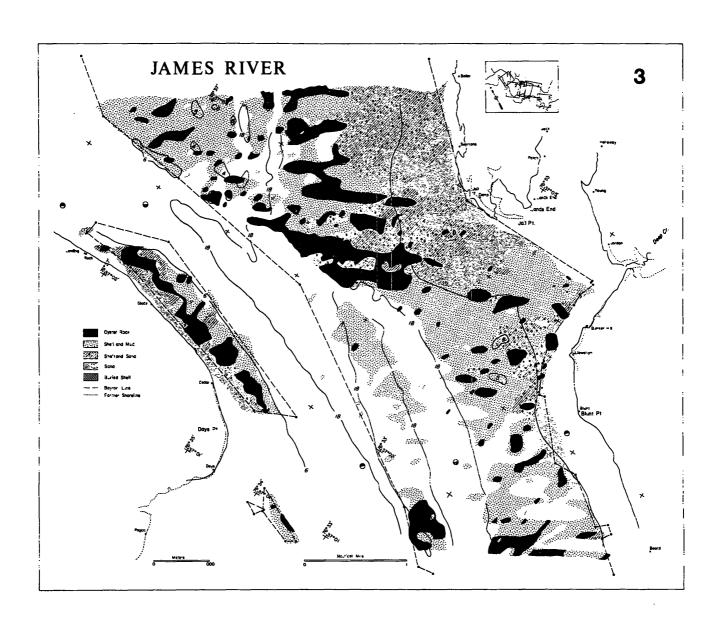
Figure 1. Locations of shellstring spatfall sample stations.

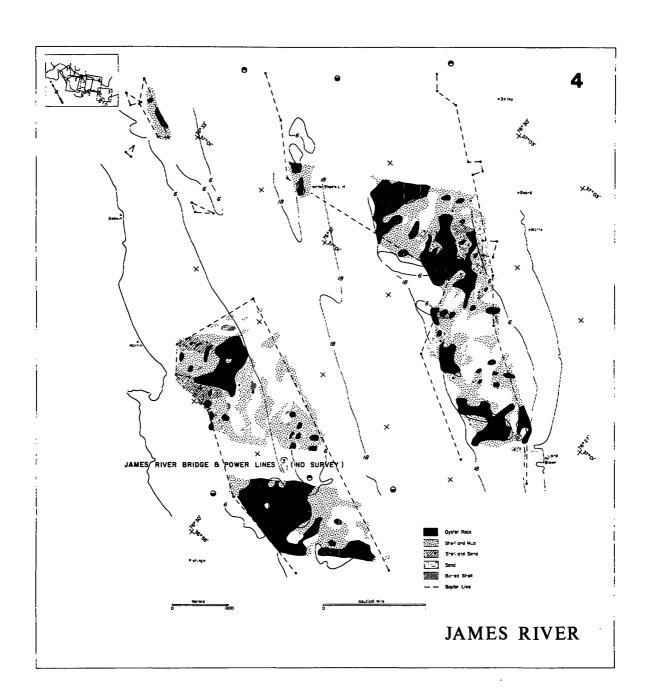
Table 2. Estimated of oyster, <u>Crassostrea virginica</u>, densities on different substrates in Baylor Survey public grounds in the Lower James River (Haven. Whitcomb and Kendall, MS in preparation).

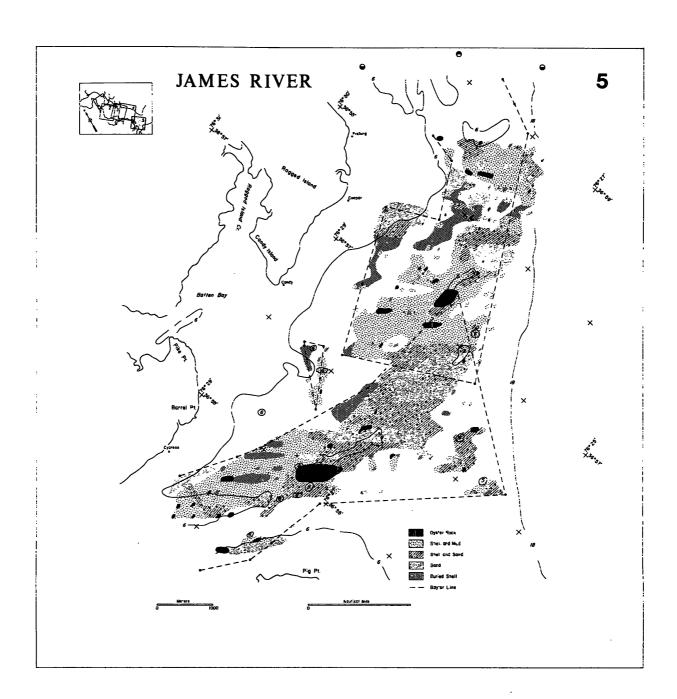
Area Designation		Est.	Total No.
Substrate Type	No. Acres	Density (bu/acre)	Bushels (Millions)
			<u> </u>
AREA I (Plates 1 and 2)			
Oyster Rock	1812	460	0.833
Mud and Shell	1962	114	0.224
Sand and Shell	1690	125	0.211
Totals	5464		1.268
AREA II (Plate 3)			
Oyster Rock	1348	405	0.546
Mud and Shell	3237	78	0.252
Sand and Shell	1599	75	0.120
Totals	6184	·	0.918
AREA III (Plates 4 and 5)			
Oyster Rock	1171	471	0.551
Mud and Shell	2475	108	0.267
Sand and Shell	1116	108	0.120
Totals	4762		0.938
TOTALS ALL AREAS	16,410		3.124











The adult hard clams have a limited amount of vertical mobility and probably will not be adversely effected by up to .5 inch of new silt.

The distribution of oysters on the Baylor Public Grounds in the Lower James River are depicted in Plates 1-5. This distribution is based on the areal extent of three different substrate types, oyster rock, mud and shell and sand and shell. These are considered productive or potentially productive oyster bottoms and are where the densest populations of oysters are found (Haven, Whitcomb and Kendall, MS in preparation).

The densities of oysters for each substrate type based on random sampling along transects across the river are presented in Table 2. In this table Area I refers to the area covered in Plates 1 and 2, Area II refers to Plate 3 and Area III refers to Plates 4 and 5 (Haven and Morales-Alamo, 1980).

The upriver limit of the distribution of the hard clam Mercenaria mercenaria in the James River is located at the level of the James River bridge. Several intensive surveys of hard clam populations in the James River have been conducted previously by VIMS (Haven and Loesch, 1972; Haven, Loesch and Whitcomb, 1973; and Haven and Kendall, 1974, 1975). The data from those studies form the basis for this report on the density of hard clams in Hampton Roads and the James River.

The region between just above the James River bridge and the mouth of the river at Old Point Comfort was divided into 31 plots (Figure 2). The acreage included in each of the plots was measured with a polar planimeter on a NOAA navigation chart. Eighteen of the plots were sampled in the surveys mentioned above and the outlines of their areas are based on those data. The other thirteen plots were not sampled and their areas were delineated following the boundaries of the areas sampled and bottom depth contours. The density of clams in plots not sampled was estimated on the basis of the

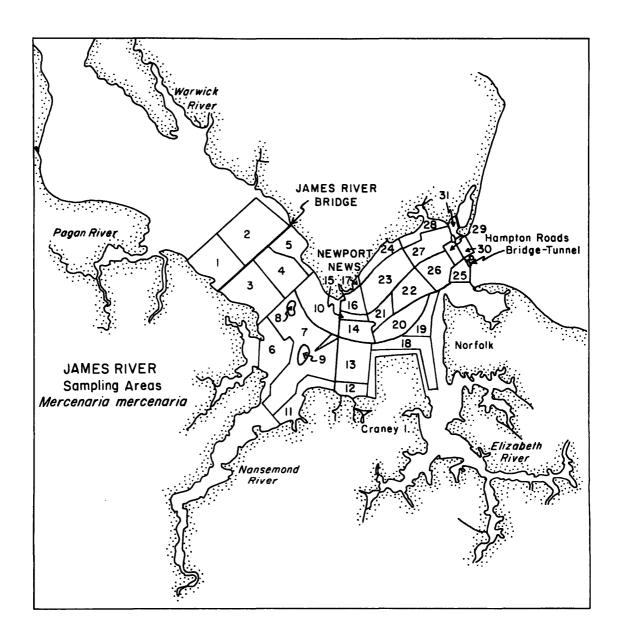


Figure 2. Division of lower James River into system of numbered plots used to estimate bottom acreage and standing crop of the hard clam <u>Mercenaria</u> mercenaria.

density in adjoining plots that were sampled, and our familiarity with the areas through conversations with clammers that work them and the nature of the bottom. These data are summarized in Table 3 (Haven and Morales-Alamo, 1980).

Table 3. Estimate of bottom acreage and densities of the hard clam, Mercenaria mercenaria, in plots surveyed between 1970 and 1974 in the Lower James River (Haven and Morales-Alamo, 1980).

Source of Data (Footnotes)	Plot No.	No. Acres	Clam Density (Bu/Acre)	Total No. (Bushels)
_	1	508	$(0.3)^5$	152
1	2	4321	6.9	29,815
<u>-</u>	3	427	$(0.3)^5$	128
1	4	1221	40.0	48,840
1	5	1928	26 1	69,601
<del>-</del>	5 6	528	$(0.3)^5$	158
2	7	5410	1.1	5,951
2	8	71	0	0
2 2 2	9	242	5.5	1,331
2	10	2352	12 1	28,459
	11	305	$(1.0)^{5}$	305
3	12	610	11.0	6,710
3	13	1126	0.3	338
3	14	1323	62.0	82,026
- 3 3 3 3 3	15	109	6.0	654
3	16	680	65.0	44,200
3	17	183	58.0 -	10,614
_	18	1075	$(25.0)_{5}^{5}$	26,875
-	19	698	$(25.0)_{5}^{5}$	17,450
_	20	1474	$(25.0)^{\frac{5}{2}}$	36,850
-	21	890	(5.0)5	4,450
•	22	1202	$(5.0)^{5}$	6,010
_	23	2266	109.8	248,807
-	24	488	109.8	53,582
4	25	571	16.08	9,182
-	26	1486	(5.0)	7,430
4	27	1473	24.12	35,529
-	28	691	$(25.0)^{-}$	17,275
4	29	386	10.05	3,879
4	30	352	3.35	1,179
4	31	182	8.04	1,471
TOTALS		34,579		565,712

Haven, D. S., J. G. Loesch and J. P. Whitcomb. 1973. An investigation into commercial aspects of the hard clam fishery and development of commercial gear for the harvest of molluscs. Final Report, Contract 3-124-R with the Virginia Marine Resources Commission, for the National Marine Fisheries Service. 119 pp. Virginia Institute of Marine Science, Gloucester Point, Virginia.

Haven, D. and P. Kendall. 1975. A survey of commercial shellfish in the vicinity of Newport News Point and Pig Point in the lower James River. Final Report to McGaughy, Marshall and McMillan - Hazen and Sawyer. In: Fang, C.S. (Project Manager): Oceanographic, Water Quality and Modeling Studies

for the Outfall from a Proposes Nansemond Waste Water Treatment Plant, Volume 4. p. 1-28 and summary. Special Report No. 86 in Applied Marine Science and Ocean Engineering. Virginia Institute of Marine Science, Gloucester Point, Virginia.

- <sup>3</sup>Haven, D. S. and J. G. Loesch. 1972. Hampton Roads corridor survey report for the Virginia Department of Highways. Final Report. 12 pp. + 6 tables. Virginia Institute of Marine Science, Gloucester Point, Virginia.
- Haven, D. and P. Kendall. 1974. A final report to the Virginia Department of Highways on hard clam (Mercenaria mercenaria) populations in the vicinity of the Hampton Roads Bridge-Tunnel (I-64). 15 pp + 6 tables + 18 figures. Virginia Institute of Marine Science, Gloucester Point, Virginia.
- <sup>5</sup>Density given represents a guess-estimate based on familiarity with the area and data from surrounding areas.

### Bibliography

- Haven, D. S. and R. Morales-Alamo. 1980. Estimate of the total weight of Kepone in the major components of the molluscan fauna of the James River, Virginia. Manuscript. Virginia Institute of Marine Science, Gloucester Point, Virginia.
- Haven, D. S., J. P. Whitcomb and P. C. Kendall. (MS in preparation). The present and potential productivity of the Baylor Grounds in Virginia. Phase III. Pocomoke Sound and James River. Contract No. 3-265-R-3 with the Virginia Marine Resources Commission for the National Marine Fisheries Service. Virginia Institute of Marine Science, Gloucester Point, Virginia.

## Spawning Activity and Nursery Utilization by Fishes in Hampton Roads and its Tributaries

bу

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March, 1981

# SPAWNING ACTIVITY AND NURSERY UTILIZATION BY FISHES IN HAMPTON ROADS AND ITS TRIBUTARIES

The information concerning the distribution of fish eggs, larvae and spawning activity in the Hampton Roads area is very limited. The available information does, however, indicate that there is considerable spawning activity, primarily forage species but with some alosine and other anadromous fish in selected areas, and heavy utilization of the area by postlarvae and juveniles as a nursery area.

The report by Hedgepeth et al. (This report) outlines nekton utilization of the study area. They state that the Hampton Roads area and the Elizabeth River are nursery grounds for juvenile spot, croaker, alewife, blueback herring, American shad, striped bass and weakfish. The most abundant spawning activity was by the resident forage species, particularly anchovies and silversides. The probability of spawning by alosine fishes and striped bass in the upper reaches of the Elizabeth River was also noted.

The presence of postlarvae of spot in the lower Elizabeth River in April was noted in the Hampton Roads Energy Company EIS (COE, 1977).

Table 1 presents data from Olney (1978) which show the numerical and temporal distribution of fish eggs and larvae in the lower Chesapeake Bay. The occurrence of most of these eggs and larvae with the exception of the shelf spawners and tropical intruders in similar numbers and at similar times of the year in Hampton Roads proper is very probable.

The most comprehensive study of the ichthyoplankton in the study area is one performed in conjunction with a study of the effects of a VEPCO power plant on the Southern Branch of the Elizabeth River by Ecological Analysts, Inc. (1979). Table 2 summarizes the species taken and the life history stages present.

Table 1. Species, total number and months of occurrence of fish eggs and larvae in the lower Chesapeake Bay. (Olney, 1978).

	Num	ber	Occu:	rrence
Species	Eggs	Larvae	Eggs	Larvae
Conger oceanicus		1		May
Brevoortia tyrannus	10	28	July-August	February, April- May, August
Anchoa mitchilli	18,121	49	May-August	All months
Anchoa hepsetus*	53		May-August	
Anchoa spp.		6834		May-September
Gobiesox strumosus		10		June-September
Lophius americanus*		1		May
Urophycis regius		9		March
Rissola marginata*		3		August-September
Membras martinica		47		March, August
Atherinid larvae		132		May, August
Syngnathus fuscus		50		All seasons
Hippocampus erectus		7		March, July-Augus
Prionotus spp.*	1	14	August	August
Cynoscion regalis		555		June-September
Menticirrhus spp.		30		June-August
Leiostomus xanthurus		12		March
Unidentified sciaenids	1248		May-August	
Tautoga onitis	10		May	
Hypsoblennius hentzi		181		June-September
Ammodytes sp.*		4		January-March
Gobiosoma ginsburgi		358		June-September
Gobiosoma bosci		5		June-August

TABLE 1. (continued)

	Nur	mber	0ccur	rence
Species	Eggs	Larvae	Eggs	Larvae
Microgobius thalassinus		9		June-August
Gobiidae, 6-spined**		1		August
Gobiidae, 7-spined		46		June-September
Scomber scombrus*		3		May
Peprilus triacanthus		1		July
Peprilus paru		13		August
Paralichthys dentatus		52		March
Etropus microstomus*		1		August
Scophthalmus aquosus		10		May
Pseudopleuronectes americanus		3		March-April
Trinectes maculatus	682	425	June- September	June-September
Symphurus plagiusa		152		July-August
Symphurus-type	192		June-August	
Sphoeroides maculatus		5		May, July, August
Unknowns	89	53	OctNov., MarApr.	July-August
Totals	20,406	9114		

<sup>\*</sup> SHELF SPAWNER \*\* TROPICAL INTRUDER

TABLE 2. SCIENTIFIC AND COMMON NAMES, WITH LIFE STAGES AND LIVE-DEAD EGG CATEGORIES, OF ICHTHYOPLANKTON CAPTURED IN THE SOUTHERN BRANCH STUDY AREA BETWEEN 13 FEBRUARY AND 5 SEPTEMBER 1978. (Ecological Analysts, Inc., 1979).

Scientific Name	Common Name	Live Egg	Dead Egg	Prolarvae	<u>Postlarvae</u>
Anguillidae Anguilla rostrata	freshwater eels American eel				(elver)
Cupeidae Alosa spp. Brevoortia tyrannus Dorosoma cepedianum	herrings Atlantic menhaden gizzard shad			x	x x x
Engraulidae Anchoa mitchilli	anchovies bay anchovy	x	X	x	x
Cyprinodontidae Fundulus heteroclitus	killifishes mummichog				x
Atherinidae  Menidia beryllina  Menidia menidia  unidentified silverside	silversides tidewater silverside Atlantic silverside		X X	x	X X X
Percichthyidae <u>Morone</u> <u>americana</u>	temperate bass white perch	X	x		x
Percidae  Etheostoma olmstedi Etheostoma spp. Perca flavescens	perches tessellated darter unidentified darter yellow perch			x	X X X
Sciaenidae <u>Cynoscion regalis</u> unidentified drum	drums weakfish 				X X
Gobiidae	gobies			X	X
Soleidae Trinectes maculatus	soles hogchoker	x			x
Unidentified	Sent - 3	x			X

Bay anchovy eggs were the most abundant ichthyoplankton comprising 94.1% of the total catch. The postlarvae of gobies were the most abundant larvae at 3.5% of the catch. The next most abundant segment of the ichthyoplankton was bay anchovy larvae at 1.7%. Taken together the eggs and larvae of the bay anchovy and the goby larvae represented 99.3% of the ichthyoplankton during the study (Ecological Analysts, Inc., 1979).

During February and March the only ichthyoplankton captured were American eel elvers and juvenile croakers. Postlarvae of the Atlantic menhaden began to appear in April. Silversides and gizzard shad began spawning in early April and continued through July. In mid-April white perch and yellow perch began spawning which continued through May. The bay anchovy also began spawning in mid-April but continued through September when the study ended (Ecological Analysts, Inc., 1979).

Gizzard shad and Alosa spp. preferred the upstream areas of the Southern Branch near the Great Bridge and Deep Creek locks for spawning. White perch preferred the upper reaches of the Southern Branch for their spawning while the yellow perch preferred the upper reaches of Deep Creek. During the periods of greatest abundance live and dead eggs and prolarvae of the bay anchovy were most numerous near the mouth of Deep Creek. The larvae of the Atlantic silverside were found only upstream of the mouth of Deep Creek and usually in low numbers. The larvae of the tidewater silverside, however, were common at all of the stations sampled. Goby postlarvae were well distributed but appeared to prefer the Elizabeth River stations over those in Deep Creek (Ecological Analysts, Inc. 1979).

The entire study area was used as a nursery area for bay anchovies, gobies, and the tidewater silverside. Yellow perch also used the entire study area as a nursery but their numbers were concentrated in Deep Creek and the upper reaches of the Southern Branch. The postlarvae of the white

perch were restricted to the area near Great Bridge. The postlarvae of the gizzard shad were found throughout the summer in the upper reaches of the Southern Branch and Deep Creek (Ecological Analysts, Inc. 1979).

### Literature Cited

- Ecological Analysts, Inc. 1979. Portsmouth power station, aquatic monitoring studies. Final Report for Virginia Electric and Power Company, Richmond, Virginia 23261. EA Report VEP83.
- Olney, J. E. 1978. Planktonic fish eggs and larvae of the lower Chesapeake Bay. Unpublished Master's Thesis. College of William and Mary. VIMS, Gloucester Point, Va. 23062. 124 pp.
- U. S. Army Engineer District, Norfolk. 1977. Final EIS Hampton Roads Energy Company's Portsmouth Refinery and Terminal, Portsmouth, Va.

MODEL AND PHYSICAL ENVIRONMENT

### A MODEL FOR DREDGE-INDUCED TURBIDITY

bу

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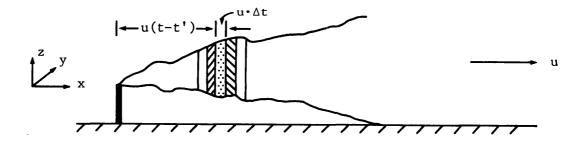
### A MODEL FOR THE DREDGE-INDUCED TURBIDITY

### I. Introduction

There are two major environmental concerns on the turbidity generated by a dredging operation. One is the temporary degradation of water quality by the turbidity plume. The other is the redeposition of dredge-induced turbidity in surrounding area, thus inflicting a negative impact on the habitat of benthic organisms. The model described in the following was formulated as a tool for quantitative estimate of these impacts.

### II. Theoretical Derivation

The model is constructed based on the concept of 'spreading-disk' diffusion model, proposed by Frenkiel (1953) to describe a plume from a continuous point source in uniform wind field. The diffusion in the wind direction is neglected by comparison with the advection by mean wind.



As shown in the sketch, the plume is considered as a series of thin slices of disks one after the other. Each of the disks consists of the material emitted from the source over a short duration of time from t' to t'+ $\Delta$ t'. The disk is

convected to the position x = u(t-t'), and has been spread in the y and z direction by diffusion during the time interval (t-t').

The diffusion of sediment particles in y and z directions may be described by the equation

$$\frac{\partial C}{\partial t} - W \frac{\partial C}{\partial z} = k_{Y} \frac{\partial^{2} C}{\partial y^{2}} + k_{z} \frac{\partial^{2} C}{\partial z^{2}}$$
 (1)

where

C is concentration,

t is time,

z is the coordinate in vertical direction,

y is the coordinate in transverse direction,

W is particle settling velocity,

 $\boldsymbol{k}_{\boldsymbol{y}}$  and  $\boldsymbol{k}_{\boldsymbol{z}}$  are diffusion coefficients in the y and z direction respectively.

The solution of equation (1) for an instantaneous line source along x-axis is

$$C = \frac{q}{4\pi\sqrt{k_{y}k_{z}}(t-t')} \exp \left[-\frac{(y-y')^{2}}{4k_{y}(t-t')} - \frac{\{z-z'+W(t-t')\}^{2}}{4k_{z}(t-t')}\right]$$
(2)

where

q is the source in mass/length/time,
y' and z' is the location of line source,
t' is the time when the material is released.

Equation (2) may be applied to the case of a continuous point source in a uniform flow field with velocity u in x-direction. In this case, the strength of line source

q becomes Q/u, where Q is the source per unit time, and

$$t - t' = \frac{x}{u}$$

Equation (2) becomes

$$C(x,y,z) = \frac{Q}{4\pi\sqrt{k_{y}k_{z}}x} \exp\left\{-\frac{(y-y')^{2}}{4k_{y}\frac{x}{u}}\right\}$$

$$-\frac{(z-z' + W\frac{x}{u})^{2}}{4k_{z}\frac{x}{u}}$$
(3)

### III. Application to Hydraulic Dredge

A. Suspended Sediment Concentration in the Turbidity Plume

The turbidity plume induced by hydraulic dredge may be considered as the result of a point source moving back and forth on the river bottom in the y-direction. Applying equation (3), the concentration field may be described as

$$C(x,y,z) = \frac{Q}{4\pi\sqrt{k_y k_z}x} \exp\left[-\frac{(y-y')^2}{4 k_y \frac{x}{u}}\right]$$

$$-\frac{(z + \frac{W}{u}x)^2}{4 k_z \frac{x}{u}}$$
(4)

where x is the distance from dredge head along flow direction, z is the distance above the bottom. In this application of solution for advective diffusion equation (equation (3)), the boundary effect of the water surface is assumed negligible, because the particle settling tends to keep them away from surface layer.

Since the dredge head moves back and forth in y-direction, y' is an implicit function of time, with  $\frac{-B}{2} \le y' \le \frac{B}{2}, \text{ where B is the sweeping range.} \text{ At given distance x from dredge head and z above the bottom, the dredge-induced turbidity will have a maximum at <math>y = y'$ ,

$$C_{m}(x,z) = \frac{Q}{4\pi\sqrt{k_{y}k_{z}x}} \exp\left(-\frac{\left(z + \frac{W}{u}x\right)^{2}}{4k_{z}\frac{x}{u}}\right)$$
 (5)

To investigate how the sediment concentration in a turbidity plume decreases with the distance from dredge head,  $C_{\rm m}$  may be normalized with its value at a reference distance  $x_{\rm r}$ . Dividing equation (5) by  $C_{\rm m}(x_{\rm r},z)$  and setting  $z=z_{\rm o}$ , a given height above river bottom, it is obtained that

$$\frac{C_{m}(x,z_{0})}{C_{m}(x_{r},z_{0})} = \frac{1}{x} \exp \left\{-\frac{1}{4} \left\{ \left(\frac{z_{0}^{2}}{k_{z}}\right) \left(\frac{x_{r}}{u}\right) \left(\frac{1}{x} - 1\right) + \left(\frac{x_{r}}{u}\right) \left(\frac{k_{z}}{w^{2}}\right) (x-1) \right\} \right\}$$
(6)

where  $X = \frac{X}{x_r}$ 

Defining the dimensionless parameters

$$t_{d} = \frac{z_{o}^{2}}{k_{z}} / \frac{x_{r}}{u}, \text{ the ratio of the time required for a particle to diffuse a distance } z_{o} \text{ to the time of advection over a distance } x_{r}'$$

$$t_s = \frac{k_z}{W^2} / \frac{x_r}{u} = \left(\frac{z_o}{W} / \frac{x_r}{u}\right)^2 / t_d$$
, where  $\left(\frac{z_o}{W} / \frac{x_r}{u}\right)$ 

is the ratio of time required for a particle to settle a distance  $z_{\rm O}$  to advection time,

equation (8) may be written as

$$C_{m}^{*}(X) = \frac{1}{X} \exp \left[ -\frac{1}{4} \left\{ t_{d}(\frac{1}{X} - 1) + \frac{1}{t_{s}}(X - 1) \right\} \right]$$
 (7)

For a continuous dredging in a tidal estuary, a new plume is formed with each change in current direction, while the old plume is dispersed rapidly under the combined effects of diffusion and settling. The turbidity plume will have its maximum extent near slack tide when the current has been going in the same direction for the maximum possible time period. The reference location may be taken at the plume front, and  $\mathbf{x}_r$  equals to a tidal excursion, thus

$$x_r = uT$$
, or  $x_r/u = T$ 

where T is one half of tidal period and u is the current speed averaged over flood tide or ebb tide.

Figures 1 through 5 show the function  $C^*(X)$  plotted versus X for the parameter ranges encompassing typical values of coastal plain estuaries. Because the diffusion in current direction is assumed negligible, this model predicts that a turbidity plume is confined within  $X \le 1$ , and the sediment concentration is zero for X > 1. This assumption is usually valid in a tidal estuary where the advective current is much stronger than diffusion. In coastal seas where the advective currents are weak, some refinement of the model is required.

The figures show that  $C^*(X)$  becomes less sensitive to  $t_s$  as the value of  $t_s$  increases, and becomes practically

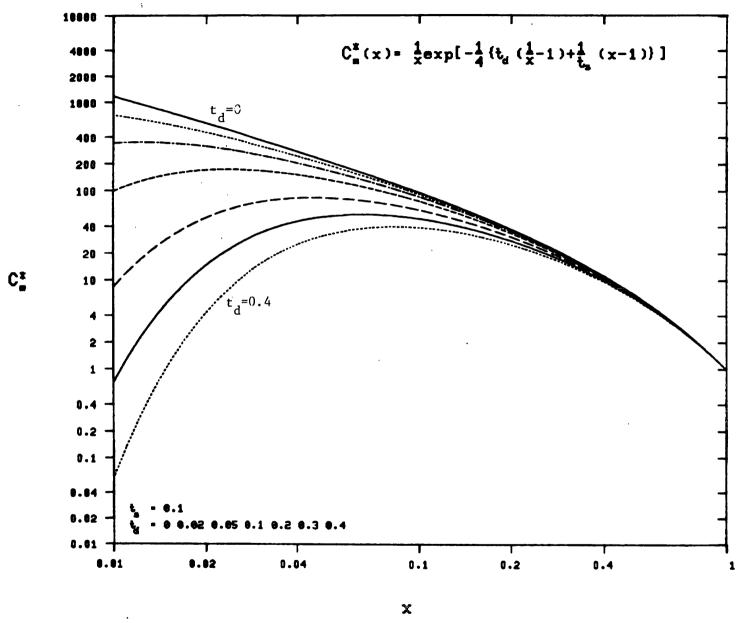


Figure 1. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, hydraulic dredge,  $t_s$ =0.1.

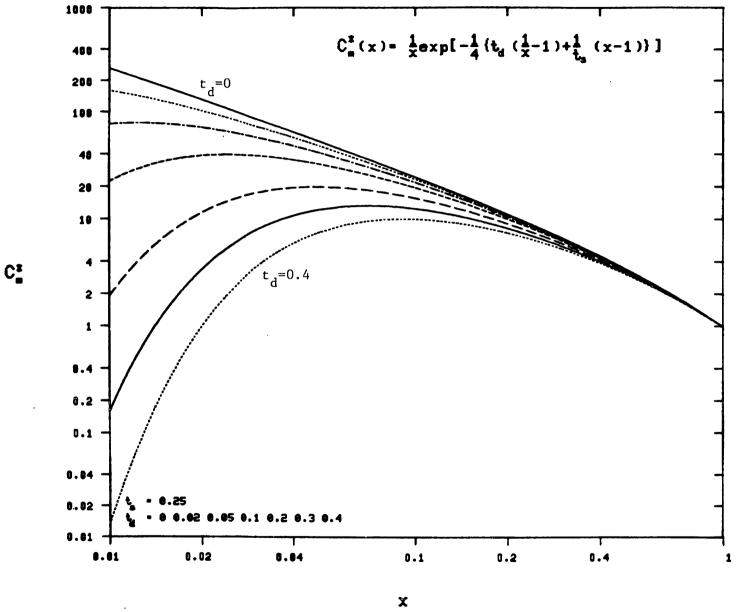


Figure 2. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, hydraulic dredge,  $t_s$ =0.25.

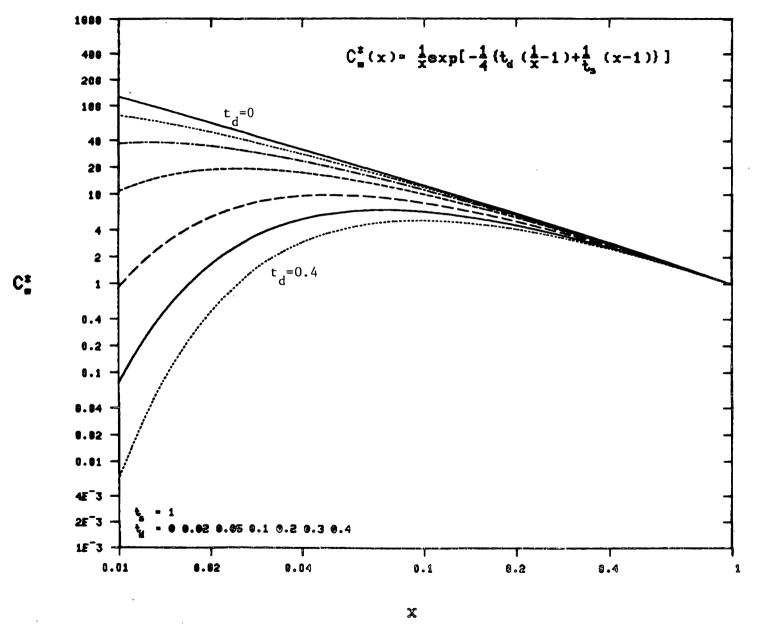


Figure 3. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, hydraulic dredge,  $t_s$ =1.0.

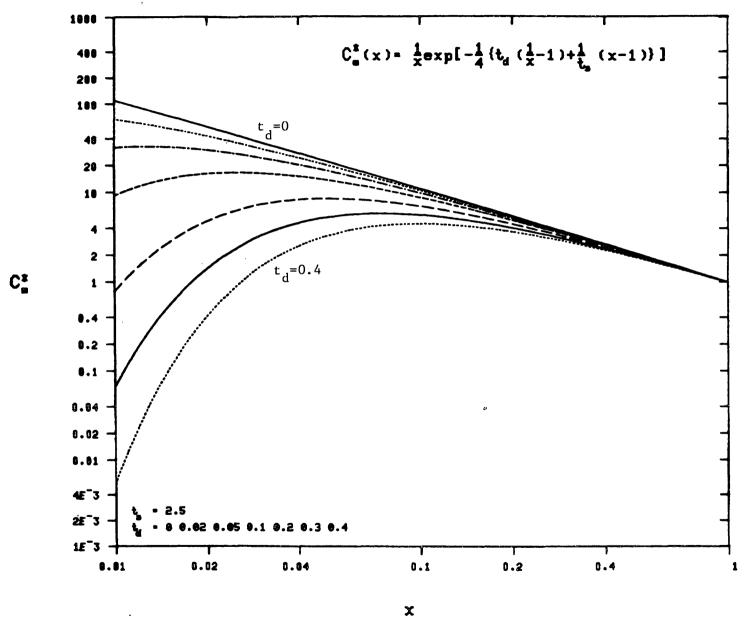


Figure 4. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, hydraulic dredge,  $t_s$ =2.5.

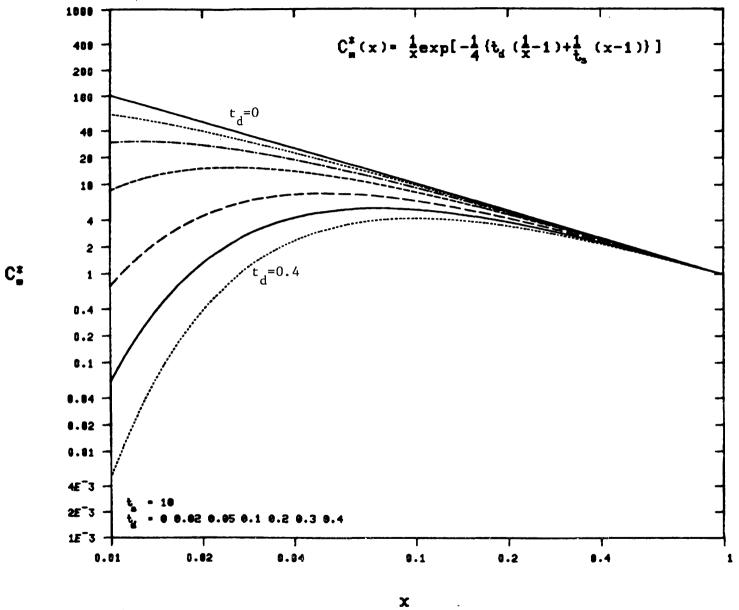


Figure 5. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, hydraulic dredge,  $t_s$ =10.0.

independent of  $t_s$  for  $t_s \ge 10$ . It is also seen that  $C^*(X)$  varies as 1/X for  $t_d = 0$  and large value of  $t_s$ .

## B. Sediment Deposition

The suspended solids in a turbidity plume will eventually redeposit on the bottom because of particle settling. If it is assumed that all particles deposit at the location where they strike the bottom, the deposition rate may be expressed as

$$D = WC \Big|_{z=z_1} + k_z \frac{\partial C}{\partial z} \Big|_{z=z_1}$$

where D is the sediment deposition rate in mass per unit area per unit time,  $C\big|_{z=z_1}$  and  $\frac{\partial C}{\partial z}\big|_{z=z_1}$  are the sediment concentration and concentration gradient at bottom respectively, and  $z_1$  is the bottom elevation. Substituting equation (4), it is obtained that

$$|WC|_{z=z_{1}} = \frac{|WQ|}{4\pi\sqrt{k_{y}k_{z}}} \exp \left[-\frac{(y-y')^{2}}{4k_{y}\frac{x}{u}} - \frac{(z_{1}+\frac{w}{u}x)^{2}}{4k_{z}\frac{x}{u}}\right]$$

for the deposition due to vertical settling, and

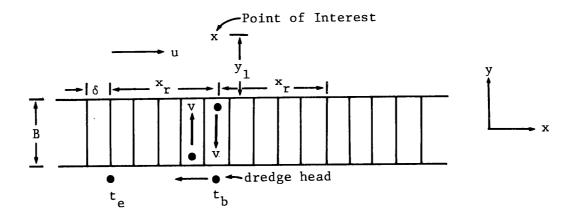
$$k_{z} \frac{\partial C}{\partial z} \mid_{z=z_{1}} = \frac{-WQ}{4\pi\sqrt{k_{y}k_{z}} x} \left(\frac{1}{2} + \frac{1}{2} \frac{u}{w} \frac{z_{1}}{x}\right) \cdot \exp \left[-\frac{(y - y')^{2}}{4k_{y} \frac{x}{u}} - \frac{(z_{1} + \frac{w}{u} x)^{2}}{4k_{z} \frac{x}{u}}\right]$$

for the deposition (or erosion) due to vertical diffusion.

The combination of the two mechanisms will give a negative deposition rate where  $\mathbf{z}_1 > \overset{\dot{W}}{\mathbf{u}} \mathbf{x}$ , which is impossible without net erosion from the bottom. For a conservative estimate of sediment deposition, the second term of upward particle diffusion is neglected and the net deposition rate is written as

$$D = \frac{WQ}{8\pi\sqrt{k_{y}k_{z}x}} \exp \left[ -\frac{(y-y')^{2}}{4 k_{y} \frac{x}{u}} - \frac{(z_{1} + \frac{W}{u}x)^{2}}{4 k_{z} \frac{x}{u}} \right]$$
(8)

To evaluate the total amount of deposition at a given location as the result of a dredging operation, the dredging operation is characterized as follows:



The previous sketch shows that a channel of width B is to be dredged along x-direction. The dredging operation may be considered as a series of swings by dredge head in y-direction. In each swing, the dredge head will move in y-direction with a speed V and cut a slice of thickness  $\delta$  in x-direction. Since the maximum extent of a dredge-induced turbidity plume is  $\mathbf{x_r}$ , only the dredging within a stripe of length  $2\mathbf{x_r}$  centered at the point of interest will contribute deposition to this location. The dredging to the left will contribute deposition when current is positive, while the dredging to the right will contribute when current is negative. To be conservative, assuming both halves of the dredging contribute deposition to the point of interest, then the total deposition per unit area is

$$M = 2 \int_{t_b}^{t_e} D dt$$
 (9)

where  $t_b$  and  $t_e$  are starting and ending time of dredging operation for the left half of the stripe. In case that it takes much more than one tidal period to complete dredging of stripe  $2x_r$ , the factor 2 in equation (9) may be dropped.

During each swing of dredge head, its position in y-direction may be written as

$$y' = -\frac{B}{2} + V(t - t_b - n\tau)$$
for  $t_b + n\tau \le t \le t_b + (n+1)\tau$  (10)

where

$$\tau = \frac{B}{V}$$

is the time required to complete one swing in y-direction, and n is a positive integer. The distance along x-direction between dredge head and the point of interest is

$$x = n\delta \tag{11}$$

The time integration in equation (9) may be substituted with the sum of a series of time integration over time period  $\tau$ , i.e.,

$$M = 2 \sum_{n=0}^{N-1} \int_{t_{b}^{+} n\tau}^{t_{b}^{+}(n+1)\tau} D dt$$
 (12)

where

$$N = \frac{x}{\delta}$$

is the number of swings required to complete dredging a distance  $\mathbf{x}_{r}$ .

Substituting equations (10) and (11) into equation (8), and substituting the results into equation (12), it is obtained that

$$M = 2 \sum_{n=0}^{N-1} \frac{WQ}{8\pi\sqrt{k_{y}k_{z}} n\delta} \exp \left[ -\frac{(z_{1} + \frac{W}{u} n\delta)^{2}}{4 k_{z} \frac{n\delta}{u}} \right] \cdot \int_{t_{b}^{+} n\tau}^{t_{b}^{+} (n+1)\tau} \exp \left[ -\frac{\{y + \frac{B}{2} - V(t - t_{b}^{-} n\tau)\}^{2}\}}{4 k_{y} \frac{n\delta}{u}} \right] dt \quad (13)$$

To simplify the process of estimating dredge-induced turbidity, Nakai (1978) introduced a concept of 'turbidity generation unit', which relates the turbidity to the volume

of dredged material. According to his definition, the suspended sediment source Q may be expressed as

$$Q = kGD\delta V \tag{14}$$

where D is the cutting depth of dredge head. The turbidity generation unit G stands for the quantity of turbidity generated when a unit volume of bed material is dredged under a standardized condition. The standardized condition was defined by the tidal current velocity at which sediment particles with diameters larger than  $74\mu$  are not resuspended. The size distribution factor is defined as

$$k = R_0/R_{74}$$

where  $R_{74}$  is the fraction of particles with a diameter smaller than  $74\mu$  and  $R_{_{\hbox{\scriptsize O}}}$  is the fraction of particles with a diameter smaller than the diameter of a particle whose critical resuspension velocity equals the current velocity in the field.

Substituting equation (14) into equation (13), and carrying out the integration, it is obtained that

$$M = 2 \sum_{n=0}^{N-1} \frac{kGD\delta W}{8\sqrt{\pi k_{z} n \delta u}} \exp \left[ -\frac{\left(z_{1} + \frac{W}{u} n \delta\right)^{2}}{4 k_{z} \frac{n \delta}{u}} \right]$$

$$\left[ \operatorname{erf} \left( \frac{y + \frac{B}{2}}{\sqrt{4 k_{y} \frac{n \delta}{u}}} \right) - \operatorname{erf} \left( \frac{y - \frac{B}{2}}{\sqrt{4 k_{y} \frac{n \delta}{u}}} \right) \right]$$

$$for |y| \ge \frac{B}{2}$$

$$(15)$$

where

erf (
$$\theta$$
) =  $\frac{2}{\sqrt{\pi}}$   $\int_0^{\theta} e^{-x^2} dx$ 

The equation may be written in terms of dimensionless parameters as

$$\frac{M}{kGD} = \frac{1}{4\sqrt{\pi t_s N}} \sum_{n=0}^{N-1} \frac{1}{\sqrt{n}} \exp\left\{-\left(\sqrt{\frac{N}{n}} t_B^2 Z\right)\right\}$$

$$+ \frac{1}{2} \sqrt{\frac{n}{n}} \frac{1}{t_s}^2 \left\{-\left(\sqrt{\frac{N}{n}} t_B^2 Z\right)\right\}$$

$$- \operatorname{erf} \left\{\sqrt{\frac{N}{n}} t_B^2 \left(y - \frac{1}{2}\right)\right\}$$
(16)

where

$$t_{B} = \frac{B^{2}}{4 k_{Y}} / \frac{x_{r}}{u}$$

$$Y = \frac{Y}{B}$$

$$z = \sqrt{\frac{k_{Y}}{k_{z}}} \frac{z_{1}}{B}$$

and  $t_s$  is defined in previous section.

Equation (16) is a very weak function of N for the practical range of N. A numerical test indicates that M/KGD varies no more than 0.3% for N ranging from 1000 to 4000. Therefore, for the results presented hereafter, N is taken to be 1000. The non-dimensional deposition rate is presented in graphical form from Figure 6 to 12.

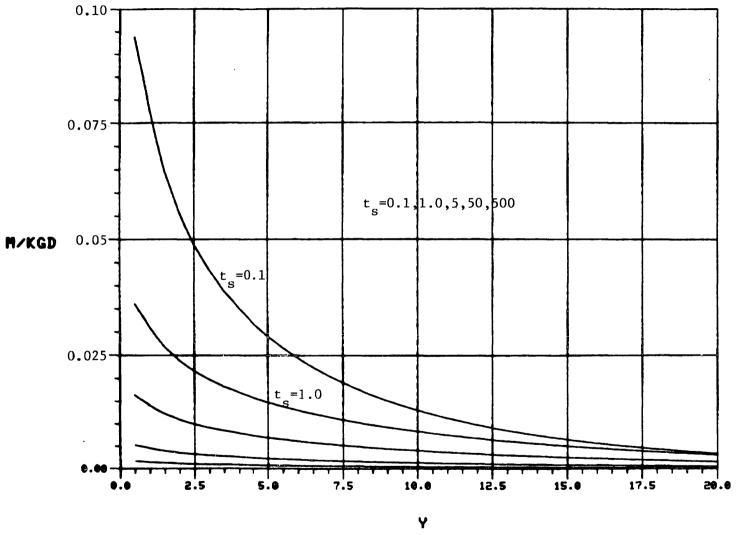


Figure 6. Dimensionless sediment deposition versus normalized lateral distance from dredge channel, hydraulic dredge,  $t_B^{-0.001}$ ,  $z_{-0.001}$ 

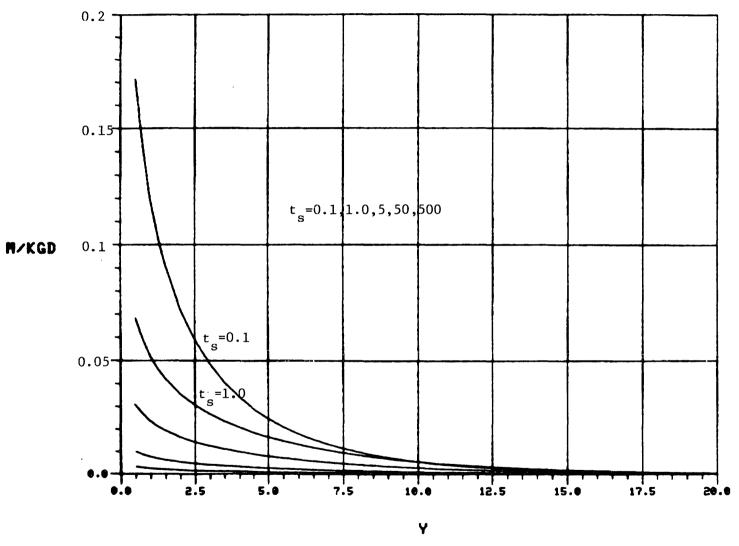


Figure 7. Dimensionless sediment deposition versus normalized lateral distance from dredge channel, hydraulic dredge,  $t_{\rm B}$ =0.005, Z=0.

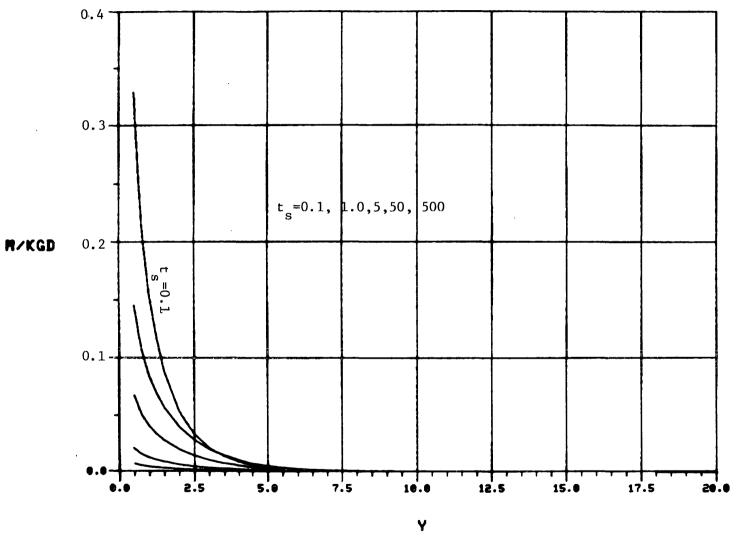


Figure 8. Dimensionless sediment deposition versus normalized lateral distance from dredge channel, hydraulic dredge,  $t_{\rm B}^{=0.05}$ , Z=0.

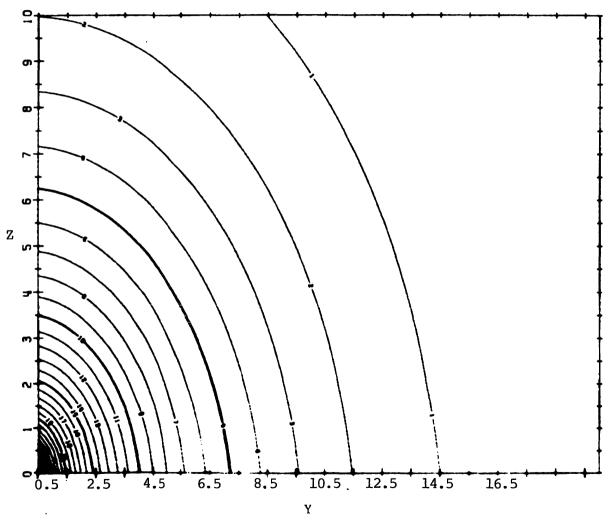


Figure 9a. Iso-deposition contours in Y-Z plane, hydraulic dredge, t = 5,  $t_{\rm B}$ =0.005. (The values of M/KGD should be multiplied by 0.001).

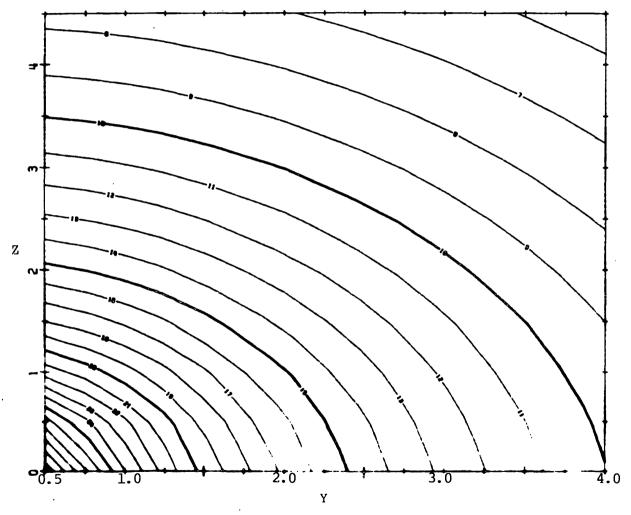


Figure 9b.

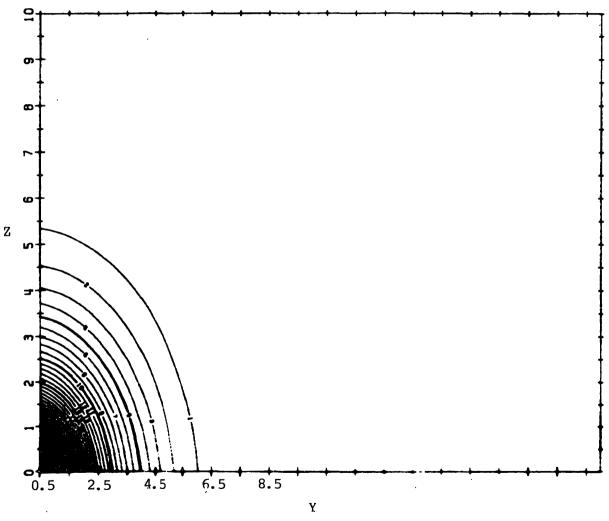


Figure 10a. Iso-deposition contours in Y-Z plane, hydraulic dredge,  $t_{\text{S}}\!=\!5$  ,  $t_{\text{B}}\!=\!0.05$  . (The values of M/KGD should be multiplied by 0.001).

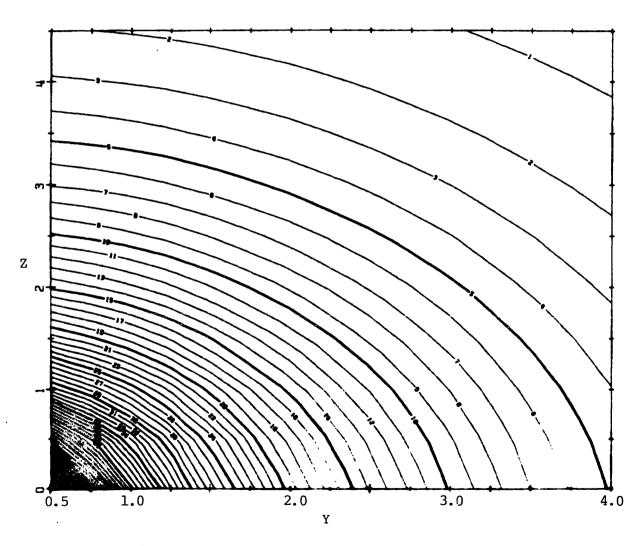


Figure 10b.

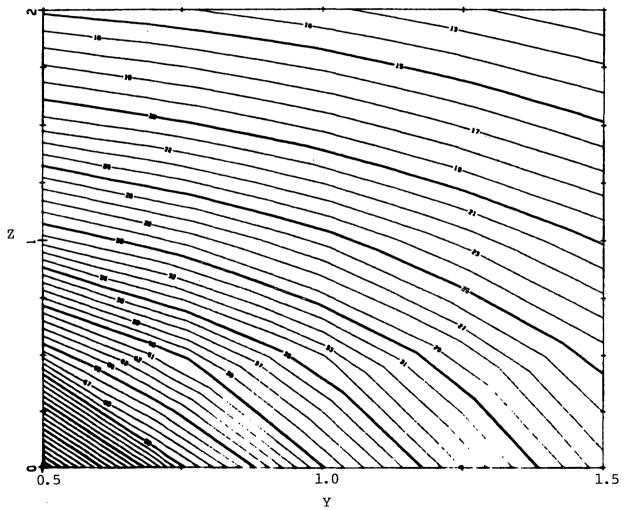


Figure 10c.

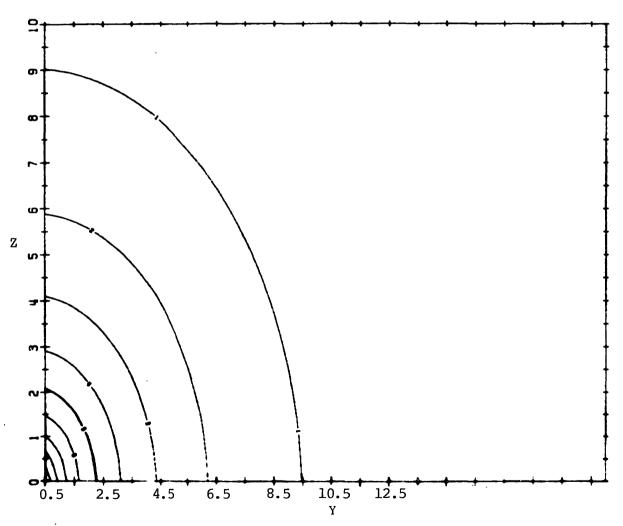


Figure 11. Iso-deposition contours in Y-Z plane, hydraulic dredge,  $t_s \!=\! 50,\, t_B \!=\! 0.005.$  (The values of M/KGD should be multiplied by 0.001).

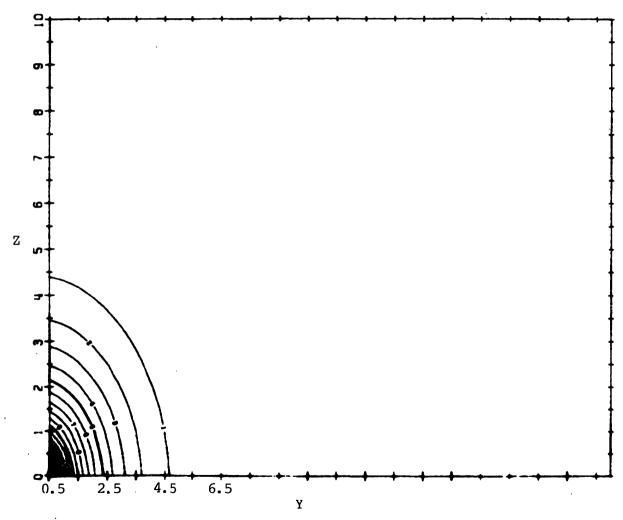


Figure 12a. Iso-deposition contours in Y-Z plane, hydraulic dredge,  $t_s \! = \! 50, \, t_B \! = \! 0.05.$  (The values of M/KGD should be multiplied by 0.001).

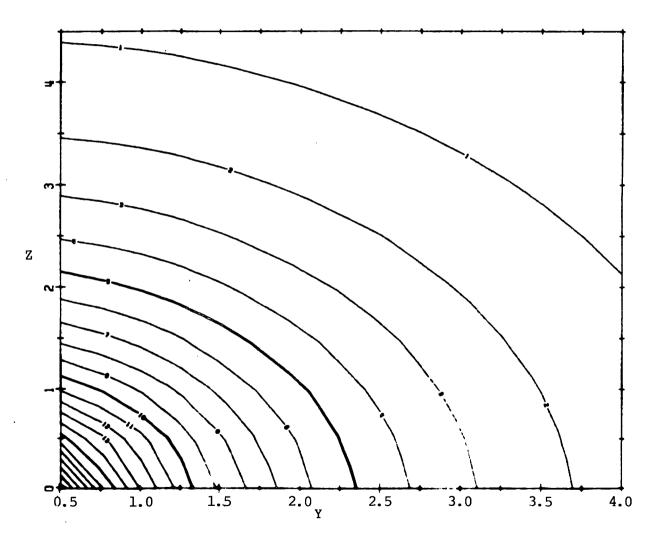


Figure 12b.

Figures 6 to 8 show the dimensionless deposition rate on the bottom of the same elevation as dredge channel (i.e. Z=0). Figures 9 to 12 present the equi-deposition contours in Y-Z plane. Some of the contour plots near the dredged channel are presented in enlarged form for clarity.

## IV. Application to Bucket Dredge

A. Suspended Sediment Concentration in the Turbidity Plume

The turbidity plume induced by a bucket dredge may be considered as the result of a line source stretching from water surface to bottom. The line source will move in y-direction, and then advance in x-direction as the dredge proceeds. To arrive at the concentration field, equation (3) is integrated with respect to z' from bottom to surface,

$$C(x,y,z) = \int_{0}^{h} \frac{Q}{h \cdot 4\pi \sqrt{k_{y}k_{z}} x} \exp \left[-\frac{(y-y')^{2}}{4k_{y} \frac{x}{u}}\right]$$

$$-\frac{(z-z' + W \frac{x}{u})^{2}}{4k_{z} \frac{x}{u}} dz'$$

$$= \frac{Q}{h\sqrt{4\pi k_{y} ux}} \exp \left[-\frac{(y-y')^{2}}{4k_{y} \frac{x}{u}}\right] \cdot \frac{1}{2} \left[\operatorname{erf}\left(\frac{z + W \frac{x}{u}}{\sqrt{4k_{z} \frac{x}{u}}}\right) - \operatorname{erf}\left(\frac{z-h + W \frac{x}{u}}{\sqrt{4k_{z} \frac{x}{u}}}\right)\right]$$

$$(17)^{*}$$

<sup>\*</sup>The error function in equation (17) should take a negative value when its argument is negative.

where h is the depth of water. At given distances x from dredge location and z above the bottom, the maximum turbidity occurs at y=y', therefore

$$C_{m}(x,z) = \frac{Q}{h\sqrt{4\pi} k_{y} ux} \cdot \frac{1}{2} \left( \text{erf} \left( \frac{z + W \frac{x}{u}}{\sqrt{4k_{z} \frac{x}{u}}} \right) - \text{erf} \left( \frac{z - h + W \frac{x}{u}}{\sqrt{4k_{z} \frac{x}{u}}} \right) \right)$$
(18)

 $C_m$  may be normalized with respect to the concentration at a reference distance  $x_r$ . Setting  $z=z_o$ , a given distance above the bottom, it is obtained that

$$C_{m}^{*}(X,Z) = \frac{C_{m}(x,z_{0})}{C_{m}(x_{r},z_{0})} = \frac{erf(\frac{1}{2}\sqrt{t_{h}}\frac{Z}{\sqrt{X}} + \frac{1}{2}\sqrt{\frac{X}{t_{s}}}) - erf(\frac{1}{2}\sqrt{t_{h}}\frac{Z-1}{\sqrt{X}} + \frac{1}{2}\sqrt{\frac{X}{t_{s}}})}{erf(\frac{1}{2}\sqrt{t_{h}}z + \frac{1}{2}\sqrt{\frac{1}{t_{s}}}) - erf(\frac{1}{2}\sqrt{t_{h}}(z-1) + \frac{1}{2}\sqrt{\frac{1}{t_{s}}})}$$

(19)

where

$$x = \frac{x}{x_r}, \quad z = \frac{z_o}{h}$$

$$t_h = \frac{h^2}{k_z} / \frac{x_r}{u}$$

Equation (19) is presented in graphical form in Figures 13 through 21 for the non-dimensional concentration

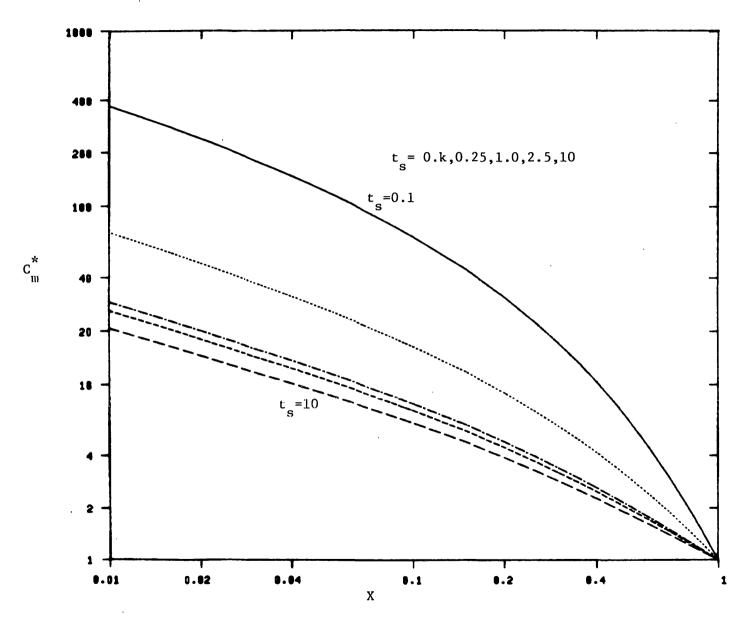


Figure 13. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=1.0, $t_h$ =1.0.

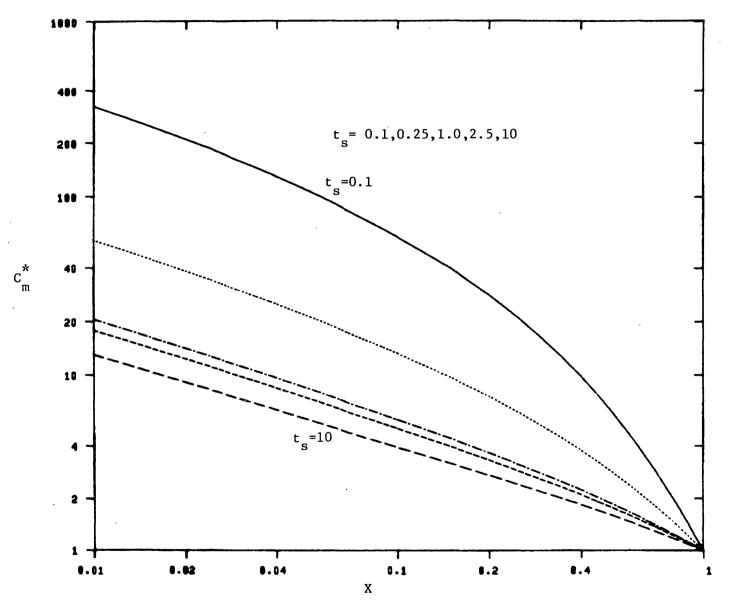


Figure 14. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=1.0, $t_h$ =5.0.

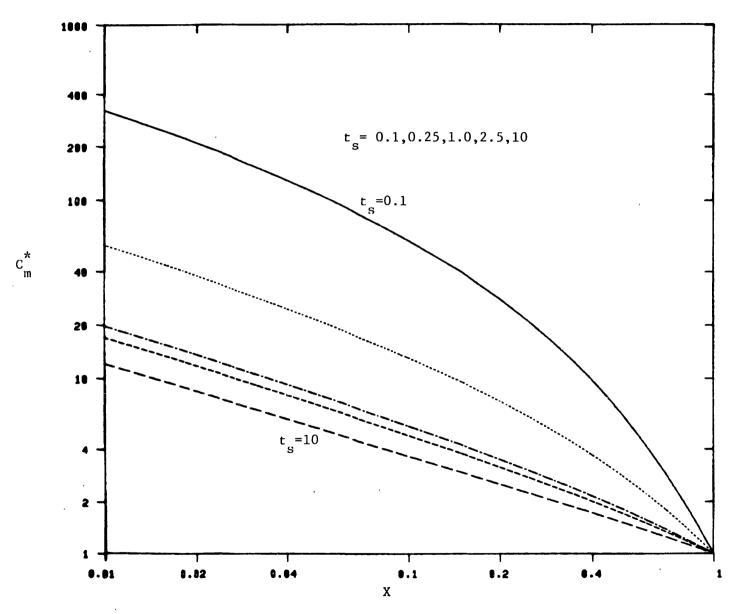


Figure 15. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=1.0,  $t_h$ =10.

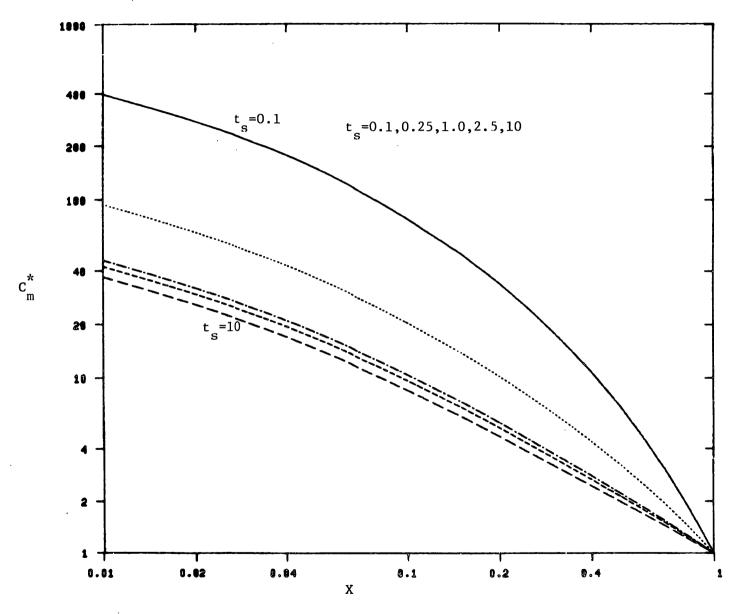


Figure 16. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=0.5, $t_h$ =1.0.

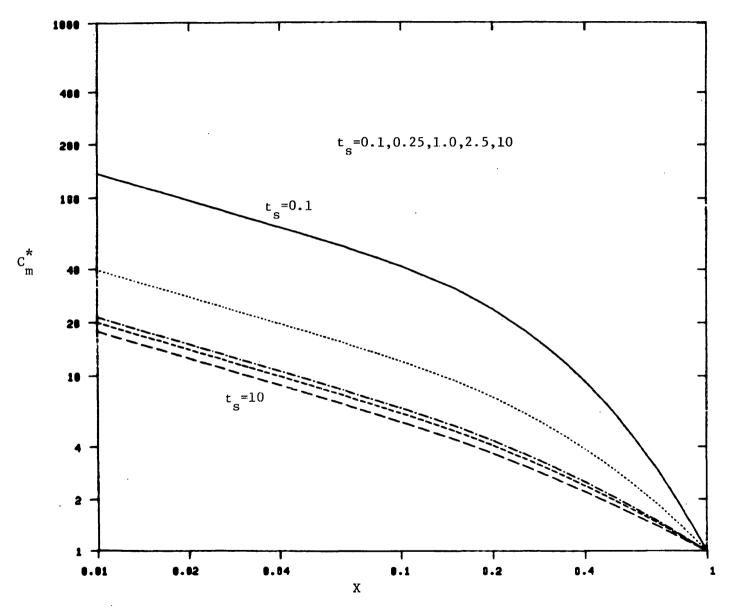


Figure 17. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=0.5, $t_h$ =5.0.

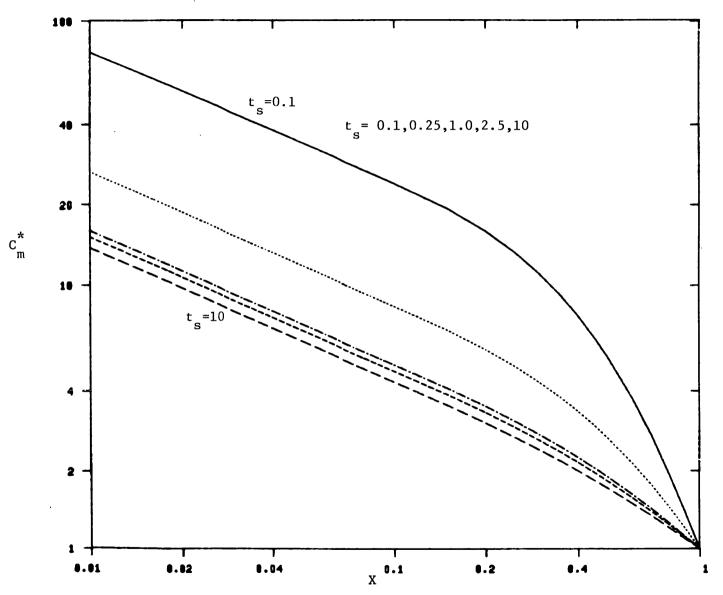


Figure 18. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=1.0, $t_h$ =10.

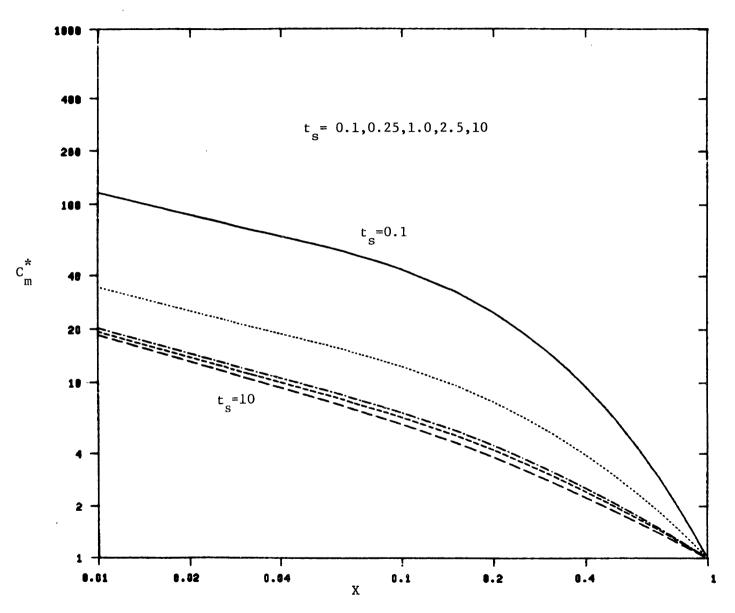


Figure 19. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=0, $t_h$ =1.0.

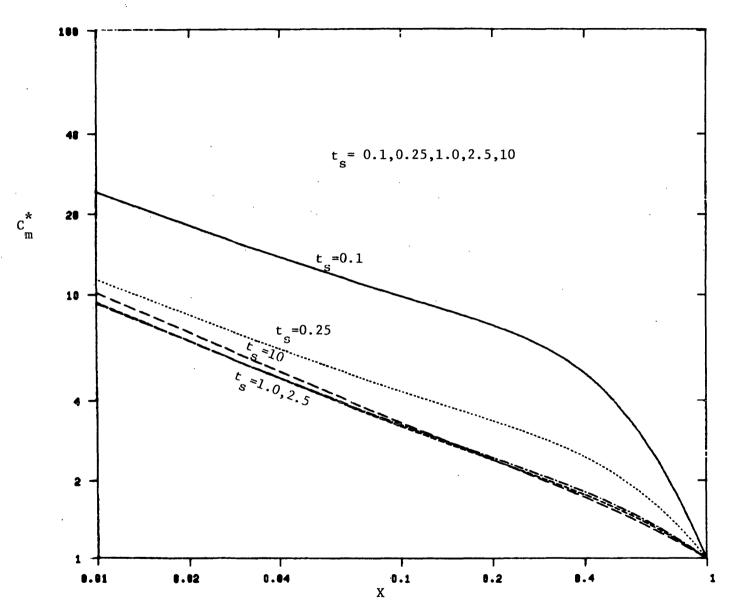


Figure 20. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=0,  $t_h$ =5.0.

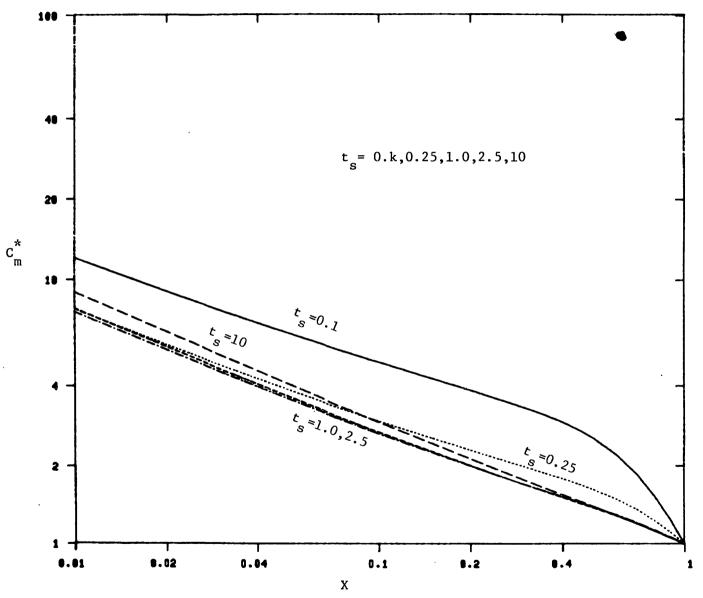


Figure 21. Normalized suspended sediment distribution versus normalized longitudinal distance from dredge head, bucket dredge, Z=0, $t_h$ =10.

distributions at surface (Z = 1.0), mid-depth (Z = 0.5) and bottom (Z = 0). It is to be noted that the sediment concentration is normalized with the concentration at the plume front at the corresponding depth. Therefore the distribution curves decrease more rapidly for the surface concentration.

## B. Sediment Deposition

As the case of hydraulic dredge, the sediment deposition rate may be expressed as

$$D = WC \Big|_{z=z_1} + k_z \frac{\partial C}{\partial z} \Big|_{z=z_1}$$

where  $\mathbf{z}_1$  is the bottom elevation. Substituting equation (17) and neglecting the upward diffusion, it is obtained that

$$D = \frac{WQ}{h\sqrt{4\pi k_{y} ux}} \exp \left(-\frac{(y-y^{*})^{2}}{4k_{y} \frac{x}{u}}\right)$$

$$\cdot \frac{1}{2} \left[\operatorname{erf}\left(\frac{z_{1} + W \frac{x}{u}}{\sqrt{4k_{z} \frac{x}{u}}}\right) - \operatorname{erf}\left(\frac{z_{1} - h + W \frac{x}{u}}{\sqrt{4k_{z} \frac{x}{u}}}\right)\right] \qquad (20)$$

Unlike the hydraulic dredge in which the point source moves continuously across the channel in y-direction, the bucket dredge generates a line source which moves discretely in y-direction. To facilitate mathematical derivation, the discrete motion is approximated by a continuous motion with

velocity V. Then, similar to the hydraulic dredge, the total sediment deposition at a given point may be written as

$$M = 2 \sum_{n=0}^{N-1} \frac{WQ}{h\sqrt{4\pi k_{y} \cdot u \cdot n\delta}} \cdot \int_{t_{b}+n\tau}^{t_{b}+(n+1)\tau} \exp \left[ -\frac{\left\{y + \frac{B}{2} - V(t - t_{b}-n\tau)\right\}^{2}}{4k_{y} \frac{n\delta}{u}} \right] dt \cdot \frac{1}{2} \left[ erf\left(\frac{z_{1} + W \frac{n\delta}{u}}{\sqrt{4k_{z} \frac{n\delta}{u}}}\right) - erf\left(\frac{z_{1} - h + W \frac{n\delta}{u}}{\sqrt{4k_{z} \frac{n\delta}{u}}}\right) \right]$$

$$(21)$$

Substituting turbidity generation unit and carrying out the integration, it is obtained that

$$M = \frac{1}{2} \sum_{n=0}^{N-1} \frac{kGD\delta W}{hu} \cdot \left[ erf\left(\frac{y + \frac{B}{2}}{\sqrt{4k_{y} \frac{n\delta}{u}}}\right) - erf\left(\frac{y - \frac{B}{2}}{\sqrt{4k_{y} \frac{n\delta}{u}}}\right) \right]$$

$$\cdot \left[ erf\left(\frac{z_{1} + W \frac{n\delta}{u}}{\sqrt{4k_{z} \frac{n\delta}{u}}}\right) - erf\left(\frac{z_{1} - h + W \frac{n\delta}{u}}{\sqrt{4k_{z} \frac{n\delta}{u}}}\right) \right]$$

$$(22)$$

or, in terms of dimensionless parameters,

$$\frac{M}{kGD} = \frac{1}{2N} \cdot \frac{1}{\sqrt{t_h t_s}} \sum_{n=0}^{N-1} \left\{ erf\left\{ \sqrt{\frac{N}{n}} t_B \left(Y + \frac{1}{2}\right) \right\} \right.$$

$$- erf\left\{ \sqrt{\frac{N}{n} t_B} \left(Y - \frac{1}{2}\right) \right\} \cdot \left\{ erf\left(\frac{1}{2} \sqrt{\frac{N}{n} t_h} z + \frac{1}{2} \sqrt{\frac{n}{N}} \frac{1}{t_s}\right) \right.$$

$$- erf\left\{ \frac{1}{2} \sqrt{\frac{N}{n} t_h} \left(z - 1\right) + \frac{1}{2} \sqrt{\frac{n}{N}} \frac{1}{t_s} \right\} \right] \tag{23}$$

where

$$Z = \frac{z_1}{h}$$
$$Y = \frac{Y}{R}$$

Equation (23) involves three independent parameters,  $t_s$ ,  $t_h$  and  $t_B$ . Using typical values of  $t_B$  and  $t_h$  for the dredging operation in the Elizabeth River, equation (23) is presented graphically in Figures 22 to 28. Equation (23) is a very weak function of N, and a numerical test shows that the value of M/kGD changes no more than 1.2% for N varies from 200 to 2000. For the results presented in Figures 22 to 28, the value of N is taken as 200. Figures 22, 23 and 24 show the amount of sediment deposition as function of distance from dredged channel, assuming the bottom is of the same elevation as the channel. Figures 25 to 28 present the equi-deposition contours on the Y-Z plane.

It is to be noted that the vertical diffusive flux of sediment particles is neglected in deriving equation (23). Therefore, the amount of sediment deposition as predicted by the equation is a conservative estimate.

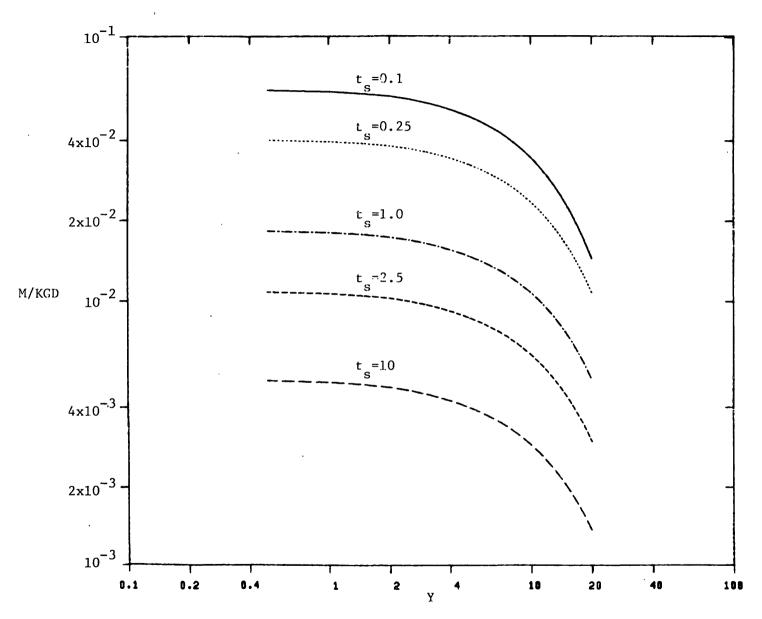


Figure 22. Dimensionless sediment deposition versus normalized lateral distance from dredge channel, bucket dredge,  $t_h$ =5.0,  $t_B$ =0.001, Z=0.

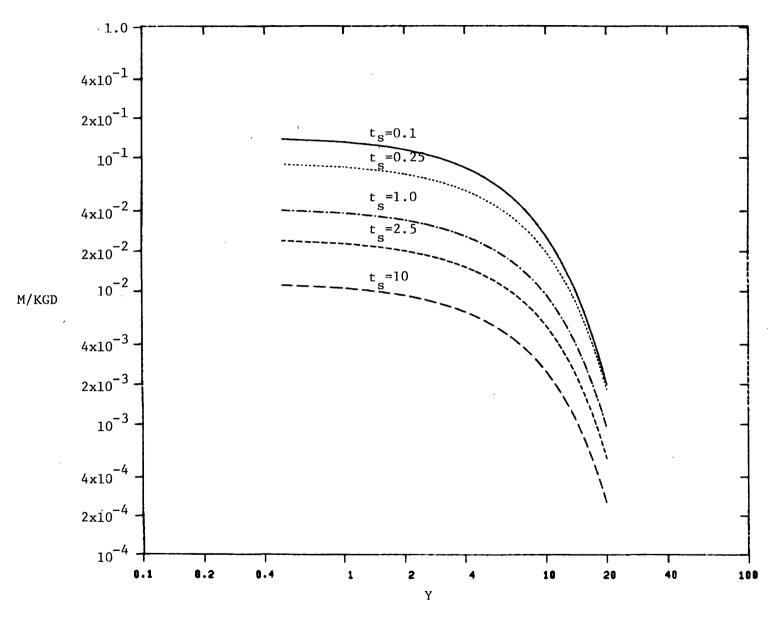


Figure 23. Dimensionless sediment deposition versus normalized lateral distance from dredge channel, bucket dredge,  $t_h$ =5.0,  $t_B$ =0.005, Z=0.

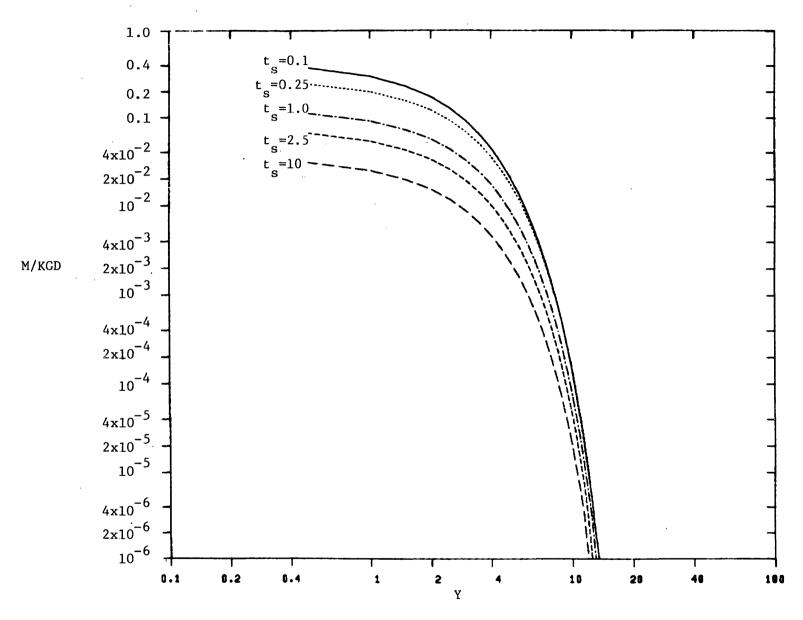


Figure 24. Dimensionless sediment deposition versus normalized lateral distance from dredge channel, bucket dredge,  $t_h$ =5.0,  $t_B$ =0.05, Z=0.

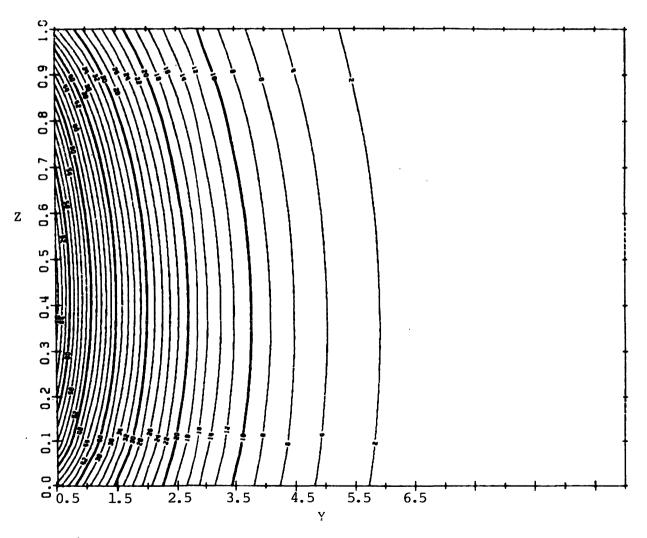


Figure 25a. Iso-deposition contours in Y=Z plane, bucket dredge,  $t_h$ =5.0,  $t_s$ =5.0,  $t_B$ = 0.05. (The values of M/KGD should be multiplied by 0.001).

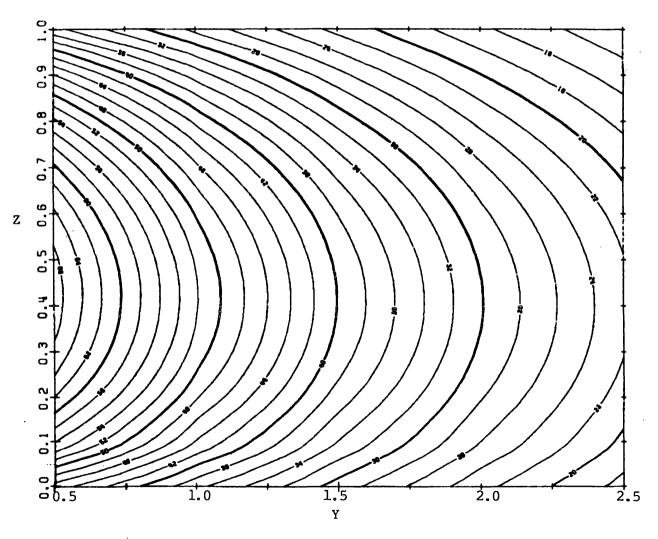


Figure 25b.

TH TS TB 5.0 0.005

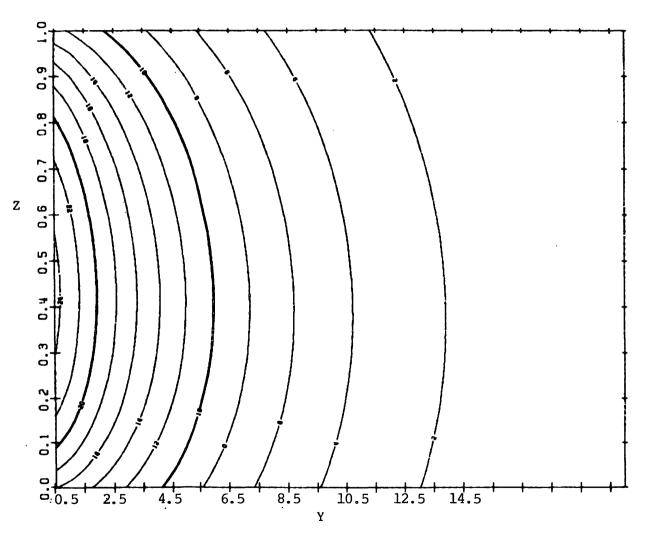
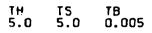


Figure 26a. Iso-deposition contours in Y-Z plane, bucket dredge,  $t_h$ =5.0,  $t_s$ =5.0,  $t_B$ =0.005. (The values of M/KGD should be multiplied by 0.001).



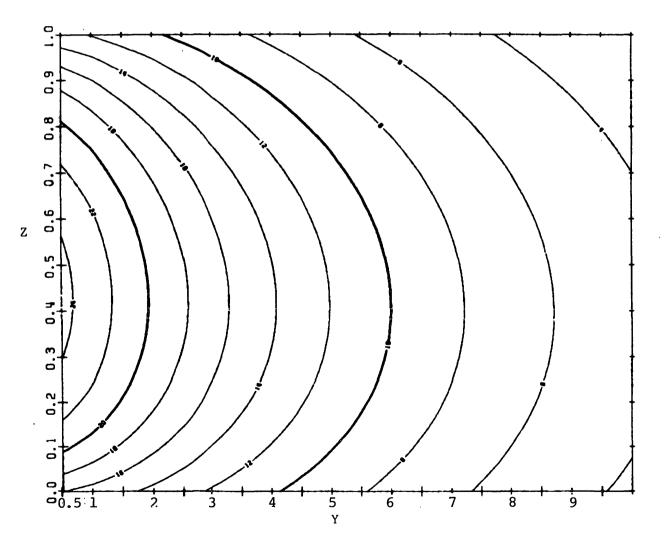


Figure 26b.

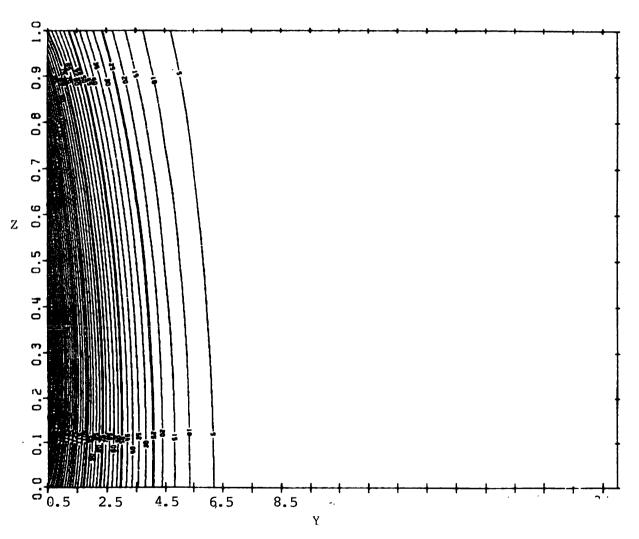


Figure 27a. Iso-deposition contours in Y-Z plane, bucket dredge,  $t_h$ =5.0,  $t_s$ = 0.5,  $t_B$ = 0.05. (The value of M/KGD should be multiplied by 0.001).

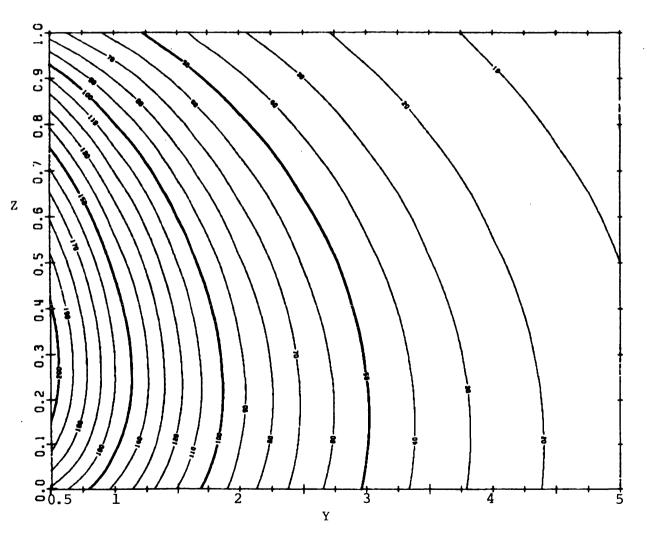


Figure 27b.

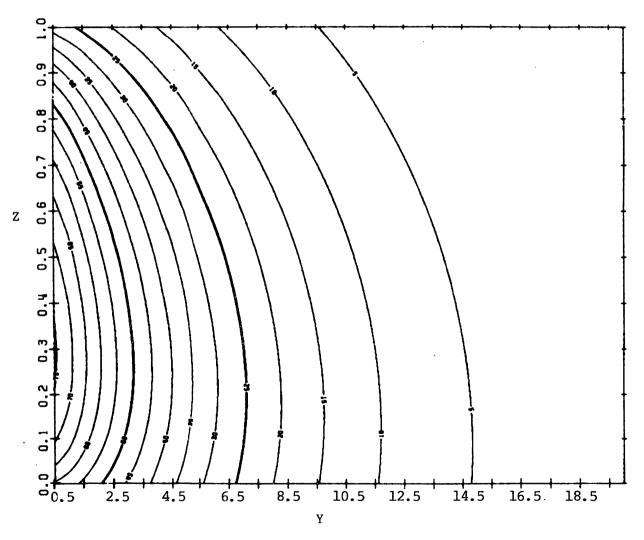


Figure 28. Iso-deposition contours in Y-Z plane, bucket dredge,  $t_h$ =5.0,  $t_s$ =0.5,  $t_B$ =0.005. (The values of M/KGD should be multiplied by 0.001).

#### References

- Frenkiel, F. N. 1953. "Turbulent Diffusion: Mean Concentration Distribution in a Flow Field of Homogeneous Turbulence". Advances in Applied Mechanics, Vol. 3, pp. 61-107.
- Kuo, A. Y. and J. P. Jacobson. 1976. "Prediction of Pollutant Distribution in Estuaries". Proceedings of the 15th Coastal Engineering Conference, pp. 3276-3293.
- Nakai, O. 1978. "Turbidity Generated by Dredging Projects." Proceedings of the 3rd U.S.-Japan Experts' Meeting on Management of Bottom Sediments Containing Toxic Substances.

# Appendix 1. Suspended Solid Concentrations at the Plume Front

In figures 1 to 5 and 13 to 21, the longitudinal distributions of suspended solid concentration are presented in dimensionless form normalized with the concentration at plume front. For practical application, the numerical values obtained from these figures need to be multiplied by the solid concentration at the plume front to arrive at the absolute concentrations. The concentrations at plume front may be evaluated with equations (5) and (18) for hydraulic dredge and bucket dredge respectively. Setting  $x = x_r$  and  $z = z_o$ , equation (5) becomes

$$C_{m}(x_{r}, z_{o}) / \frac{Q}{4\pi\sqrt{k_{v}k_{z}} x_{r}} = \exp\left(-\frac{1}{4}(\sqrt{t_{d}} + \sqrt{\frac{1}{t_{s}}})^{2}\right)$$
 (A1)

and equation (18) becomes

$$C_{m}(x_{r},z_{o}) / \frac{Q}{4h\sqrt{\pi k_{y}u x_{r}}} = erf\left(\frac{1}{2} / t_{h} z + \frac{1}{2} / \frac{1}{t_{s}}\right)$$

$$- erf\left(\frac{1}{2} / t_{h} (z-1) + \frac{1}{2} / \frac{1}{t_{s}}\right)$$
(A2)

Equations (A1) and (A2) are presented graphically in figures A1(a) to A4(a) with linear scales, and in figures A1(b) to A4(b) with logarithmic scales. Figures A1 are for hydraulic dredge, they show the variation of non-dimensional plume front concentration versus  $t_d$ , with  $t_s$  as a parameter. Figures A2 to A4 show the plume front concentrations at surface (Z=1), mid depth (Z=0.5) and bottom (Z=0) respectively for a plume induced by bucket dredge. They show the non-dimensional concentrations versus  $t_h$  with  $t_s$  as a parameter.

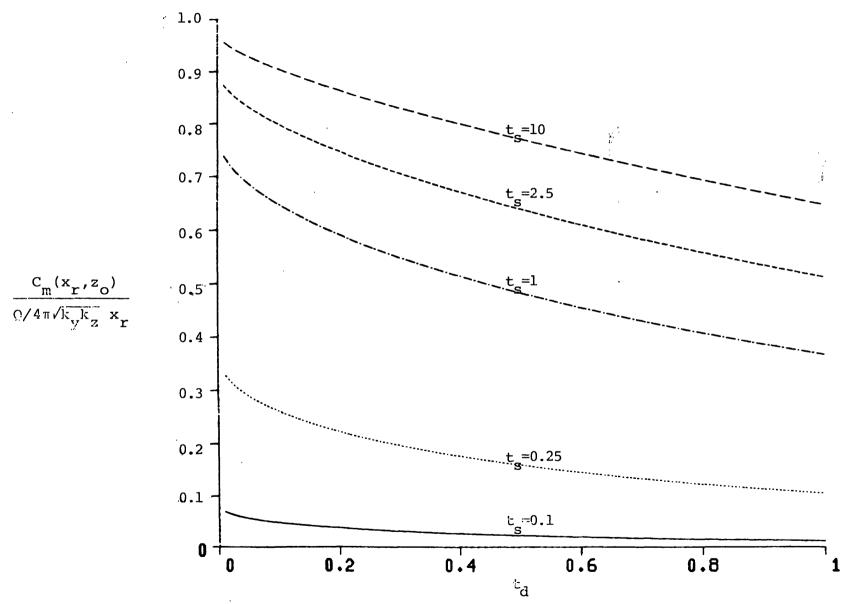


Figure Al(a). Mormalized suspended solid concentration at the front of a turbidity plume induced by hydraulic dredge, linear scale.

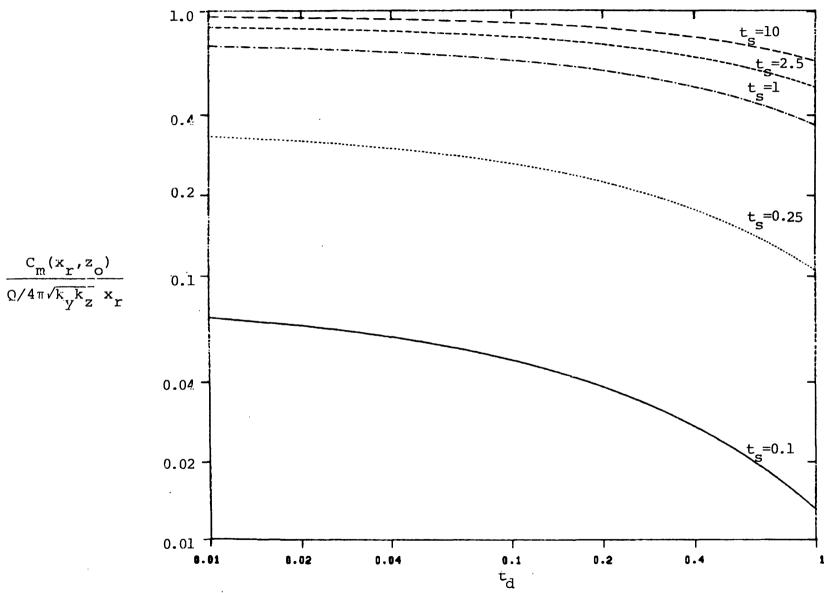


Figure Al(b). Normalized suspended solid concentration at the front of a turbidity plume induced by hydraulic dredge, logarithmic scale.

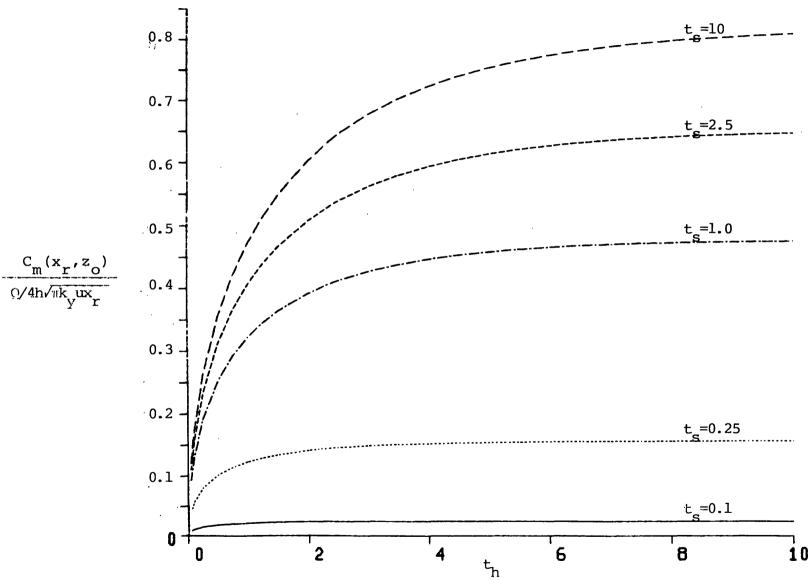


Figure A2(a). Normalized suspended solid concentration at the front of a turbidity plume induced by bucket dredge, Z=1.0, linear scale.

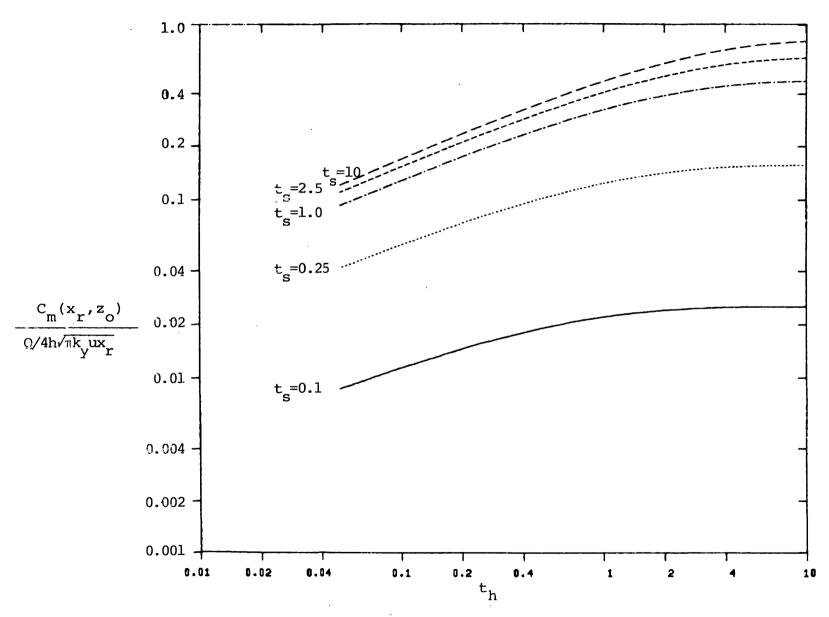


Figure A2(b). Normalized suspended solid concentration at the front of a turbidity plume induced by bucket dredge, Z=1.0, logarithmic scale.

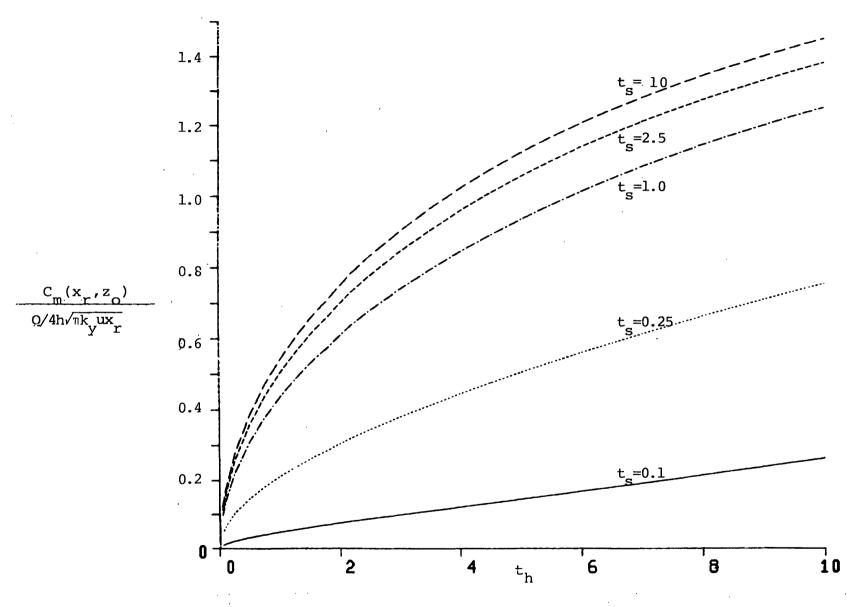


Figure A3(a). Normalized suspended solid concentration at the front of a turbidity plume induced by bucket dredge, Z=0.5, linear scale.

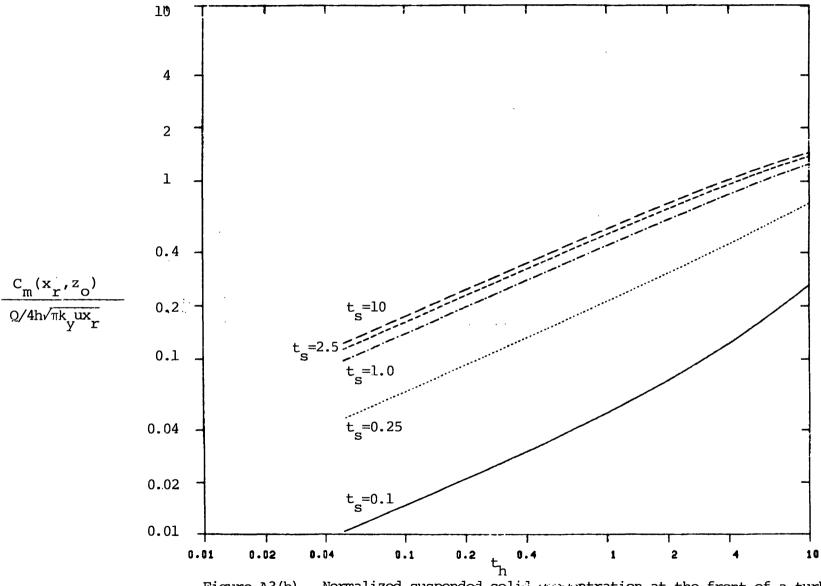


Figure A3(b). Normalized suspended solid concentration at the front of a turbidity plume induced by bucket dredge, Z=0.5, logarithmic scale.

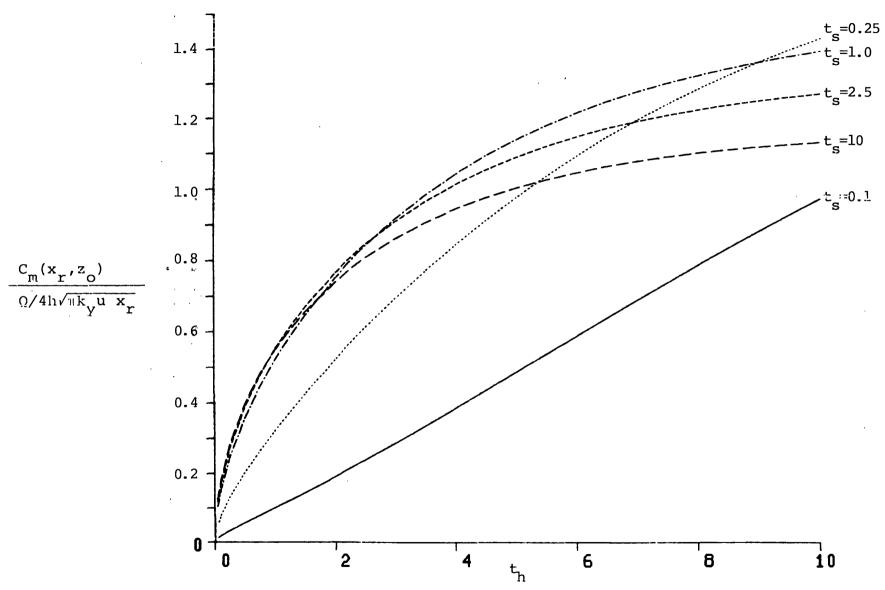


Figure A4(a). Normalized suspended solid concentration at the front of a turbidity plume induced by bucket dredge, Z=0, linear scale.

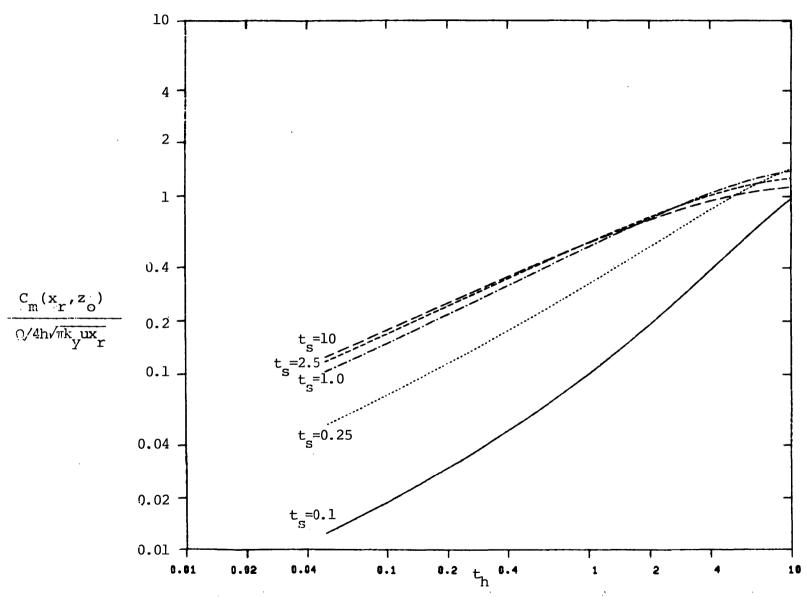


Figure A4(b). Normalized suspended solid concentration at the front of a turbidity plume induced by bucket dredge, Z=0, logarithmic scale.

### Appendix 2. Applications to Example Problems

Taking some typical dredging operations in the Virginia estuaries as examples, the following demonstrates how the model may be used to predict the dredge-induced turbidity and subsequent sediment deposition.

- I. Hydraulic Dredge in the Elizabeth River
- A. Input Information
  - (1) Specifications of dredging operation channel width

$$B = 200 \text{ ft} = 61 \text{ m.} = 6.1 \text{ x } 10^3 \text{ cm}$$
 dredging thickness

10 ft, to be completed in two steps, each step dredges 5 ft.

$$D = 5 ft = 1.52 m$$

swing speed of cutter head

$$V = 0.67 \text{ ft/sec} = 0.20 \text{ m/s}$$

 $\tau = 5$  minutes

cutter head cuts 6 ft in x-direction in each swing  $\delta$  = 6 ft = 1.83 m

(2) Characteristics of sediments at the channel bottom mean particle size

$$d = 6\mu = 6 \times 10^{-4} \text{ cm}$$

variance

$$s^2 = 70\mu^2 = 70 \times 10^{-8} \text{ cm}^2$$

the fraction of particles with diameter smaller than 74  $\mu$   $R_{74}$  > 99.99%

(3) Characteristics of ambient flow field mean velocity

$$u = 13 \text{ cm/sec}$$

period of flood or ebb

$$T = 2.24 \times 10^4 \text{ sec}$$

vertical turbulent diffusion coefficient

$$k_z = 10 \text{ cm}^2/\text{sec}$$

lateral turbulent diffusion coefficient  $k_y = 10^5 \text{cm}^2/\text{sec}$ 

- B. Information Sought
  - (1) The longitudinal distribution of suspended solid concentration in the turbidity plume at 1 meter above bottom.
  - (2) The amount of sediment deposition in the surrounding area.
- C. Calculation of Model Parameters
  - (1) settling velocity of sediment particles

$$W = 9 \times 10^{3} (d^{2} + s^{2}) \text{ in cgs unit}$$

$$= 9 \times 10^{3} (36 \times 10^{-8} + 70 \times 10^{-8})$$

$$= 10^{-2} \text{ cm/sec}$$

(2) particle size with critical resuspension velocity equals ambient velocity, 13 cm/sec

 $d_{c}$  = 276  $\mu$  (equation of Ingersol and equation of Camp et al.)\*

- (3) the fraction of particles with diameter smaller than  $d_{c}$   $R_{o} = 100\%$
- (4) the particle size distribution factor

$$k = R_0/R_{74} = 1.0$$

(5) the turbidity generation unit

$$G = 5.3 \sim 36.4 \text{ kg/m}^3$$
 for hydraulic dredge of silty clay material (Nakai, 1978) use  $G = 15 \text{ kg/m}^3$ 

(6) source strength of suspended solid

Q = kGD 
$$\delta$$
 V  
= 1.0 x 15 x 1.52 x 1.83 x 0.2  
= 8.34 kg/sec

\*From the data provided by Nakai (1978), the equations may be written as:

where  $V_{C}$  and  $d_{C}$  are in the units of cm/s and microns respectively.

(7) the maximum longitudinal extent of the dredge-induced plume

$$x_r = uT$$

$$= 0.13 \times 2.24 \times 10^4$$

$$= 2.91 \times 10^3 \text{ m}$$

$$z_r^2 / x z^2$$

(8) 
$$t_{d} = \frac{z_{o}^{2}}{k_{z}} / \frac{x_{r}}{u} = \frac{z_{o}^{2}}{k_{z}} / T$$
$$= \frac{100^{2}}{10} / 2.24 \times 10^{4}$$
$$= 0.045$$

(9) 
$$t_{s} = \frac{k_{z}}{W^{2}} / \frac{x_{r}}{u} = \frac{k_{z}}{W^{2}} / T$$
$$= \frac{10}{0.01^{2}} / 2.24 \times 10^{4}$$
$$= 4.5$$

(10) 
$$t_B = \frac{B^2}{4k_y} / \frac{x_r}{u} = \frac{B^2}{4k_y} / T$$

$$= \frac{(6.1 \times 10^3)^2}{4 \times 10^5} / 2.24 \times 10^4 = 4.15 \times 10^{-3}$$

# D. Application of the Model

(1) from figure Al(a) (or equation Al), with  $t_s = 4.5$ ,  $t_d = 0.045$ 

$$\frac{C_{\rm m}}{Q/4\pi\sqrt{k_{\rm y}k_{\rm z}}} = 0.9$$

... 
$$C_{m} (x_{r}, z_{o}) = 0.9 \cdot \frac{Q}{4\pi \sqrt{k_{x} k_{z}} \cdot x_{r}}$$

$$Q = 8.34 \times 10^3 \text{ gm/sec}$$

$$k_y = 10^5 \text{ cm}^2/\text{sec}$$

$$k_z = 10 \text{ cm}^2/\text{sec}$$
 $x_r = 2.91 \times 10^5 \text{ cm}$ 
 $C_m(x_r, z_0) = 2.05 \times 10^{-6} \text{ gm/cm}^3$ 
 $= 2.05 \text{ mg/l}$ 

(2) Since  $C_m(x,z_0)/C_m(x_r,z_0)$  is nearly independent of  $t_s$  for  $t_s \ge 2.5$ , use figure 4 (or equation (7)) for evaluating

$$C_{m}^{*} = C_{m}(x,z_{o})/C_{m}(x_{r},z_{o})$$
 $C_{m}(x,z_{o}) = 2.05 \cdot C_{m}^{*} \text{ mg/l}$ 
e.g.

(3) With  $t_B = 0.0042$  and  $t_S = 4.5$ , use figure 9 (or equation (16)) to calculate M/kGD.

$$kG \cdot (2D)^* = 1.0 \times 15 \times 305 = 4.58 \times 10^3 \text{ mg/cm}^2$$
  
= 4.58 gm/cm<sup>2</sup>  
e.g. Z = 0

Y y(m) M/kGD M(gm/cm<sup>2</sup>)  
1.0 61 m 30 x 
$$10^{-3}$$
 0.137  
5 305 9 x  $10^{-3}$  0.041  
10 610 m 3 x  $10^{-3}$  0.014

The factor 2 is introduced because the dredging operation required two cuts each with dredging thickness of 1.52 m.

- II. Maintenance Dredge in the Hampton Roads
- A. Input Information
  - (1) Specification of dredging operations channel width

 $B = 800 \text{ ft} = 244 \text{ m} = 2.44 \text{ x} 10^4 \text{cm}$  dredging depth

D = 5 ft = 1.52 m = 152 cm

(a) hydraulic dredge

swing speed of cutter head

V = 0.67 ft/sec = 0.2 m/sec

cutter head advance in each swing

 $\delta = 6 \text{ ft} = 1.83 \text{ m}$ 

(b) bucket dredge

bucket volume

$$\nabla = 3 \text{ m}^3$$

dredging frequency

f = 1/120 sec.

(2) Characteristics of sediments at channel bottom mean particle size

$$d = 6\mu = 6 \times 10^{-4} \text{ cm}$$

variance

$$s^2 = 70 \mu^2 = 70 \times 10^{-8} \text{ cm}^2$$

the fraction of particles with diameter smaller than  $74 \mu$ 

$$R_{74} > 0.999$$

(3) Characteristics of ambient flow field mean velocity

u = 40 cm/sec

period of flood or ebb

$$T = 2.24 \times 10^4 \text{ sec}$$

vertical turbulent diffusion coefficient

$$k_z = 10 \text{ cm}^2/\text{sec}$$

lateral turbulent diffusion coefficient

$$k_v = 10^5 \text{ cm}^2/\text{sec}$$

water depth

$$h = 45 \text{ ft} = 13.7 \text{ m} = 1.37 \text{ x} 10^3 \text{ cm}$$

- B. Calculation of Model Parameters
  - (1) Settling velocity of sediment particles  $W = 9 \times 10^{3} (d^{2} + S^{2}) \text{ in cgs unit.}$   $= 9 \times 10^{3} (36 \times 10^{-8} + 70 \times 10^{-8})$   $= 10^{-2} \text{ cm/sec}$
  - (2) Particle size with critical resuspension velocity equals ambient velocity, 40 cm/sec

$$d_c = V_c^2/0.783^2$$
 (eqn. of Camp et al.)  
=  $40^2/0.783^2 = 2.6 \times 10^3 \mu$ 

(3) The fraction of particles with diameter smaller than  $d_c$ .

$$R_{o} = 1.0$$

(4) The particle size distribution factor

$$k = R_0/R_{74} = 1.0$$

- (5) The turbidity generation unit
  - (a) hydraulic dredge

$$G = 30 \text{ kg/m}^3$$

(b) bucket dredge

$$G = 100 \text{ kg/m}^3$$

Note: the high values reported by Nakai (1979) are used for the sake of conservative assumption

- (6) Source strength of suspended solids
  - (a) hydraulic dredge

$$Q = kGD\delta V$$

= 
$$1.0 \times 30 \times 1.52 \times 1.83 \times 0.2$$
  
=  $16.7 \text{ kg/sec} = 1.67 \times 10^4 \text{ gm/sec}$ 

(b) bucket dredge

$$Q = kGVf = 1.0 \times 100 \times 3/120 = 2.5 \text{ kg/sec}$$

(7) The maximum longitudinal extent of the dredgeinduced plume

$$x_r = uT$$
  
= 0.4 x 2.24 x 10<sup>4</sup>  
= 8.96 x 10<sup>3</sup> m

(8)

(a) hydraulic dredge

$$t_d = \frac{z_o^2}{k_z} / T$$

$$= \frac{100^2}{10} / 2.24 \times 10^4 \quad \text{if } z_o = 1 \text{ m}$$

$$= 0.045$$

(b) bucket dredge

$$t_{h} = \frac{h^{2}}{k_{z}} / T$$

$$= \frac{(1.37 \times 100)^{2}}{10} / 2.24 \times 10^{4}$$

$$= 8.45$$

(9) 
$$t_s = \frac{k_z}{W^2} / T$$

$$= \frac{10}{0.01^2} / 2.24 \times 10^4$$

$$= 4.5$$

(10) 
$$t_B = \frac{B^2}{4k_y} / T$$
  
=  $\frac{(2.44 \times 10^4)^2}{4 \times 10^5} / 2.24 \times 10^4$   
=  $6.65 \times 10^{-2}$ 

- C. Application of the Model for Hydraulic Dredge
  - (1) Calculate concentration at plume front. From figure Al(b) (or equation Al), with  $t_s = 4.5$ ,  $t_d = 0.045$  (i.e. 1 meter above bottom,  $z_o = 1$  m)

$$\frac{C_{m}}{Q/4\pi\sqrt{k_{y}k_{z}}} = 0.9$$

$$\cdot \cdot C_{m}(x_{r},z_{o}) = 0.9 \cdot \frac{Q}{4\pi\sqrt{k_{y}k_{z}}} \cdot C_{m}(x_{r},z_{$$

(2) Calculate near bottom ( $z_{O} = lm$ ) concentration along plume axis as function of distance from the dredge. Since  $C_{m}(x,z_{O})/C_{m}(x_{r},z_{O})$  is nearly independent of  $t_{S}$  for  $t_{S} \geq 2.5$ , use figure 4 (or equation (7)) for evaluating  $C_{m}^{*}$ 

$$C_{m}^{*} = C_{m}(x,z_{o})/C_{m}(x_{r},z_{o})$$

$$\cdot \cdot \cdot C_{m}(x,z_{o}) = C_{m}^{*} \cdot C_{m}(x_{r},z_{o})$$

$$= 1.33 C_{m}^{*}$$

e.g.

(3) With  $t_B$  = 0.0665 and  $t_S$  = 4.5, use figure 10 (or equation (16)) to calculate M/kGD

$$kGD = 1.0 \times 30 \times 152 = 4.56 \times 10^3 \text{ mg/cm}^2$$
  
= 4.56 gm/cm<sup>2</sup>

e.g.

Y	y (m)	M/kGD	M(gm/cm <sup>2</sup> )
0.5	122	$67 \times 10^{-3}$ $40 \times 10^{-3}$	0.31
1.0	244	$40 \times 10^{-3}$	0.18
5.0	1220	$3 \times 10^{-3}$	0.014

- D. Application of the Model for Bucket Dredge
  - (1) Calculate surface concentration at the plume front

$$z_0 = 13.7 \text{ m}$$
 $z = 1.0$ 

from figure A2 (or equation A2), with

$$t_s = 4.5, t_h = 8.45$$

$$\frac{C_{\rm m}(x_{\rm r},z_{\rm o})}{Q/4h\sqrt{\pi k_{\rm y}} u x_{\rm r}} = 0.75$$

with

Q = 2.5 x 
$$10^3$$
 gm/sec  
h = 1.37 x  $10^3$  cm  
k<sub>y</sub> =  $10^5$  cm<sup>2</sup>/sec  
u = 40 cm/sec  
x<sub>r</sub> = 8.96 x  $10^5$  cm  
C<sub>m</sub>(x<sub>r</sub>,z<sub>o</sub>) = 1.02 x  $10^{-7}$  gm/cm<sup>3</sup> = 0.102 mg/1

(2) Calculate surface concentration along plume axis as function of distance from the dredge. With  $t_s = 4.5$ ,  $t_h = 8.45$ , z = 1.0. figure 17 (or eqn. (19)) is used to evaluate  $c_m^*$ .

$$C_{m}^{*} = C_{m}(x,z_{o})/C_{m}(x_{r},z_{o})$$

$$C_{m}(x,z_{o}) = C_{m}^{*} \cdot C_{m}(x_{r},z_{o})$$
  
= 0.102  $C_{m}^{*}$ 

e.g.

X	<b>x</b> (m)	c <sub>m</sub> *	C <sub>m</sub> (mg/l)
0.01	89.6	15	1.52
0.1	896	5	0.51
0.5	4480	1.3	0.13

(3) With  $t_B = 0.0665$ ,  $t_S = 4.5$ ,  $t_h = 8.45$ , use figures 26,27 (or eqn. (23)) to calculate M/kGD.

$$kGD = 1.0 \times 100 \times 152 = 1.52 \times 10^4 \text{ mg/cm}^2$$
  
= 15.2 gm/cm<sup>2</sup>

e.g. Z = 0

Y	y (m)	M/kGD	M(gm/cm <sup>2</sup> )
0.5	122	0.045	0.68
1.0	244	0.037	0.56
5.0	1220	0.004	0.061

- III. Bucket Dredge in Small Creek
- A. Input Information
  - (1) Specification of dredging operations channel width

$$B = 50 \text{ ft} = 15.2 \text{ m}$$

dredging depth

$$D = 1 m$$

bucket volume

$$\nabla = 1 \text{ m}^3$$

dredging frequency

$$f = 1/60 \text{ sec.}$$

(2) Characteristics of sediments at channel bottom mean particle size

$$d = 6\mu = 6 \times 10^{-4} \text{ cm}$$

variance

$$s^2 = 70\mu^2 = 70 \times 10^{-8} \text{ cm}^2$$

the fraction of particles with diameter smaller than  $74\,\mu$ 

$$R_{74} > 0.999$$

(3) Characteristics of ambient flow field mean velocity

$$u = 5 \text{ cm/sec}$$

period of flood or ebb

$$T = 2.24 \times 10^4 \text{ sec}$$

vertical turbulent diffusion coefficient

$$k_z = 2 cm^2/sec$$

lateral turbulent diffusion coefficient

$$k_v = 10^4 \text{cm}^2/\text{sec}$$

water depth

$$h = 1 m = 100 cm$$

- B. Calculation of Model Parameters
  - (1) Settling velocity of sediment particles

$$W = 9 \times 10^3 (d^2 + s^2)$$
 in cgs unit  
=  $9 \times 10^3 (36 \times 10^{-8} + 70 \times 10^{-8})$   
= 0.01 cm/sec

(2) Particle size with critical resuspension velocity equals ambient velocity,  $V_C = 5$  cm/sec

$$d_{c} = (V_{c}/0.00128)^{\frac{1}{2}}$$
 (eqn. of Ingersol)  
=  $(5/0.00128)^{\frac{1}{2}}$   
= 62.5u

(3) The fraction of particles with diameter smaller than  $\mathbf{d}_{\mathbf{C}}$ 

$$R_{O} = 0.90$$

determined from particle size analysis of bottom

(4) The particle size distribution factor

$$k = R_0/R_{74}$$
$$= 0.90$$

(5) The turbidity generation unit  $G = 100 \text{ km/m}^3$ 

Note: The high value reported by Nakai (1979) is used for the sake of conservative assumption

(6) Source strength of suspended solids

$$Q = kG\nabla f$$
  
= 0.90 x 100 x 1 x 1/60 = 1.5 kg/sec

(7) The maximum longitudinal extent of the dredge-induced plume

$$x_r = uT$$
  
= 5 x 2.24 x 10<sup>4</sup>  
= 1.12 x 10<sup>5</sup> cm = 1120 m

(8)  

$$t_{h} = \frac{h^{2}}{k_{z}} / T$$

$$= \frac{100^{2}}{2} / 2.24 \times 10^{4}$$

$$= 0.22$$

(9)  

$$t_{s} = \frac{k_{z}}{W^{2}} / T$$

$$= \frac{2}{(0.01)^{2}} / 2.24 \times 10^{4}$$

$$= 0.9$$

(10)  

$$t_{B} = \frac{B^{2}}{4k_{y}} / T$$

$$= \frac{(1520)^{2}}{4 \times 10^{4}} / 2.24 \times 10^{4}$$

$$= 0.0026$$

# C. Application of the Model

(1) Calculate surface concentration at the plume front

$$z_O = 100 \text{ cm}$$
 $Z = 1.0$ 

from figure A2(b) (or equation A2), with  $t_s = 0.9$ ,  $t_h = 0.22$ 

$$\frac{C_{\rm m}(x_{\rm r},z_{\rm o})}{Q/4h\sqrt{\pi k_{\rm v}}ux_{\rm r}} = 0.15$$

$$C_{m}(x_{r},z_{o}) = 0.15 \cdot \frac{Q}{4h\sqrt{\pi k_{y} u x_{r}}}$$

with

Q = 1.5 x 10<sup>3</sup> gm/sec  
h = 100 cm  

$$k_{y} = 10^{4}$$
 cm<sup>2</sup>/sec  
 $u = 5$  cm/sec  
 $x_{r} = 1.12 \times 10^{5}$  cm  
 $C_{m}(x_{r}, z_{0}) = 4.25 \times 10^{-6}$  gm/cm<sup>3</sup> = 4.25 mg/1

(2) Calculate surface concentration along plume axis as function of distance from the dredge. With  $t_s$  = 0.9,  $t_h$  = 0.22,  $^Z$  = 1.0, eqn. (19) is used to evaluate  $C_m^*$ 

$$C_{m}^{*} = C_{m}(x,z_{o})/C_{m}(x_{r},z_{o})$$

$$C_{m}^{*} = C_{m}(x,z_{o})/C_{m}(x_{r},z_{o})$$

$$C_{m}^{*} + C_{m}(x_{r},z_{o})$$

$$C_{m}^{*} + C_{m}(x_{r},z_{o})$$

$$C_{m}^{*} + C_{m}(x_{r},z_{o})$$

$$C_{m}^{*} + C_{m}(x_{r},z_{o})$$

e.g.

(3) With  $t_B = 0.0026$ ,  $t_S = 0.9$ ,  $t_h = 0.22$ , equation (23) is used to calculate M/kGD

$$kGD = 0.90 \times 100 \times 100$$
  
=  $9 \times 10^3 \text{ mg/cm}^2$   
=  $9 \text{ gm/cm}^2$ 

e.g. 
$$Z = 0$$

Y	У(ш)	M/kGD	M(gm/cm²)
0.5	7.6	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	0.27
5	76		0.16

Suspended Sediment Experiment and Model Calibration

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Suspended Sediment Experiment and Model Calibration

In order to examine the plume from a dredging operation both to calibrate the model and to characterize the plume from field data, an experiment was conducted in September 1978 in the Elizabeth River to measure the extent of the plume resulting from hydraulic maintenance dredging of a ship channel. This experiment used a fluorometer operated as a nephelometer sampling continuously at a depth of about 1 meter from the bottom, or at mid-depth. The channel is maintained at 50 ft., with the surrounding bottom about 20 feet. The fluorometer was towed through the plume in various patterns in order to obtain the plume shape. The tracks of the tows are shown in Appendix 1 as are the associated suspended sediment data. all cases, the tidal flow in the Elizabeth River was towards the north or the south. Also, the positions of the plume are all relative to the observed central position of the cutter head for the dredge, the source for the sediment plume. During the tests, the dredge was operating in the main channel of the reach opposite the Craney Island landfill area. The values of suspended sediment concentration are calculated from the measured light transmission by an empirical calibration from samples obtained during the data runs. These values are also shown in the appendix for the tracks. The set of runs encompasses most of the tidal cycle, from late flood through high slack, ebb, and low slack water. The early and full flood phases are not sampled, but in the Elizabeth River, they may be plausibly expected to be similar to their ebb counterparts.

The model of the plume presented elsewhere in this report (Kuo and Lukens, 1981) describes a nearly steady plume from a constant point source which is generated at the bottom of the channel and never reaches the surface. In actuality, the plume is generated by an oscillating and moving source, the cutter head of the dredge incising a notch with a cross-section of 30 ft<sup>2</sup> for a length of 200 feet in a period of 5 minutes. non-random currents and finite size of the cutter head are not well modeled by a point source model in some near field region, but this discrepancy is expected to be reduced rapidly outside of the immediate vicinity of the cutter head. The sweep produces a series of diagonal intermittent plumes rather than a steady state plume. The angle of the diagonal plume axis relative to the stream lines in the case studied was less than 45° except near slack tides, so the axial model is applicable except near the source at slack water. Because the model does not consider longitudinal dispersion, the intermittent nature of the actual plume is not a serious drawback to model application, although experimental data showing an absence or great reduction of the plume may be expected. Finally, the along-axis section made at mid-depth (25 feet, track 2 on 9/28/78) failed to detect any suspended sediment above the ambient level (20 mg/l). Thus from a qualitative standpoint, the model is generally applicable to the generated plumes provided that the intermittent nature near the endpoints of the swings are considered in the analyses of observations. The qualitative data which do not support

the model description are particularly high values of sediment found near the dredge head near slack tide (9/19/80 track 3) and isolated peaks within 400 feet of the source during high currents (9/7/78 track 7). These occurrences may serve to define a near field region of about 400 feet from the dredge head, particularly near slack water, where anomalously high values of sediment may be found within the sediment plume. Apart from these exceptions, the model seems qualitatively applicable to the data.

The calibration of the model starts from equation 7, repeated here for reference

$$C_{m}^{*}(X) = \frac{1}{X} \exp \left[ -\frac{1}{4} \left\{ t_{d} \left( \frac{1}{X} - 1 \right) + \frac{1}{t_{s}} (X - 1) \right\} \right]$$
 (7)

A new time scale,  $t_{\overline{W}}$ , is introduced as the ratio of the settling time to the horizontal advection time for the purpose of calibration.

$$t_{w} = \frac{z_{o}}{w} \cdot \frac{u}{x_{r}}$$

With this definition, we have

$$t_s = t_w^2 / t_d$$
, and equation (7) becomes
$$C_m^* (X) = \frac{1}{X} \exp \left[ -\frac{1}{4} t_d \left\{ \left( \frac{1}{X} - 1 \right) + \frac{1}{t_w^2} (X - 1) \right\} \right]$$
 (1)

In this form, the calibration task is seen to be the determination of estimates for  $t_w$  and  $t_d$  from measurements of concentration and distance from the source (X). To this end, it is convenient to transform equation (X.1) to the form

$$\frac{X}{(1-X)} \ln (X C_m^*) = \frac{1}{4} \frac{t_d}{t_w^2} \left( X - t_w^2 \right)$$
 (2)

In this form, the left hand side consists entirely of values which can be calculated from observations, and the right hand side has the form of a straight line with X intercept at  $X=t_w^2$ . The slope of the line is related to  $t_w$  and  $t_d$  in the same way that  $t_s$  is, and can be expressed as 1/4  $t_s$ . A calibration procedure which is suggested by this form is to transform the data into the left hand side, fit a straight line by regression to the points, and evaluate the parameters on the right hand side from the equation for the line.

Before this procedure can be followed, a further scaling is required because  $C_{\rm m}^{\phantom{\rm m}}$  is a ratio of observed excess sediment concentration to a reference value, chosen in the theory to be the value at the full extent of the plume. In practice, such a value can be observed only at slack tide, for the plume is fully developed only then. In addition, the excess value of sediment concentration at that location may be below the detection threshold. These two difficulties may be overcome by defining, for the purpose of calibration, a new advective distance scale  $x_{\rm p} = \alpha x_{\rm r}$  such that the level of suspended sediment at  $\mathbf{x}_{\mathrm{B}}$  is easily detectable. The corresponding nondimensional scale of distance is  $X' = x/x_{R}$ , and the corresponding derived parameters become  $t_d$ ' and  $t_w$ '. After these are determined, the unprimed values are evaluated as  $t_w = \alpha t_d$ . The equations used in evaluating the calibration data are presented in table 1.

For the particular calibration calculations, the tracks listed in the appendix were plotted on a common distance scale,

Table 1. Equations Used in Model Calibration and Interpretation for the Sediment Plume Study

Symbol	Units	General Formula	Formula Applied to Elizabeth River
ū	ft/min	$\frac{\Delta H/\Delta t}{\Delta H/\Delta t} \times \overline{u}_{m} \times 60$	$5.57 \times 10^3 \times \Delta H/\Delta t \times u_p$
xr	ft	$x_r = \overline{u} T$	$x_{r} = 5903 x R$
× <sub>B</sub>	ft	chosen from data	chosen from data
C <sub>.</sub> B	mg/liter	chosen from data	chosen from data
α	1	x <sub>B</sub> /x <sub>r</sub>	x <sub>B</sub> /x <sub>r</sub>
t <sub>w</sub> '	1	$t_{w}' = \sqrt{X_{o}}$	$t_{W}^{\prime} = \sqrt{X_{O}^{\prime}}$
t <sub>w</sub>	1	$t_{\mathbf{w}} = \alpha t_{\mathbf{w}}'$	$t_{\mathbf{W}} = \alpha t_{\mathbf{W}}'$
<sup>t</sup> d'	1	$t_d' = 4mX_o'$	$t_d = 4mX_o'$
<sup>t</sup> d	1	$t_d = \alpha t_d'$	$t_d = \alpha t_d'$
ts	1	$t_{s} = \frac{1}{4m}$	$t_s = \frac{1}{4m}$
W	cm/sec	$W = \frac{z_0}{t_w} - \frac{1}{60 \text{ T}}$	$W = \frac{1.67}{t_w T}$
c <sub>m</sub> *	1	$C_{\rm m}^{\star\prime} = \frac{C_{\rm m}}{C_{\rm B}}$	$C_{\rm m}^{\star i} = C_{\rm m}/C_{\rm B}$
k <sub>z</sub>	cm <sup>2</sup> /sec	$k_z = \frac{1}{60 \text{ T}} \frac{z_0^2}{t_d}$	$k_z = \frac{166.7}{t_d T}$
X '	1	x/x <sub>B</sub>	x/x <sub>B</sub>
a	microns	a = lll W <sup>1/2</sup> @ 20 <sup>O</sup> C for a sediment particle of specific gravity 2.	$a = 111 W^{\frac{1}{2}}$

# Table 1 (Cont'd)

Symbol	<u>Definitions</u>
T	Period, in minutes, of rising or falling tide during observations from tide gauge or tables.
R	Range, in feet, of tide rise or fall for tidal half cycle during which observations were taken from tide gauge or tables.
x	Distance of a given observation, in feet, from the source in the downstream direction.
<sup>u</sup> p	Peak speed in a given locality for mean tide as given by Cerco and Kuo (unpublished ms.).
ū	Mean speed over tidal phase during which observations were obtained.
$\overline{\mathbf{u}}_{\mathtt{m}}$	Mean speed over a mean tidal phase.
$c_{\rm m}$	A maximum measured value of sediment concentration in an approximately transverse section of the plume.
x' <sub>o</sub>	The horizontal intercept of the line fitted to the data.
m	The slope of the straight line fitted to the data.
<b>x</b> B	The base distance, in feet, chosen from the data to represent the advective extent of the easily detectable part of the plume.
$C_{\mathbf{B}}$	The sediment concentration, in mg/liter, inferred or measured at $x_B$ .
<sup>z</sup> o	Height, in meters, of the observations over the bottom.
a	Diameter of a representative sediment grain.
ΔН	Difference in height from high to low (or low to high) tide in feet.
Δt	Time span, in minutes, between successive tidal height extrema.

Table 2. Plume Axis Estimates for Calibration

Day	Track	Maximum Concentration	Background Value	Distance from Source	X'	c <sub>m</sub>	$\frac{X'}{1-X'}$ in $(X'C_m^*)$	C <sub>m</sub> *
9/7/78	1	54	27	260		27		
9/7/78	2	46	26	200		20		
9/7/78	3	103	(27)	50		76		
9/7/78	4	51	30	200		21		
9/7/78	5	30	25	200		5		
9/7/78	6	73	35	220	.22	38	246	1.90
9/7/78	7	90	27	320	.32	63	+.004	3.15
9/7/78	8	80	24	440	.44	56	+.164	2.80
9/7/78	8	69	24	460	.46	45	+.029	2.25
9/7/78	9	60	33	870	.87	27	+1.076	1.35
9/7/78	10	44	34	1130	1.13	10	+4.963	0.50
9/7/78	11	44	34	1230	1.23	10	+2.600	0.50
9/19/78	1	86	17	115	.575	69	1.41	4.93
9/19/78	1	73	17	160	.800	56	4.65	4.00
9/19/78	1	31	17	220	1.100	14	-1.05	1.00
9/19/78	2	114	38	110	.22	76	309	1.52
9/19/78	2	114	38	160	.32	76	339	1.52
9/19/78	2	121	38	360	.72	83	+.459	1.66
9/19/78	2	46	38	680	1.36	8	+5.76	0.16

Table 2 (Cont'd)

Day	Track	Maximum Concentration	Background Value	Distance from Source	X'	C <sub>m</sub>	$\frac{X'}{1-X'}$ ln $(X'C_m^*)$	c <sub>m</sub> *
9/19/78	3	181	24	60	.300	157	+0.520	11.21
no	3	49	24	130	.650	25	-3.50	1.79
parabola	3	38	24	200	1.00	14	-0.189	1.00
9/19/78	5	65	22	60	-	43		-
9/26/78	2(I)	40	9	230	.575	31	1.209	4.25
9/26/78	2(I)	22	9	360	.90	13	3.449	1.63
9/26/78	2(I)	18	9	390	.975	9	3.779	1.13
9/26/78	2(II)	40	9	230	.288	31	-0.417	1.24
9/26/78	2(II)	37	9	700	.875	26	-0.660	1.04
9/26/78	2(II)	32	9	880	1.100	23	-0.131	0.92
9/28/78	No cle	ar interpretati	on					

Table 3. Calibration Calculated Values

						I	II
Date			9/19/78	•	•		
Plume	1	2	1	2	3	1	1
Tidal Phase	late flood	full ebb	early ebb	late ebb	late ebb	low slack	low slack
T	384	375	379	379	379	365	365
R	2.8	-2.6	3.3	3.3	3.3	-2.0	-2.0
$\frac{\Delta H}{\Delta t}$	$7.3 \times 10^{-3}$	$-6.9 \times 10^{-3}$	$-8.7 \times 10^{-3}$	$-8.7 \times 10^{-3}$	$^{3}$ -8.7 x 10 <sup>-3</sup>	$-5.5 \times 10^{-3}$	$-5.5 \times 10^{-3}$
× <sub>r</sub>	16549	-15276	-19480	-19480	-19480	-11806	-11806
x <sub>B</sub>	Failed	-1000	200	500	200	-400	-800
C <sub>B</sub>		20	14	50	14	8	25
α		.065	.0103	.0257	.0103	.0339	.0678
x' <sub>o</sub>		.340	Failed	.355	Failed	.383	Failed
m		2.6-4.7		4.15		6.6	
t <sub>w</sub> '		.583		.596		.619	
t <sub>w</sub>		.038		.015		.021	
t <sub>d</sub> '		3.5-6.4		5.89		10.11	
<sup>t</sup> d		.230415		.151		.343	
t <sub>s</sub>		.0035006	3	.0015		.0013	
W		.117		.293		.217	138
a		38		60		52	
k		1.08-1.95		2.90		1.36	

based on linear interpolation between listed positions. Values of sediment concentrations for plume peaks and background were then obtained from these graphs. These values are given in table 2. The peak concentration values were plotted versus distance and an "eyeball" line was used to estimate the general shape of the plume. A value for the reference distance  $(\mathbf{x}_{\mathrm{B}})$  and concentration  $(\mathbf{C}_{\mathrm{B}})$  were then read from the line. These values were used to compute the relevant model parameters with results shown in table 3. In this table, the notation "failed" indicates those cases for which the correlation of the points from the data was clearly low or the line sloped downwards instead of upwards.

Interpretation of the calibration results consists of examining the reasons for the "failed" data and comparing particle sizes and vertical diffusivities corresponding to the model parameters chosen to other published values. The earliest data set for which the calibration failed was the first plume on 9/7/78. In this instance, the three estimates of C<sub>m</sub> at a single distance, 200 feet, prevented the analysis from being stable, so the failure can be assigned to sampling strategy rather than properties of the plume. The data for plumes 1 and 3 on 9/19/78 also failed. In the first case, the correlation was low. As in all but one of the failed cases, the plume was not found further than 250 feet from the source, because it had already dispersed, because the survey did not happen to cross it, or because the dredge operation was not

producing a detectable plume at the time of sampling or during the preceeding 20 minutes. Plume numbers of 9/19/78 also failed, and this is of particular interest because it produced the highest measured suspended sediment concentrations (>180 mg/liter) for the entire study. Such large maxima were never found far from the dredge head, and near the head the operation must appear as a distributed source rather than the point source assumed in the model formulation. Thus, we can estimate that the near field region, for which the point source theory is not expected to describe the plume extends about 300 feet from the dredge head. The final failure concerns the second interpretation of the data from 9/26/78. If we choose the first interpretation, which fits the data, there are two unexplained peaks of sediment concentration downstream from the dredge head, at distances of 700 and 880 feet from the head. These peaks could be attributed to earlier dredging at a different source strength or to extraneous sources, such as the passage of vessels down the channel.

Particle diameter (a) and coefficient of vertical diffusivity  $(k_{_{\rm Z}})$  are related to the non-dimensional times used in the analysis,  $t_{_{\rm W}}$  and  $t_{_{\rm d}}$ , respectively through formulas listed in Table 1. Some insight into the effectiveness of the model and its calibration can be gained by comparing the calibration-derived values of particle diameter and vertical diffusivity to other estimates from other studies.

The particle diameters obtained from the calibration analyses ranged between 38 and 60 microns. These sizes are

in accordance with the "type B" sediments of Nichols (1972), who noted that sediments in the James River of type B were found in the lower estuary both on the shoals and on the channel floor, where tidal current peaks reached 30 cm/sec. Because the tidal currents in the lower Elizabeth River near the bottom reach 30 cm/sec and because the Elizabeth River is directly connected to the main channel of the lower James River, the model results appear to be consistent with the previous work. On the other hand, samples taken from the general area subsequent to the dredging have a mean diameter of only 6 microns, with a variance of 70 microns<sup>2</sup>. The discrepancy between these numbers could be ascribed to any or a combination of several sources, including a substantial variability of the sediments within that reach of the river, differences in laboratory techniques used in the various size measurements and the response of the model calibration procedure to a heterogeneous mix of sediment sizes.

Values of the coefficient of vertical diffusivity, where such a formulation is used to depict vertical transfer of material in a fluid with turbulent fluctuations, range over a wide range of values. Kullenberg (1971), measuring the vertical and horizontal growth of dye patches in a shallow part of the Kategat, reported values of  $k_z$  ranging from .05 to  $110~{\rm cm}^2/{\rm sec}$ . The values were strongly related to the degree of vertical stratification in the water column, higher stratifications inhibiting the vertical mixing. In the James River,

Pritchard (1967) estimated values of  $k_z$  which ranged from 0 at the surface and bottom, due to the analysis method, to a pair of peak values of 5 and 9 cm<sup>2</sup>/sec. At a distance of 1 meter from the bottom, the value is slightly greater than 1 cm<sup>2</sup>/sec. In view of the wide range of values found in the Kategat, the range of values found in the calibration study, 1.08 to 2.90 cm<sup>2</sup>/sec, appears to agree well with the available previous data.

With these results, the model, which before had been shown to be in accord with the field data in a qualitative sense outside of a near field region of about 300 feet, seems to give results in the process of calibration which are quantitatively consistent with other studies in the study area. This agreement serves as a verification of the model formulation.

#### REFERENCES

- Kullenberg, G., 1971, Vertical diffusion in shallow waters, Tellus XXIII, pp. 129-135.
- Kuo, A. J. and R. L. Lukens, 1981. A Model for the Dredge-Induced Turbidity, Virginia Institute of Marine Science, 1981.
- Nichols, M. M., 1972, Sediments of the James River Estuary, Virginia, The Geological Society of America Memoir/33, 43 pp.
- Pritchard, D. W., 1967, Observations of circulation in coastal plain estuaries in Lauff, ed., Estuaries, Publication No. 83, American Association for the Advancement of Science, pp. 37-44.

# Appendix 1

# SUSPENDED SEDIMENT DATA

# Position Data

Date-Track	Time	Locati N/S	on(ft) E/W	Time Relative to High(H) or Low(L) Water	Tidal Phase
9/7/78-1	1032.40 1034.81 1039.31 1042.60 1045.52 1047.67	-95S -150S -200S -250S -300S -350S	0 0 0 0 0	L + 4:53 H - 1:29	Late Flood
9/7/78-2	1202.50 1211.00	-200S -200S	400E -450W	н - 0:00	High Slack
9/7/78-3	1215.70 1217.11 1219.21	-50s -50s -50s	260E 0 -250W	H + 0:12	High Slack
9/7/78-4	1233.67 1240.18	-80S -1000S	0 0	H + 0:30	High Slack
9/7/78-5	1306.70 1308.83	-40S -400S	0 0	H + 1:04 L - 5:11	High Slack
9/7/78-6	1607.16 1607.90 1608.62	220N 220N 220N	-80W 0 200E	H + 4:04 L - 2:11	Full Ebb
9/7/78-7	1608.62 1611.51	220N 400N	200E -300W		
9/7/78-8	1611.51 1613.71 1615.22	400N 420N 500N	-300W -80W 300E		
9/7/78-9	1615.22 1618.70	500N 1100N	300E -100W		
9/7/78-10	1618.70 1620.90	1100N 1300N	-100W 300E	H + 4:16 L - 1:59	
9/19/78-1	1303.87 1304.20 1305.96 1306.52 1307.20 1307.78	0 100N 300N 500N 700N 900N	0 0 0 0 0	H + 2:14 L - 4:03	Early Ebb

Appendix 1 - Position Data (Cont'd)

Date-Track	Time	Location N/S	(ft) E/W	Time Relative to High(H) or Low(L) Water	Tidal Phase
9/19/78-2	1540.35 1541.22 1542.11 1543.00 1543.83 1545.01	0 200N 400N 600N 800N 990N	0 0 0 0 0	H + 4:51 L - 1:26	Late Ebb
9/19/78-3	1552.01 1553.79 1554.82 1555.97 1556.92 1558.81 1559.02	1000N 800N 600N 400N 200N 50N	0 0 0 0 0 0	H + 5:03 L - 1:15	Late Ebb
9/19/78-4	1607.20 1608.14 1608.89	-200S 0 200N	0 0 0	H + 6:17 L - 1:00	Late Ebb
9/19/78-5	1609.60 1611.22 1612.34	200N 0 -200S	0 0 0	H + 6:20 L - 0:58	Late Ebb
9/26/78-1	1042.01 1044:21 1046.40 1047.90 1050.17 1050.93 1052.05	1000N 800N 600N 400N 200N 0	0 0 0 0 0 0	L + 0:13 H - 6:03	Low Slack
9/28/78-1	1144.60 1148.21 1152.10 1155.25 1158.01 1200.81 1203.52	800N 600N 400N 200N 0 -200S -400S	0 0 0 0 0 0	H + 5:33 L - 0:29	Low Slack
9/26/78-2	1104.02 1104.72 1106.82 1108.01 1109.60 1111.24	0 200N 400N 600N 800N 1000N	0 0 0 0 0	L + 0:35 H - 5:47	Low Slack

Appendix 1 - Position Data (Cont'd)

Date-Track	Time	Locati N/S	on(ft) E/W	Time Relative to High(H) or Low(L) Water	Tidal Phase
9/26/78-3	1749.11 1750.30 1751.82 1753.11 1755.13 1756.40 1757.80 1758.90 1800.00 1801.63	1000N 800N 600N 400N 200N 0 -200S -400S -600S -800S	0 0 0 0 0 0 0	H + 0:58 L - 5:27	High Slack
9/28/78-2 25 ft.	1210.80 1212.29 1213.86 1215.68 1217.05 1218.69 1219.97	-400S -200S 0 200N 400N 600N 800N	0 0 0 0 0 0	H + 5:59 L - 0:06	Low Slack
9/28/78-3	1226.60 1229.01	800N	-200W +200E		
9/28/78-4	1231.13 1238.31	600N 600N	200E -200W		
9/28/78-5	1303.10 1305.13	400N 400N	-200W 200E		
9/28/78-6	1306.66 1316.00	200N 200N	300E -300W		
9/28/78-7 25 ft.	1320.80 1324.11	200N 200N	-300W 300E		
9/28/78-8 25 ft.	1327.57 1335.40	600N	300E -300W	L + 1:11 H - 5:02	
9/28/78-9	1736.50 1739.55 1740.75	200N -200S -400S	0 0 0	L + 5:20 H - 0:53	Late Flood

# Suspended Sediment Data 9/7/78 Track 1 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft)	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U
1032.48 1032.48 1032.54 1032.59 1032.64 1032.67 1032.67 1032.67 1032.67 1032.77 1032.84 1032.89 1032.99 1032.99 1033.91 1033.91 1033.91 1033.91 1033.91	#g 22110000000000000000000000000000000000			1033.42 1033.45 1033.54 1033.56 1033.56 1033.67 1033.67 1033.75 1033.77 1033.83 1033.83 1033.89 1033.89 1033.91 1033.91 1033.91 1034.01 1034.01 1034.01 1034.01 1034.01				1034.46 1034.54 1034.54 1034.57 1034.61 1034.69 1034.69 1034.81 1034.81 1034.81 1034.81 1034.81 1034.81 1034.81 1034.91 1034.91 1035.02 1035.14 1035.12 1035.22 1035.24 1035.32 1035.32			
1033.37 1033.39	36 36			1034.41 1034.43	31 31			1035.46 1035.49	27 26		

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location (ft) N/S E/U
1035.51	26 25			1036.49	35 36			1037.47	47	
1035.54	26			1036.52	36			1037.51	47	
1035.56	26			1036.54	37			1037.53	47	
1035.59	26 25 25 24			1036.57	37			1037.56	47	
1035.61	25			1036.60	38			1037.61	47	
1035.64	24			1036.63	38			1037.64	47	
1035.66	24			1036.66	37			1037.66	47	
1035.69	24			1036.68	37			1037.69	47	
1035.72	24			1036.71	37			1037.71	47	
1035.74	53 53			1936.74	37			1037.74	47	
1035.77	23 22			1036.77	38			1037.76	46	
1035.79	53 53 53			1036.79	39			1037.79	46	
1035.82 1035.84	23			1036.82 1036.84	41 43			1037.82	46	
1035.87	23			1036.87	45			1037.86	46	
1035.89	23 23			1036.89	46			1037.86 1037.89 1037.91	46 46 45 45	
1035.92	24			1036.92	46			1037.91	45 44	
1035.94	25			1036.94	46 46			1037.97	73	
1035.94 1035.97	24 25 26 27 28 31 33			1036.97	46			1037.99	43 42	
1035.99	27			1036.99	45			1038.02	41	
1936.92	58			1037.02	44			1038.04	40	
1036.04	31			1037.04	43			1038.07	38	
1036.07	33			1037.07	43			1038.09	37	
1036.09	34			1037.09 1037.12	43			1038.12	38 37 36 35 34	
1036.12				1037.12	43			1038.14 1038.17	35	
1036.14	38			1037.14	44			1038.17	34	
1036.17	39			1037.17	45 46			1038.19	33	
1036.19	39			1037.19	46			1038.22	32	
1036.22 1036.25	39			1037.22 1037.24	46			1038.24	33 32 32 33	
1036.27	38 30			1037.24	47			1038.29	32	
1036.27	30 37			1037.27	47			1038.31	33	
1036.30 1036.32	39 39 38 38 37 36 35			1037.29	47 47			1038.34	34	
1036.35	35			1037.32 1037.34	47			1038.36 1038.39	35 36 37	
1036.37	34			1037.37	46			1038.41	30 37	
1036.40	34			1037.39	46			1038.44	38	
1036.44	34			1037.42	46			1038.46	39	
1036.46	34			1037.45	47			1038.49	39	

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location (ft) N/S E/U
1038.51	40			1039.61	44			1040.79	51	
1038.54	40			1039.63	45			1040.81	ši	
1038.56	40			1039.66	45			1040.84	51	
1038.59	40			1039.69	45			1040.86	51	
1038.61	40			1039.73	45			1040.89	Šī	
1038.64	40			1039.76	45			1040.92	51	
1038.67	40			1039.79	45 45 45			1040.94	51 52 52 52	
1038.69	40			1039.82 1039.86	45			1040.97	šā	
1038.72	41			1039.86	44			1040.99	52	
1038.76	41			1039.89	44			1041.06	52	
1038.78	41			1039.92	43			1041.09	51	
1038.81 1038.84	41			1039.96	42			1041.12	-51	
1038.84	41			1039.99	42			1041.15	51	
1038.87	41			1040.01	41			1041.18	51	
1038.90	41			1040.04	41			1041.18 1041.21	51	
1038.92	41			1040.07	41			1041.24	51	
1038.95	41			1040.10	40			1041.26	51 51 52 52 52 52 52 52 52 52	
1038.99	41			1040.14	40			1041.29	51	
1039.01	41			1040.18	40			1041.33	51	
1039.05	40			1040.21	40			1041.37	52	
1039.08	40			1040.23	41			1041.39	52	
1039.11 1039.13	49 41			1040.26	41			1041.42	52	•
1039.13	71			1040.28	41			1041.45	52	
1039.10	41			1040.31	41			1041.47	52	
1039.22	41 42			1040.33	42			1041.50	52	
1039.25	42			1040.37	42			1041.52	55	
1039.29	42			1040.40	43			1041.56	51	
1039.31		-200 S	0	1040.43 1040.46	44 46			1041.58	51 51 51	
1039.34	42	-600 3	•	1040.49	47			1041.61	51	
1039.37	42			1040.52	48			1041.63	51	
1039.37 1039.39	43			1040.54	49 49			1041.67 1041.70	51	
1039.42	43			1040.57	50			1041.73	21	
1039.45	43			1040.60	50			1041.76	51 51 52 52 53 53	
1039.49	44			1040.66	50			1041.78	55	
1039.52	44			1040.69	50			1041.81	รัร	
1039.56	44			1040.73	Šě			1041.84	53	
1039.58	44			1040.76	50			1041.87	52	

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/U
EST 1041.99 1041.95 1041.97 1042.00 1042.03 1042.08 1042.11 1042.13 1042.19 1042.24 1042.24 1042.36 1042.36 1042.36 1042.41 1042.47 1042.49 1042.55 1042.55 1042.55 1042.60 1042.60 1042.60	mg/l 51 548 47 46 443 441 440 441 443 444 443 444 445 445 459 553			EST 1042.94 1042.99 1043.07 1043.07 1043.17 1043.27 1043.23 1043.34 1043.34 1043.46 1043.51 1043.51 1043.61 1043.51 1043.79 1043.79 1043.79 1043.87	mg/ 52255115555555555555555555555555555555			1044.11 1044.17 1044.27 1044.25 1044.27 1044.32 1044.37 1044.37 1044.37 1044.44 1044.51 1044.54 1044.54 1044.54 1044.64 1044.67 1044.67 1044.74 1044.74 1044.74 1044.74	mg/ 466 455 455 444 443 210 00 00 00 00 00 00 00 00 00 00 00 00 0	
1042.68 1042.71 1042.73 1042.79 1042.82 1042.84 1042.87	54 55 55 55 55 54 54 53 53			1043.87 1043.89 1043.92 1043.95 1044.01 1044.04 1044.06	47 47 47 47 46 46 46 46			1045.02 1045.04 1045.07 1045.11 1045.13 1045.17 1045.21 1045.24	33 33 33 33 33 33 33 33 33	

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/W
1045.36	33 33 33			1046.52	40					
1045.39	33			1046.56	40					
1045.42	33			1046.59	39 39					
1945.46	33			1046.62	39					
1 <b>04</b> 5.49 1 <b>04</b> 5.52	33	700 C	•	1046.66	39 39					
1045.55	33	-3 <del>0</del> 0 S	0	1046.69	39					
1045.57	33 33 34 34			1046.72	39					
1045.61	33			1046.75 1046.79	39 39 38					
1045.64	37			1046.82	39					
1045.67	32			1046.86	38					
1845.71	36			1046.89	30					
1045.73	37			1046.92	30					
1045.78	35 36 37 38			1046.95	38 39 39 39					
1045.81	38			1046.99	39					
1045.83	39			1047.01	39					
1045.88	39			1047.04	39					
1045.91	40			1047.07	40					
1045.94	40			1047.10	40					
1045.97	40			1047.12	40					
1046.00	40			1047.17	39 39					
1046.02	40			1047.22	39					
1046.06	41			1047.25	39					
1046.09	41			1047.31	39 39 39 38					
1046.12 1046.15	75			1047.34	39					
1046.19	42 42 43			1047.37	39					
1046.23	44			1047.41	38					
1046.26	44			1047.43 1047.51	38					
1046.28	45			1047.53	38 39 39 39					
1046.31	45			1047.56	39					
1046.34	45			1047.59	30					
1046.37	45 45 45 44			1047.62	39					
1046.39	· 43			1047.64	39					
1046.42	43			1047.67	39 39	-350 S	0			
1046.44	42						-			
1046.47	41									
1046.49	41									

# Suspended Sediment Data 9/7/78 Track 2 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(fも) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/W
1202.50	29	-200 S	400 E	1203.49	30			1204.52	29	
1202.50	29			1203.52	29			1204.55		
1202.53	28			1203.55	28			1204.57	20	
1202.57	28			1203.57	28			1204.60	20	
1202.57	28			1203.60	ŽŽ			1204.62	29	
1202.59	28			1203.63	27			1204.67	29	
1202.62	28			1203.65	27			1204.70	29	
1202.65	28 28 28 28 28 28			1203.68	28 27 27 27 27			1204.73	29999999999999999999	
1202.68	28			1203.72	27			1204.76	29	
1202.69	28			1203.75	27			1204.80	29	
1202.71	28 28 28 28 28 28			1203.78	27			1204.82	29	
1202.75	28			1203.82	59 59 85			1204.85	28	
1202.77	28			1203.85	<b>S</b> 9			1204.87	28	
1202.80	28			1203.88	29			1204.88	28	
1202.82	28			1203.88 1203.91 1203.93	28			1204.91	29	
1202.85	27			1203.93	28			1204.93	ĕš	
1202.87	27			1203.95	27			1204.95	29	
1202.90	27 27			1203.98	27 27 27			1204.97	22222222222222222222222222222222222222	
1202.93	27			1204.02	27			1205.00	29	
1202.96	27 28 27 27			1204.02	27			1205.02	29	
1203.00	28			1204.05	27			1205.05	29	
1203.03	27			1204.08	27 28			1205.07	29	
1203.06 1203.08	27			1204.12	29			1205.07	29	
1203.08	27			1204.15	30			1205.10	28	
1203.12	28 28 28			1204.16 1204.17	30			1205.12	28	
1203.15	58			1204.17	30			1205.16	28	
1203.18	58			1204.20	30			1205.18	. 58	
1203.22	29 29			1204.22	29 29			1205.20	28	
1203.25	58			1204.23	29			1205.22	28	
1203.27	59			1204.25	29			1205.24	28	
1203.30	59			1204.27	29			1205.25	28	
1203.34				1204.30	29 29 29			1205.27	28 28 28 28 28 28 28	
1203.37 1203.38	ح8 20			1204.35	29			1205.30	58	
1203.38				1204.37	29			1205.33	28	
1203.39	20 20			1204.40	29			1205.36	28	
1203.45	29			1204.42	29 29			1205.40	29 29	
1203.47	30 30			1204.46	29			1205.43	29	
1503.71	30			1204.48	29			1205.48	29	

Time EST	Sed.Conc mg/l	Location N/S	(ft)	Time EST	Sed.Conc mg/l	location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	ENN (IF)
1205.52	30 59			1206.73	39	•		1207.71	41		
1205.56	30			1206.76	39 39 38			1207.75	40		
1205.58	30			1206.78	39			1207.78	49		
1205.62	30			1206.82	38			1207.80 1207.83	40		
1205.64	<i>3</i> 8			1206.85	37			1207.83	39		
1205.67	30 30 30 30 30			1206.87 1206.89	35 34			1207.85	37		
1205.70 1205.74	30			1206.03	37			1207.85	37		
1205.77	30			1206.92 1206.96	35 36			1207.86	37 38		
1205.80	36			1206.99	34			1207.88 1207.92	. 40		
1205.84	30 30 30			1207.02	31			1207.92	40		
1205.85 1205.87 1205.87	30			1207.04	30			1207.94	39		
1205.87	30			1207.07 1207.08 1207.10	30 30			1207.97	37		
1205.87	30			1207.08	29 29 29 29 29 30			1207.99 1208.00	35		
1205.90	30 30 30			1207.10	29			1208.00	35		
1205.93	30			1207.13	30			1208.02	35		
12 <b>0</b> 5.97	3 <b>6</b> 31			1207.16	59			1208.05	35		
1206.05	31			1207.20 1207.22 1207.23	59			1208.05	35 35 35 35 35		
1206.10	31			1207.66	30			1208.07	35 36		
1206.13	31			1297.26	20			1208.12	36 37		
1296.17	31 31 31			1207.27	30			1208.12	37		
1206.20	31			1207.29	30			1208.13	37		
1206.22	31 31 31			1207.27 1207.29 1207.32 1207.33	30 30 30 31			1208.13 1208.15 1208.16	37		
1206.25	31			1207.33	71			1208.16	35		
1206.28	31			1207.35	35 35 35 35			1208.17	35 35 35		
1206.30	31			1207.37	32			1208.17	35		
1206.34	32			1207.38 1207.40	35			1208.21	39		
1206.37 1206.39	31 31 30			1207.40	35			1208.24	44		
1206.43	37			1207.45	33			1208.27	46		
1206.47	31			1207.50	35 36			1208.30	46 46		
1206.51	32			1207.54 1207.57	33 35 36 38			1208.30 1208.33 1208.35	43		
1206.55				1207.60	39			1208.35	39		
1206.59	36			1207.62	40			1208.37	39		
1206.62	37			1207.64 1207.67	40			1208.38	39		
1206.66	37			1207.67	41			1208.39	39		
1206.70	38			1207.68	41			1208.42	40		

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/ <b>U</b>	Time EST	Sed.Conc mg/l	Location N/S	EVW (1f)	Time EST	Sed.Conc mg/l	Location N/S	(ft)
1208.44	38			1209.28	33			1210.13	31		
1298.47	38 38			1209.32 1209.33	34			1210.14	31 31		
1208.48	38			1209.33	34			1210.17	31		
1208.51	38			1209.35	34			1210.20	31		
1208.54	40			1209.37	33			1210.22	30		
1208.57				1209.40	33 32 32 32 33			1210.23	30		
1208.60	46			1209.42	32			1210.25	30		
1208.60 1208.63	46 44			1209.42 1209.46	3 <b>2</b>			1210.27	30		
1208.65				1209.48	33 34			1210.28 1210.30	30		
1208.66	44			1209.50	34 34			1210.30	29 27		
1208.69	44			1209.50	34			1210.35	26		
1208.72	42			1209.51 1209.52 1209.55	34			1210.37	25		
1208.74	40			1209.55	34			1210.40	25 26		
1208.77	36			1209.57	34			1210.40 1210.42	27		
1208.79	36 34 32 30			1209.58	35			1210.43	ĒŻ		
1208.82	32			1209.60 1209.62 1209.62	35			1210.47	27		
1208.84	30			1209.62	34			1210.49	27		
1208.85	29			1209.62	34			1210.50 1210.53	27		
1208.87	58			1209.65	34			1210.53	27		
1208.90	es es			1209.67	34 34			1210.57	30		
1208.93 1208.97	29			1209.68 1209.71 1209.73	34			1210.60	35 37		
1208.97	36			1209.71	<b>שב</b> סכ			1210.63	37		
1209.00				1209.77	32 29 26 26 27			1210.63 1210.63 1210.65	37		
1209.02	30			1209.79	2C			1210.68	36 34		
1209.07	30			1200.82	27			1210.69	37		
1209.09	31			1209.82 1209.85 1209.88	žģ			1210.71	34 34		
1209.10	31			1209.88	30			1210.71 1210.74	35		
1209.11	32			1209.92	32			1210.76	35		
1209.11 1209.13 1209.15	31 32 32 31			1209.95 1209.97 1209.98	32 33			1219.77	35		
1209.15	31			1209.97	34			1210.80 1210.82	34		
1209.16	31			1209.98	34			1210.82	33		
1209.18 1209.20	31			1210.02	35 37			1210.85	35		
1209.20	31 31			1210.05	37			1210.87	31		
1209.22				1210.06 1210.08	36 33			1210.90	35		
1209.25				1210.11	35			1210.92 1210.93	35 35		
	~				36			1010.93	JE		

Time EST	Sed.Conc mg/l	Location N/S	E/U	Tame EST	Sed.Conc mg/l	Location N/S	E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft)
1210.95 1210.95 1210.98 1211.00	<b>28</b>	-200 5 -	450 U								

# Suspended Sediment Data 9/7/78 Track 3 Depth is 50 feet

1215.70 83 -50 S 260 E 1216.53 72 1217.32 68 1215.74 99 1216.58 74 1217.34 71 1215.76 100 1216.58 74 1217.36 74 1215.77 100 1216.58 74 1217.36 74 1215.77 100 1216.62 81 1217.37 74 1215.77 100 1216.62 81 1217.37 74 1215.82 102 1216.63 81 1217.40 68 1215.82 103 1216.65 82 1217.42 63 1215.84 103 1216.67 81 1217.44 61 1215.88 103 1216.67 78 1217.45 61 1215.88 102 1216.67 78 1217.45 61 1215.89 99 1216.75 76 1217.45 61 1215.89 99 1216.75 76 1217.55 60 1215.93 87 1216.82 81 1216.87 77 1217.55 60 1215.93 88 1216.82 81 1217.55 60 1215.95 84 1216.82 81 1217.55 60 1216.62 89 1216.63 83 1217.55 60 1216.63 83 1217.55 84 1216.82 81 1217.57 58 1216.62 90 1216.63 83 1217.63 60 1216.65 89 1216.85 83 1217.63 60 1216.67 84 1216.88 84 1216.88 84 1216.89 83 1217.69 60 1216.10 76 1216.10 76 1216.10 76 1216.10 76 1216.10 76 1216.10 76 1216.90 90 1216.90 90 1216.90 90 1216.90 90 1216.90 90 1216.90 90 1216.88 84 1216.89 83 1217.89 60 1216.60 90 1216.60 90 1216.80 80 1216.80 80 1216.80 80 1217.80 60 1217.80 60 1216.10 76 1216.20 80 1216.80 80 1216.80 80 1217.80 60 1217.80 60 1216.12 60 1216.12 60 1216.90 73 1217.70 57 1216.13 66 1216.20 60 1217.00 73 1217.70 57 1216.13 60 1217.00 60 1217.00 73 1217.70 60 1	Time	Sed.Conc	Location	({{t}})	Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location (ft)
	EST	mg/l	N/S	E/ <b>U</b>	EST	mg/l	N/S	E/W	EST	mg/l	N/S E/W
	1215.70 1215.76 1215.77 1215.82 1215.82 1215.83 1215.83 1215.93 1215.93 1215.93 1215.93 1216.07 1216.15 1216.20 1216.21 1216.23 1216.23 1216.32 1216.32 1216.44 1216.44 1216.44	3900023329074480946976308933214676667 100233290744809469766665556666666666666666666666666666			1216.53 1216.59 1216.62 1216.62 1216.62 1216.63 1216.65 1216.73 1216.73 1216.83 1216.83 1216.88 1216.98 1216.99 1217.07 1217.07 1217.11 1217.12 1217.12	724 7777 888 777667 888 888 77777 888 888 7777766 889 999 1099 1099 1099			1217.32 1217.36 1217.40 1217.40 1217.45 1217.55 1217.55 1217.57 1217.57 1217.61 1217.61 1217.61 1217.61 1217.61 1217.61 1217.67 1217.70 1217.70 1217.70 1217.70 1217.70 1217.90 1217.90 1218.05 1218.05 1218.07	81442831134308880009733627888892339816	N/S E/W

Time EST	Sed.Conc mg/l	Location N/S	(ft)	Time EST	Sed.Conc mg/l	Location N/S	([t) E/W	Time EST	Sed.Conc mg/l	Location (f	'ቲ) 'ዜ
1218.15 1218.26 1218.26 1218.26 1218.37 1218.37 1218.37 1218.37 1218.47 1218.47 1218.53 1218.65 1218.65 1218.65 1218.65 1218.65 1218.83 1218.83 1218.83 1218.83 1218.83 1218.83	5332233322117776678801197544333332222333333333333333333333333333			1219.08 1219.10 1219.13 1219.16 1219.18 1219.21	31 31 30 30 30 30 30 30		250 U	£31	mg/l	N/5 E/	

# Suspended Sediment Data 9/7/78 Track 4 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	location N/S	(1t) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/U
1233.67	36	-80 S	9	1234.75	44			1235.84	35	
1233.70	36			1234.75	44			1235.87	35 35	
1233.74	35			1234.79 1234.82	42			1235.91 1235.95 1235.98	35	
1233.77	35			1234.82	41			1235.95	35	
1233.82	35			1234.85	40			1235.98	34	
1233.85	36			1234.89	39			1236.02	34	
1233.85 1233.87 1233.90	36 36 35 35 35 35 36 37 38			1234.92	38 39 39			1236.05	34 34	
1233.90	38			1234.95 1234.97	38			1236.08 1236.12 1236.15	34	
1233.93				1234.97	38			1536.15	34	
1233.96	41			1234.98	38			1236.15	34	
1233.99	41			1235.01	38			1236.19	34	
1234.01 1234.04	43			1235.02	38 39			1236.22 1236.25 1236.27 1236.30	34	
1234.68	44			1235.07	36 36			1235.25	35	
1234.10	44			1235.10	35			1236.51	35 35	
1234.13	44 45 47			1235.13	38 38 38 38 36 35 34 34			1236.32	35	
1234.16	47			1235.17	34			1236.36	35 35 35	
1234.18	48			1235.19	34			1236.37	35	
1234.19	48 48 47			1235.02 1235.02 1235.07 1235.10 1235.13 1235.13 1235.22	36 37 37 37			1236.34 1236.37 1236.40	35	
1234.21 1234.25	47			1235.24	37			1236.45	34	
1234.25	46			1235.25	37			1236.48	34 33 33 33	
1234.28	47			1235.27	37			1236.52 1236.56 1236.60	33	
1234.30				1235.30	37			1236.56	33	
1234.33	49			1235.34	37			1236.60	33	
1234.35 1234.38	50 51			1235.37	37 37			1236.63	33	
1234.41				1235.42	37			1236.65	35	
1234.43	51 51			1235.45	37			1536.68	35	
1234.44	51			1235.49	36			1236.68 1236.70 1236.75	33	
1234.44 1234.47	50			1232.36	36 36 36			1236.75	33	
1234.50	Šě			1235.55	36			1236.78	33 33	
1234.53	49			1235.62	36			1236 04	33	
1234.56	49 49 48			1235.67	36			1236.78 1236.81 1236.84 1236.86	33	•
1234.59	48			1235.70	36			1236.88	33	
1234.63	47			1235.24 1235.35 1235.37 1235.37 1235.37 1235.45 1235.45 1235.55 1235.56 1235.67 1235.67 1235.77	36 36 35			1236.88 1236.90 1236.94	33	
1234.67				1235.77	35			1236.94	34	
1234.70				1235.80	35			1236.95	34	
1234.72	44			1235.82	35			1236.98	33	

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(if)
1237.01	33 34 34			1238.28	31			1239.50	32		
1237.05	34			1238.31	31	•		1239.52	32		
1237.06	34			1238.35	31 31			1239.55	33		
1237.10	33			1238.38 1238.41 1238.46	31	_		1239.55	33		
1237.13 1237.16	33			1238.41	31			1239.58	33		
1237.19	33			1238.49	31			1239.58 1239.62 1239.65	32 33 33 33 33 33		
1237.21	33 33 33 33 32 32 32 32 31			1238.52	31 31 31 31 31			1239.65	33		
1237.23	33			1238.55	31			1239.69 1239.73	33		
1237.26	32			1238.59	31			1229.73	33		
1237.23 1237.26 1237.30	35			1238.55 1238.59 1238.62	31			1239.77 1239.78 1239.82	33		
1237.33	3ž			1238.66	31 31			1239.82	33		
1237.36	32			1238.69	31			1239.85	33		
1237.39 1237.41	31			1238.71	31			1239.88	33		
1237.41	31			1238.71 1238.74 1238.77	31 31 31 31			1239.92	35		
1237.43	31			1238.77	31			1239.96 1239.99	32		
1237.46	31			1238.80	30			1239.99	35		
1237.51	31 31 31 31 31			1238.83	30			1240.02	33 33 33 33 33 32 32 32 32 32 32 32 32 3		
1237.55 1237.59	31 34			1238.87	30 30			1240.05	35		
1237.63	31			1238.90 1238.94	30 30			1240.08	35		
1237.66	31			1238.98	30			1240.11 1240.15	35		
1237.69	31 31 31 31			1239.02	30			1240.18	32 32		
1237.73	31			1239.05	29			1670.10	36		
1237.73 1237.77	31			1239.09 1239.14 1239.17	29						
1237.80	31			1239.14	29						
1237.83	30			1239.17	29						
1237.87	30 30 31			1239.19	30						
1237.91 1237.96	31			1239.22	30 31						
1237.96	31			1239.24 1239.28	31						
1238.02	30 30			1239.28	31						
1238.04	30			1239.31 1239.34	31						
1238.08	31			1239.37	31 31						
1238.11	31			1239.35 1239.38	31						
1238.15	30			1239.41	31						
1238.19	30			1239.45	31						
1238.24	31			1239.50	31						

# Suspended Sediment Data 9/7/78 Track 5 Depth is 50 feet

Time	Sed.Conc	Location	(1t)	Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location	(ft)
EST	mg/l	N/S	E/W	EST	mg/l	N/S	E/W	EST	mg/l	N/S	E/W
1306.70 1306.73 1306.76 1306.86 1306.89 1306.93 1306.96 1307.02 1307.17 1307.21 1307.24 1307.30 1307.30 1307.30 1307.36 1307.45 1307.45 1307.45 1307.51	255566666666666665555666666667 <b>89998877</b>	-40 5	•	1307.83 1307.86 1307.89 1307.99 1308.03 1308.05 1308.12 1308.12 1308.12 1308.24 1308.27 1308.33 1308.37 1308.31 1308.33 1308.37 1308.47 1308.47 1308.50 1308.50 1308.50 1308.50	26666666666555555445447333334455555	- <b>400</b> S					

# Suspended Sediment Data 9/7/78 Track 6 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(it)
1607.16	64	220 N	-80 U	1607.87	62						
1607.19	73 73			1607.88	64	222 1	•				
1607.19 1607.22	54			1607.90	54	220 N	0				
1607.26	46			1607.93	42						
1607.27	45			1607.95 1607.96	38 39						
1607.28				1607.99	37						
1607.30	46			1608.01	36						
1607.32	46 46 49 53 52			1608.03	36 37						
1607.34	53			1608.03	37						
1607.35	52			1608.06	41						
1607.35	51			1608.07	42						
1607.37	53 51			1608.08	42						
1607.38	51			1608.10	40						
1607.40	49			1608.11	39 39						
1607.43	45			1608.13	39						
1607.46	53			1608.15	38						
1607.47 1607.49	55 48 42 43			1608.19	34						
1607.51	70 42			1608.20 1608.22	34 34 37				•		
1607.52	43			1608.25	3 <del>4</del>						
1607.55	43			1608.25	38						
1607.58	40			1608.27	37						
1607.58	39			1608.30	37						
1607.60	39 42 48			1608.30	37 37						
1607.63	48			1608.33	36						
1607.63	48			1608.34	35						
1607.66	47 45 38 47			1608.36	40						
1607.67	45			1608.39	41						
1607.70 1607.73	38 43			1608.41	41						
1607.75	51			1608.42	41						
1607.75	52			1608.45 1608.47	51						
1607.77	52 52 51			1608.51	65 65 61						
1607.78	51			1608.54	61						
1607.81	48			1608.57	52						
1607.82	48 55			1608.59	47						
1607.84	55			1608.62	46	250 N	500 E				

# Suspended Sediment Data 9/7/78 Track 7 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	E/H	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc	Location (ft) N/S E/W
1608.62	46	220 N	200 E	1609.30	53			1609.96	78	
1608.62	46			1609.33	51			1609.98	77	
1608.64	44			1609.36	47			1610.01	78	
1608.65	46			1609.37	47			1610.03	80	
1608.67	50			1609.39	48			1610.04	85	
1608.67	53			1609.40	49			1610.05	85	
1608.70	55			1609.42	50			1610.06	87	
1608.70	53 55 56			1609.44	54	•		1610.08	86	
1608.72	56 52			1609.47	57			1610.10	84	
1608.75	52			1609.47	57			1610.11	84	
1608.75	47			1609.49	59			1610.12	85	
1608.77	44			1609.49	59 51			1610.15	87	
1608.78	42			1609.52 1609.55	51			1610.17	90	
1608.80	42			1609.55	47			1610.18	84	
1608.82	44			1609.55	47			1610.19	77	
1608.83	44			1609.57	53			1610.20	70	
1608.87	40			1609.58	54			1610.22	63	
1608.88	40			1609.59	60			1610.24	64	
1608.90	44			1609.60	64			1610.26	65 65 62	
1608.92	45			1609.61	66			1610.27	65	
1608.92	45			1609.64	<b>67</b>	•		1610.30	65	
1608.95	46			1609.67	66 63			1610.31	62	
1608.96	46			1609.69	63			1610.33	63	
1608.98	45			1609.70	66			1610.34 1610.37	64	
1609.00	43			1609.72	71			1610.37	58	
1609.02	43			1609.75	77			1610.40	52	
1609.04	49			1609.77	79			1610.41	54	
1609.05	51			1609.80	76			1610.42	64	
1609.07	49			1609.81	76			1610.44	68 72	
1609.10	47			1609.83	80			1610.45	72	
1609.12	48			1609.85	81			1610.47	66	
1609.14	51			1609.86	81 82 82			1610.49	61	
1609.17	51			1609.88	82			1610.50 1610.53 1610.53	60 52	
1609.20 1609.23	<u> 52</u>			1609.89	79			1610.53	52	
1609.25	51 52 52 52 52	•		1609.90	76			1610.53	50	
1609.27	26 E2			1609.93	76			1610.56	45 42	
1609.28	56 E2			1609.94	78			1610.58	42	
1003.59	36			1609.95	78			1610.60	40	

1610 - 60	Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location (ft)
	EST	mg/l	N/S	E/U	EST	mg/l	N/S	E/W	EST	mg/l	N/S E/U
	1610.60 1610.61 1610.63 1610.64 1610.66 1610.69 1610.71 1610.74 1610.77 1610.79 1610.82 1610.83 1610.83 1610.93 1610.93 1610.93 1610.93 1610.93 1610.93 1610.93 1610.93 1610.93	40 41 39 39 41 42 43 43 41 112 112 113 113 113 113 113 113 113 11			1611.28 1611.31 1611.34 1611.38 1611.39 1611.42 1611.42 1611.48	36 32 27 26 28 28 28			EST	mg/(	N/S E/U

Depth 13 50 feet											
Time EST	Sed.Conc mg/l	Location N/S	E\A (lf)	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W
1614.11 1614.14 1614.14 1614.18 1614.20 1614.24 1614.27 1614.27 1614.30 1614.30 1614.31 1614.38	45 43 43 42 43 43			1614.94 1614.95 1614.99 1615.02 1615.04 1615.05 1615.11 1615.15 1615.22	37 39 40 41 41 40 38 35 38 42	500 N ;	300 E				
1614.42 1614.43 1614.51 1614.57 1614.58 1614.61 1614.65 1614.68 1614.72 1614.78 1614.79 1614.88 1614.81	37 36 37 40									•••	

## Suspended Sediment Data 9/7/78 Track 8 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	E/U
1611.51 1611.54	26 26	400 N -	-300 H	1612.46	29 3 <b>0</b>			1613.35	47		
1611.56	31			1612.48 1612.50	36			1613.36	. 45		
1611.58	31			1612.53	50			1613.38	46 48		
1611.60	28			1612.57	24			1613.40 1613.40	48		
1611.61	31 28 27			1612.59	28 25 24 25 26 28			1613.42	46		
1611.63	29 27			1612.62	26			1613.44	40		
1611.68	27			1612.64 1612.67	28			1613.46	49 51		
1611.69	26			1612.67	30			1613.47	48		
1611-70	58			1612.69	28			1613.49 1613.52	48		
1611.73	31			1612.71	38 38 38 28			1613.52	49		
1611.77 1611.80 1611.83	28			1612.73	38			1613.53	<u>51</u>		
1611.00	25			1612.76 1612.79 1612.82	36 40			1613.54	54		
1611.85	27			1615 65	36			1613.57	57		
1611.88	ės			1612.85	37			1613.57 1613.57 1613.60	57 53		
1611.90	31			1612.88	35			1613.60	53 53		
1611.93	31			1612.88 1612.92 1612.95	37 35 34			1613.63	56		
1611.97	29			1612.92	36			1613.65	56 55 56		
1611.98	29			1612.95	40			1613.67	56		
1612.01	31			1612.95	41			1613.67 1613.71	67	420 N	-80 U
1612.01 1612.05 1612.06 1612.09	35			1612.98	37			1613.74	75		
1615.00	33 20			1613.00	36 38			1613.76	81		
1612.12	29 25 25 25 25 27			1613.00 1613.02 1613.04	38			1613.77	80 76		
1612.16	25			1613.05	41			1613.80 1613.83 1613.86	75		
1612.18	25			1613.03	48 50			1613.83	7 <b>0</b> 64		
1612.18 1612.20	27			1613.07 1613.10 1613.12 1613.13	41			1613.86			
1612.23				1613.12	41			1613.92	57		
1612.27	26			1613.13	44			1613.94	รัร		
1612.28	25			1617.14	49			1613.97	55		
1612.28 1612.31 1612.33	28 26 25 28 28			1613.18 1613.21 1613.24	51			1613.97	54 52 53 55 55 53 52		
1612.33	28			1613.21	51			1614.00	53		
1612.37	56 59			1613.24	47			1614.01			
1612.39	27			1613.25 1613.27	48 50			1614.02	54		
1612.42	30			1613.30	50 50			1614. <b>0</b> 5 1614. <b>0</b> 6	65 67		
1612.44	59			1613.32	47			1614.07	68		

## Suspended Sediment Data 9/7/78 Track 9 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ít E∕W		Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U
1615.22	42	500 N	300	Ε	1616.19	40			1617.01	47		
1615.22	43				1616.23	39			1617.02	47		
1615.26	41				1616.26	37			1617.04	51		
1615.27	40				1616.29	36			1617.07 1617.09	55 58		
1615.29	39 37				1616.31	36			1617.09	58		
1615.33	37				1616.32	36			1617.11	59		
1615.33	36 38				1616.35	36 36 37 38			1617.13	59 57 58		
1615.37 1615.40	36				1616.35 1616.39 1616.42	38			1617.15	<b>58</b> .		
1615.40	38 20				1616.39	41			1617.17	58		
1615.40 1615.42	38 38 38 37				1616.42	42			1617.17 1617.20 1617.22	59		
1615.45	36				1616.45	41 40			1617.22	59		
1615.46	32				1616.46	40			1617.25	60		
1615.46 1615.50 1615.53	37				1616.48	41			1617.65	54 48		
1615.53	35				1616.48 1616.49	41			1617.31	45		
1615.55	34				1616.54	42			1617.32	46		
1615.57	35 34 34				1616.57	40			1617.28 1617.31 1617.32 1617.35	47		
1615.59 1615.60 1615.62	34				1616.57	41			1617.36	50		
1615.60	34				1616.60	40			1617.41	50 45 42		
1615.62	35 37				1616.60	41			1617.45 1617.48 1617.49	42		
1615.65	37				1616.63	46			1617.48	39		
1615.67	37				1616.63	47			1617.49	38		
1615.68 1615.70	38				1616.65	46			1617.52	39 41		
1615.73	37 38 38 38 38 37 37 36				1616.70 1616.72 1616.74	47 46 45 45 45			1617.55	41		
1615.77	38 30				1616.72	45			1617.57 1617.58 1617.60	45		
1615.81	30 37				1616.79	75			1617.58	46		
1615.85	37				1616.78	44			1617.63	46		
1615.85 1615.88	36				1616.80	43 44			1617.65	43 44		
1615.91	35				1616.83	45			1617.65	46		
1615.95	35				1616.87	48			1617.67 1617.70	44		
1616.01	35				1616.87	50			1617.73	40		
1616.01	35 35 35 35 36				1616.89 1616.93	48			1617.75	39		
1616.05	36				1616.93	49			1617.77 1617.80	41		
1616.10	35 35				1616.94	49			1617.80	43		
1616.11	35				1616.96	47			1617.82	46		
1616.13 1616.16					1616.98	47			1617.85	48		
1010.10	38				1617.00	47			1617.89	50		

					<b>p</b>						
Time EST	Sed.Conc mg/l	location N/S	(ft)	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U
1617.94	54 51										
1617.97	51										
1618.00	47										
1618.04 1618.08	43 43										
1618.11	45										
1618.11 1618.11	45										
1618.13 1618.18 1618.22	46										
1618.18	45										
1618.22	45 45										
1618.24	45										
1618.25 1618.27	45										
1618.30	44 46 46										
1618.33	46										
1618.33 1618.37	44										
1618.41	44										
1618.45 1618.50 1618.53	46										
1618.50	46 45										
1618.53	44										
1618.55	45										
1618.55 1618.57	46										
1618.60	46 45										
1618.63 1618.63	43										
1618.63	42 42 43										
1618.67	42 43	1100 N									
1618.70	73	1100 N -	100 8								

## Suspended Sediment Data 9/7/78 Track 10 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft)	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U
EST 1618.79 1618.75 1618.78 1618.87 1618.89 1618.99 1618.99 1619.09 1619.09 1619.09 1619.19 1619.19 1619.19 1619.19 1619.19 1619.19 1619.19 1619.19 1619.19 1619.19 1619.19	mg 440 44119755559134451112210132429922	N/S				Location N/S		Time EST 1620.77 1620.80 1620.84 1620.88 1620.90		N/S	
1619.58 1619.62 1619.63 1619.65 1619.67	41 41 40 39 40 41			1620.55 1620.57 1620.60 1620.63 1620.66 1620.68	36 36 36 38 39						

#### Suspended Sediment Data 9/19/78 Track 1 Depth is 50 feet

Time EST Sed-Conc Location (ft) mg/l N/S E/U

				•	Depth is	50 feet	
Time EST	Sed.Conc mg/l	Location N/S	(It)	Time EST	Sed.Conc mg/l	Location N/S	(ft)
1303.87 1304.02 1304.08 1304.10 1304.14 1304.16 1304.20 1304.23 1304.28 1304.31 1304.34 1304.34	27 29 31 36 41 45 49 48 59 86 78 61 47 48	6 199 N	e e	1305.16 1305.20 1305.24 1305.96 1306.52 1307.20 1307.78	18 18 17 16 17 16 14	399 N 599 N 799 N 999 N	6 0 0 0
1304.43 1304.45 1304.52 1304.57 1304.60 1304.63 1304.66 1304.70 1304.71 1304.80 1304.84 1304.87 1304.93 1304.93 1304.93 1305.03	55 65 767 67 37 38 26 27 28 29 21 21 22 21 22 22 21 22 22 22 22 22 22						

## Suspended Sediment Data 9/19/78 Track 2 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft)	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft)
1540.35	54	0	9	1541.44	50			1542.41	69		
1540.39	54			1541.47	49			1542.44	66		
1540.43	54			1541.50	49			1542.46	63		
1540.47	55			1541.53	49			1542.49 1542.53	63 60 57		
1540.51	55			1541.56	48			1542.53	57		
1540.55	54			1541.59	49			1542.55	56		
1540.58	54 54 54 54 57 58			1541.62	48			1542.58	56 56 54		
1540.62	27			1541.64 1541.69	48			1542.63	56		
1540.65	24			1541.69	48			1542.65 1542.70 1542.73	54		
1540.69	5(			1541.73	48			1542.70	50		
1540.73 1540.77				1541.75	47			1542.73	46		
1540.80	- 61			1541.76	47			1542.78	46		
1540.83	69			1541.79 1541.82 1541.83	63 79			1542.82	45		
1540.86	78			1571.05	86			1542.84	45		
1540.89	99			1541.84	86			1542.84 1542.87 1542.89	43		
1540.92	111			1541.85	86			1542.89	42		
1540.93	114			1541.88	99			1542.95	41		
1540.95				1541.90	110			1545.95	41 40		
1540.98	89			1541.93	110			1542.98	39	600 N	•
1541.01	70			1541.96	119			1543.00 1543.04	39	000 11	0
1541.01	69			1541.97	iżi			1543.07	39		
1541.04	85			1542.00	119			1543.11	41		
1541.07	184			1542.00 1542.02 1542.04	117			1543.14	41		
1541.09	115			1542.04	114			1543.14 1543.17 1543.22	30		
1541.11	112			1542.05	113			1543.22	39 39		
1541.14	94			1542.09	111			1543.28	43		
1541.16				1542.11	106	400 N	8	1543.31	44		
1541.19	62			1542.14	112		•	1543.35 1543.39 1543.42	44		
1541.22	57	200 N	0	1542.14	112			1543.39	45		
1541.24				1542.17	108			1543.42	44		
1541.25	52			1542.20 1542.24 1542.26	101			1543.44	43		
1541.27	56			1542.24	95			1543.47	40		
1541.30				1542.26	88			1543.47 1543.49 1543.52	40		
1541.33	. 59			1542.29	75			1543.52	40		
1541.36 1541.38	73 54			1542.31	7 <b>0</b>			1543.55	38		
1541.41				1542.34	69			1543.62	38		
1341.41	70			1542.38	71			1543.64	37		

Time EST	Sed.Conc mg/l	Location N/S	E/U	Time EST	Sed.Conc mg/l	Location N/S	(If)	Time EST	Sed.Conc mg/l	Location N/S	(It)
1543.69 1543.73 1543.76 1543.79 1543.83 1543.86	38 38 39 39 39			1544.96 1545. <b>0</b> 1	58 58	990 N	0				
1543. <i>1</i> 9 1543.83	39 39	899 N	0								
1543.86	40	550 13	•								
1543.89	39										
1543.92	38										
1543.92 1543.95 1543.97	38										
1544.01	38										
1544.04	38										
1544.09	38										
1544.12	38										
1544.15 1544.18	398888888888922200355533322000										
. 1544.18 1544.22	38 38										
1544.27	39										
1544.32	42										
1544.35	42										
1544.38	<b>5</b> 4										
1544.40 1544.44	38										
1544.47	35										
1544.50	35										
1544.53	35										
1544.58 1544.61	33										
1544.64	33										
1544.67	žč										
1544.67 1544.70 1544.72	30										
1544.72	30										
1544.75 1544.78	28 28 28 28 28 27										
1544.82	. 28										
1544.84	58										
1544.88	5 <b>8</b>										
1544.92	e r										

## Suspended Sediment Data 9/19/78 Track 3 Depth is 50 feet

Time EST	Sed.Conc mg/l	Loca N/S		(ft)	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft)
1552.01	27	1000	N	0	1556.52	21			1557.74	35		
1552.89	29				1556.56	21			1557.77	35 37		
1553.37	31				1556.59	22 22 22 22 22 22 22 22 22 22 22 22 22			1557.80 1557.83	43		
1553.79	33	800	N	•	1556.62	22			1557.83	49		
1554.15	32 27				1556.65	28			1557.86	49		
1554.32	27				1556.69	29			1557.89	40		
1554.44	30				1556.73	29			1557.92	34		
1554.57	24				1556.76	28			1557.94	30		
1554.66	28				1556.79	<b>29</b>			1557.94 1557.97	58		
1554.70	25				1556.85	30			1557.99	56		
1554.77	28 25 31				1556.89	31			1558.02	26		
1554.82	31	600	N	8	1556.92 1556.95 1556.98	32	200 N	0	1558.04	26		
1554.86	26				1556.95	36			1558.07 1558.10 1558.14	24		
1554.97	33				1556.98	38			1558.10	24		
1555.02	27				1557.01	33			1558.14	24		
1555.09	28				1557.01 1557.04	28			1558.18	24		
1555.15 1555.29	33 27 28 22 23 28 29 31				1557.07	33 28 26 25 26			1558.22 1558.26 1558.30 1558.33	24		
1555.29	23				1557.10 1557.13	25			1558.26	24		
1555.35	28				1557.13	26			1558.30	24		
1555.50	29				1557.16	27			1558.33	24		
1555.67	31				1557.10	28			1558.36	24		
1555.84	38 38				1557.22	27			1558.40	24		
1555.90	28				1557.24	25			1558.44	24		
1555.97	30	488	N	0	1557.22 1557.24 1557.27 1557.31	28 27 25 22			1558.44 1558.48 1558.52	27		
1556.02	31				1557.31	24			1558.52	33		
1556.09	21				1557.34	31			1558.55	36		
1556.13	21				1557.37	32			1558.58	34		
1556.16	56				1557.40 1557.43	32 29 26			1558.58 1558.60 1558.63 1558.66	34 32 32		
1556.19	31 35 37 33				1557.43	26			1558.63	35		
1556.22	35				1557.47	24			1558.66	40		
1556.25	37				1557.50	28			1558.69	68		
1556.28	33				1557.54 1557.57	33 58			1558.72	87		
1556.31	27 22 21 20				1557.57	35			1558.72 1558.74 1558.77	162		
1556.34	22				1557.59 1557.62	34			1558.77	181		
1556.37	21				1557.62	33			1552.72	181		
1556.49 1556.43	20				1557.64	32			1558.80	179		_
1556.43	20				1557.67	32			1558.81	179	50 N	0
1222.4/	20				1557.71	34			1558.82	179		

Time EST	Sed.Conc	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc	Location N/S	(ft) E/U
1558.83 1558.86 1558.99 1558.94 1558.97 1558.98	170 142 82 45 36 29 27										
1558.99 1559.02	26	•	•								

## Suspended Sediment Data 9/19/78 Track 4 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	EVU (If)	Time EST	Sed.Conc mg/l	Location N/S	(lf)	Time EST	Sed.Conc mg/l	location N/S	(ft) E/W
1607.20 1608.14 1608.89	25 26 24	-200 S 0 200 N	0 0								

## Suspended Sediment Data 9/19/78 Track 5 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/W
1609.60 1609.63 1609.63 1609.67 1609.72 1609.73 1609.83 1609.83 1609.91 1609.94 1610.05 1610.05 1610.03 1610.30 1610.31 1610.31 1610.31 1610.31 1610.31 1610.31 1610.31 1610.31 1610.31	422222222223347726666674118522222222222222222222222222222222222	299 N		1611.34 1611.41 1611.47 1611.47 1611.50 1611.50 1611.51 1611.61 1611.61 1611.61 1611.72 1611.72 1611.72 1611.72 1611.73 1611.84 1611.84 1611.88 1611.81 1611.81 1612.00 1612.01 1612.12 1612.13	2179 23767466855275386322222244 5586275322222222222222222222222222222222222	-200 S	•	EST	mg/l	N/S E/W
1611.22	21	0	0							

## Suspended Sediment Data 9/26/78 Track 1 Depth is 50 feet

1042.01	
1042.05 21 1043.20 18 1044.47 13 1042.07 21 1043.23 17 1044.50 16	
1042.07 21 1043.23 17 1044.50 16	
1042.11 24 1043.26 16 1044.53 16	
1042.13 24 1043.29 17 1044.56 15	
1042.11     24     1043.26     16     1044.53     16       1042.13     24     1043.29     17     1044.56     15       1042.16     23     1043.32     17     1044.59     15       1042.19     20     1043.36     16     1044.62     16       1042.22     16     1044.65     16       1043.39     15     1044.65     16	
1042.19 20 1043.36 16 1044.62 16	
1042.22 16 1043.39 15 1044.65 16	
1042.25 15 1043.42 16 1044.69 16	
1042.28 18 1043.45 15 1044.72 16	
1042.31 22 1043.49 15 1044.74 16	
1042.37 21 1043.55 13 1044.80 14	
1042.40 23 1043.58 14 1044.85 14 1042.43 20 1043.62 14 1044.89 14	
1042.43 20 1043.62 14 1044.89 14	
1042.48 21 1043.65 11 1044.92 13	
1042.51 21 1043.69 11 1044.96 13 1042.53 23 1043.72 12 1045.00 12	
1042.48       21       1043.65       11       1044.92       13         1042.51       21       1043.69       11       1044.96       13         1042.53       23       1043.72       12       1045.00       12         1042.56       21       1043.75       12       1045.03       14         1042.59       21       1043.78       12       1045.08       13         1042.62       20       1043.83       12       1045.11       13	
1042.56 21 1043.75 12 1045.03 14 1042.59 21 1043.78 12 1045.08 13	
1042.59	
1042.62 20 1043.83 12 1045.11 13 1042.66 22 1043.85 14 1045.16 12 1042.69 20 1043.89 13 1045.20 12	
1042.66 22 1043.85 14 1045.16 12 1042.69 20 1043.89 13 1045.20 12	
1042.66 22 1043.85 14 1045.16 12 1042.69 20 1043.89 13 1045.20 12 1042.72 22 1043.91 15 1045.23 13	
1042.66     22     1043.85     14     1045.16     12       1042.69     20     1043.89     13     1045.20     12       1042.72     22     1043.91     15     1045.23     13       1042.75     19     1043.94     15     1045.27     13       1042.79     18     1043.98     14     1045.30     13	
1042.75 19 1043.94 15 1045.27 13 1042.79 18 1043.98 14 1045.30 13	
1042.79 18 1043.98 14 1045.30 13 1042.82 16 1044.02 13 1045.34 13	
1042.82     16     1044.02     13     1045.34     13       1042.85     15     1044.05     12     1045.37     14       1042.88     24     1044.09     13     1045.40     14       1042.91     17     1044.12     12     1045.45     14       1042.95     14     1044.15     15     1045.48     14	
1042.85 15 1044.05 12 1045.37 14 1042.88 24 1044.09 13 1045.40 14	
1042.88 24 1044.09 13 1045.40 14 1042.91 17 1044.12 12 1045.45 14	
1042.91 17 1044.12 12 1045.45 14 1042.95 14 1044.15 15 1045.48 14	
1042.95 14 1044.15 15 1045.48 14 1042.97 15 1045.70 14	
1943-99 15 1044.21 14 800 N 0 1045.80 17	
1043.03 16 1044.24 15 1045.87 14	
1043.06 16 1044.27 15 1045.92 16 1043.09 15 1045.97 13	
1043.12 14 1044.35 13 1046.05 13 1043.15 16 1046.19 15	
1043.12	

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc	Location N/S	(ft) E/U
1046.30 1046.32	13 18			1048.03	15			1051.30	9		
1046.37	10			1048.05 1048.06	12			1051.47	. 9		
1046.40	12	600 N	6	1048.09	21			1051.60 1051.65	11		
1046.49	13	500 11	·	1048.17	13 12			1051.70	12		
1046.55	13			1048.21	10			1051.80	10 11 8		
1046.62	13 13 16 16			1048.27	11			1051.87	Ŕ		
1846.70	13			1048.33	12			1051.90	15		
1046.76	16			1048.38	11			1051.95	10		
1046.82	16			1048.42	9 15 9 12 11 12 13			1052.05	9	-200 5	0
1946.84	18			1048.45	15						
1046.84	21			1048.46	9						
1046.88 1046.92	18 16 16			1048.50	49						
1046.96	16			1048.53 1048.57	16						
1046.97	18			1048.65	15						
1047.00	16			1048.71	13						
1047.03	16			1048.78	10						
1047.09	14			1048.86	11						
1047.13	16 16			1048.90	11 12 10						
1047.15	16			1048.95	10						
1047.20	13			1049.18	9						
1047.25	12 11			1049.38	10						
1047.30 1047.34	11			1049.52	10						
1047.37	11			1049.68	10						
1047.56	10			1049.78 1049.87	11						
1047.65	11 10 9 13 11			1050.02	11						
1047.69	īĭ			1050.17	ii	200 N	0				
1047.74	10			1050.35	10	200 11	v				
1047.77	14			1050.55	īě						
1047.80	11 12			1050.65	10						
1047.83	12			1050.80	10						
1047.85 1047.90	10	400 N		1050.93	10	0	0				
1047.95	12 10	400 N	0	1051.05 1051.15	11						
1047.95	12			1051.15	13 9						
1048.00	10			1051.20	11						

## Suspended Sediment Data 9/28/78 Track 1 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W
1144.60	51	800 N	0	1145.18	43			1145.81	45		
1144.63	39		•	1145.18	43 42			1145.83	44		
1144.65	37			1145.20	44			1145.83	43		
1144.66	37			1145.21	45			1145.84	44		
1144.69	37			1145.20 1145.21 1145.23	44			1145.86	43		
1144.69	37 38 36			1145.24	43			1145.84 1145.86 1145.87	47		
1144.72	36			1145.26	43 44			1145.88	44		
1144.73	39 38			1145.28 1145.29 1145.32	43			1145.89	42		
1144.74	38			1145.29	43 42 43 42 43 42			1145.91	42		
1144.74	40			1145.32	43			1145.91	42		
1144.76	39			1145.35	42			1145.91 1145.93	41		
1144.76	41			1145.38	43			1145.95	42		
1144.77	40			1145.41 1145.41 1145.44	42			1145.97	41		
1144.79				1145.41	44			1145.98	43		
1144.79	41			1145.44	40			1145.98	44		
1144.83	3 <mark>8</mark> 36			1145.47	43			1146.00	43		
1144.84	36			1145.48 1145.50 1145.52 1145.53	43 43 40			1146.02	44		
1144.86	37			1145.50	40			1146.03	45		
1144.88	40			1145.52	38			1146.03	45 48		
1144.91	41 42 42			1145.53	30			1146.04 1146.05	45		
1144.93	42			1145.55 1145.56 1145.58 1145.59	42			1146.05	47		
1144.94	42			1145.56	41 40			1146.05	49		
1144.94	44			1145.58	40			1146.09	44		
1144.95	49 46 45			1145.59	40			1146.11 1146.13 1146.13	41		
1144.98	46			1145.60	46			1146.13	43		
1144.98	45			1145.61	46 46 43			1146.13	46		
1144.99 1145.01	44			1145.62	46			1146.15 1146.16	44		
	47			1145.62 1145.63 1145.64	43			1146.16	45 45		
1145.01 1145.04	47			1145.64	44			1146.18 1146.19	45		
1145 05	42 41			1145.65	43 46			1146.19	47		
1145.05 1145.06	43			1145.67	46			1146.21	48		
1145.06	43			1145.67 1145.68 1145.70	47			1146.23	48		
1145.09	44			1145 72	44 40			1146.24 1146.24	48 45 45		
1145.11				1145.73 1145.74 1145.78 1145.78	44			1146.24	45		
1145.14				1145.79	42			1146.28 1146.30	42		
1145.15				1145.79	41				43 46		
1145.16				1145.80	44			1146.33 1146.37	41		
	•			10.00	77			1170.3	71		

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U
1146.38	41			1146.96	40			1147.75	37		
1146.39				1146.99	40 36			1147.78	35		
1146.39				1146.99	37			1147.80	42		
1146.43				1146.99	41			1147.81 1147.83 1147.84	40		
1146.46				1147.02	38			1147.83	36		
1146.48	40			1147.05	38			1147.84	36		
1146.49	42			1147.08	38 36 36 36			1147.86	36 35 38 37		
1146.49	42			1147.10	36			1147.88	35		
1146.53				1147.11	36			1147.88	38		
1146.54	38			1147.14	36			1147.89	37		
1146.55	39			1147.17	36 37			1147.89 1147.91	34		
1146.58				1147.19	37			1147.92	36		
1146.60	40			1147.23	37			1147.93	37 37		
1146.61				1147.26 1147.26	37			1147.94	37		
1146.62				1147.26	37			1147.96	36 36		
1146.63				1147.29	37			1147.98	36		
1146.65				1147.31	37 38 35 36			1148.00	38 39 37 37 37		
1146.66				1147.35	38			1148.00	39		
1146.67				1147.38	35			1148.03	37		
1146.69				1147.41	36			1148.04	37		
1146.71				1147.42	46			1148.08	37		
1146.72	40			1147.44	37			1148.10	37		
1146.74				1147.44	42 39			1148.13	38		
1146.75 1146.78	40			1147.46 1147.48	39			1148.15	38 39 40		
1146.78				1147.48	36			1148.15 1148.18	40		
1146.79				1147.48				1148.18	36		
1146.80				1147.49	40			1148.18	38		
1146.81	48			1147.53	37			1148.18	39 3?		_
1146.83				1147.56	36			1148.21	37	600 N	8
1146.84				1147.56 1147.59				1148.24	37		
1146.85				1147.62	37 36			1148.28	36		
1146.85	50				35			1148.30	39		
1146.87	44			1147.65	36 35			1148.33	40 41		
1146.89	39			1147.68 1147.68	37			1148.35 1148.38	40		
1146.90	39			1147.70	35			1148.41	40		
1146.93				1147.71	36			1148.43	75		
1146.95				1147.72				1148.46	42 42		

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (	(ft) E/U
1148.48	41			1149.16	44			1150.00	37		
1148.49	41			1149.16	44			1150.03	äĖ		
1148.53	41			1149.19	42			1150.06	36 36		
1148.56	41			1149.19 1149.21	44			1150.09	40		
1148.58	41			1149.24	45			1150.10	37		
1148.59	41			1149.27	44			1150.11	40		
1148.59	41 47			1149.29	44			1150.12	40		
1148.61	44			1149.33 1149.35 1149.39	44			1150.15	37		
1148.64	41			1149.35	44			1150.19	37 34		
1148.66	41			1149.39	44			1150.23	34		
1148.66	41 43 40			1149.43	43 43 45 46			1150.25	34		
1148.68	49			1149.44	43			1150.27	35		
1148.70	41			1149.46	45			1150.28	35 38		
1148.73	41			1149.47	46			1150.29	41		
1148.74	41			1149.47 1149.49 1149.51	45 46			1150.31	36		
1148.75	41 45 44			1149.51	46			1150.34	32		
1148.76	45			1149.54	43 44 46			1150.35	35		
1148.78	44			1149.54	44			1150.38	32		
1148.79	42			1149.57 1149.58 1149.60	46			1150.39	32 31 33		
1148.81	41			1149.58	44			1150.40	33		
1148.84	41			1149.60	43			1150.43	34		
1148.87	44			1149.64	42			1150.45	36		
1148.88	44			1149.67	41			1150.46	38		
1148.90	41			1149.68 1149.69 1149.70	41			1150.48	36 38 38		
1148.91	40			1149.69	42			1150.50	40		
1148.93	41			1149.70	44			1150.53	39		
1148.95				1149.72	43			1150.56	41		
1148.95	41			1149.74	41			1150.60	40		
1148.96				1149.74 1149.74	43			1150.63	41 42		
1148.98				1149.74	43			1150.65	42		
1149.00	44			1149.78	38			1150.68	43		
1149.03 1149.04	44			1149.83	37			1150.71	39		
1149.04	44			1149.83 1149.86 1149.87	43			1150.74	42		
	. 54			1149.86	37			1150.78	41		
1149.06 1149.09	55 46			1149.87	37			1150.79	40		
1149.12				1149.90	37			1150.81	37 35		
1149.13				1149.93	38			1150.84	35		
-173-13	73			1149.96	37			1150.88	35		

Time EST	Sed.Conc mg/l	Location N/S	(作) E/出	Time EST	Sed.Conc mg/l	Location N/S	(It) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/U
1150.90 1150.91 1150.93 1151.09 1151.09 1151.05 1151.10 1151.12 1151.13 1151.13 1151.13 1151.13 1151.13 1151.21 1151.21 1151.21 1151.21 1151.34 1151.34 1151.34 1151.34 1151.36 1151.61	mg/1 34 34 34 34 34 44 45 46 43 47 46 44 41 41 41 41 41 41 41 41 41 41 41 41				mg/l 37 38 31 39 39 39 39 39 39 39 39 39 39 39 39 39				mg/t 44 38 31 32 34 31 31 33 31 33 31 33 31 33 31 33 31 33 31 33 31 33 31	
1151.73 1151.75	35 36			1152.68 1152.72	33 31			1153.63 1153.66	31 31	

Time EST	Sed.Conc mg/l	Location N/S	(ft)	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/W	
1153.69 1153.72				1154.57 1154.58				1156.89	29	•	
1153.74	30			1154.59	38			1156.94 1156.97	58 58		
1153.78	31			1154.60	38 36 31			1156.98	34		
1153.80 1153.81	30			1154.63 1154.64	31			1157.03	34 29 31		
1153.81	30			1154.64	31			1157.05 1157.13 1157.16	31		
1153.83	33			1154.68	31			1157.13	29		
1153.84 1153.88	31 30 30			1154.70	31 32 32			1157.16	29 27		
1153.00	30			1154.73 1154.77	3E			1157.19 1157.21	27		
1153.89 1153.92	34			1154.80	30			1157.61	31 28		
1153.94	36 31 31 31			1154.80 1155.07	<b>88</b>			1157.24 1157.28 1157.29	30		
1153.96	31			1155.25	29	200 N	0	1157.29	29		
1153.98 1153.99 1154.02	31			1155.25	31			1157.30	29 31		
1153.99	31			1155.40 1155.56 1155.60	30			1157.31	29 28 31		
1154.04	33			1155.56	3 <b>8</b>			1157.34	58		
1154.08	33 30 31			1155.68	36			1157.34 1157.36 1157.40	31		
1154.12	31			1155.84	29 30 29 31			1157.42	29 30 28 29		
1154.15	30			1155.94	žš			1157.46	28		
1154.17	30			1155.94 1155.99	31		•	1157.49 1157.52 1157.54	29		
1154.20	30			1156.03	58			1157.52	30		
1154.22	30			1156.08	30			1157.54	28		
1154.24 1154.25	30 30 30 30 33 35 30 30			1156.17	29 30 29 28 30 28 31 29 31			1157.55	28 29 23 33 33 33 33 33		
1154.26	35			1156.24 1156.32	20			1157.58	29		
1154.28	30			1156.33	28			1157.63	31		
1154.28	30			1156.38	31			1157.61 1157.63 1157.65	33		
1154.31	30			1156.43	29			1157.66	33		
1154.34 1154.38	30			1156.44	31			1157.68	33		
1154.41				1156.47 1156.49	29 31			1157.68 1157.70 1157.74	33		
1154.44	30			1156.59	5 <i>q</i>			1157.74	39 4 <b>0</b>		
1154.46	30			1156.65	29 29			1157.76	39		
1154.46	35 36			1156.65 1156.74	29			1157.76 1157.79 1157.83	36		
1154.47	37			1156.80	59			1157.83	31		
1154.50				1156.81	31			1157.85	34		
1154.55	53			1156.83	30			1157.88	36		

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Tise EST	Sed.Conc	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/U
EST 1157.90 1157.91 1157.93 1157.94 1158.01 1158.01 1158.05 1158.08 1158.11 1158.14 1158.14	mg/l 34 35 42 40 37 35 33 32 33 33			1158.83 1158.84 1158.88 1158.88 1158.93 1158.95 1158.97 1159.00 1159.02	mg/l 29 33 37 34 31 29 30 31 29 32 37 38			1159.52 1159.54 1159.57 1159.59 1159.61 1159.62 1159.63 1159.63 1159.68 1159.68 1159.72	mg / l 32 30 29 29 28 30 30 30 29	
1158.21 1158.23 1158.28 1158.32 1158.38 1158.38 1158.43 1158.45 1158.52 1158.55	35 36 36 35 34 33 33 34			1159.09 1159.10 1159.13 1159.16 1159.16 1159.21 1159.24 1159.27 1159.29 1159.33	30 310 330 267 229 239			1159.74 1159.76 1159.81 1159.84 1159.86 1159.87 1159.89 1159.91	31 33 33 33 33 33 33 33 33 33 33 33 33 3	
1158.58 1158.61 1158.64 1158.66 1158.68 1158.71 1158.73 1158.75 1158.75	34 32 35 37 38 38 34 33 32			1159.36 1159.38 1159.38 1159.42 1159.42 1159.45 1159.48 1159.59	29 28 29 30 30 30			1159.98 1200.00 1200.03 1200.14 1200.17 1200.22 1200.24 1200.27 1200.31	299 287 277 288 289 290 327 28	

Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location	(ft)
EST	mg/l	N/S	E/U	EST	mg/l	N/S	E/W	EST	mg/l	N/S	E/U
1200.39 1200.42 1200.45 1200.54 1200.57 1200.60 1200.67 1200.77 1200.77 1200.81 1200.82 1200.82 1200.82 1200.82 1200.83 1200.82 1200.83 1200.82 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83 1200.83	28 29 29 29 28 28 28 27 28 29 29 21 21 21 21 21 21 21 21 21 21 21 21 21	-2 <b>99</b> S	•	1201.99 1201.99 1201.99 1201.99 1201.99 1209.04 1209.2.13 1209.2.13 1209.2.2.3 1209.	227088311109888889 <b>4</b> 7119908809882			1202.93 1202.96 1203.04 1203.09 1203.19 1203.28 1203.28 1203.33 1203.43 1203.47 1203.47	27779777088877087	- <b>499</b> S	9

## Suspended Sediment Data 9/26/78 Track 2 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W
1104.05 1104.05 1104.16 1104.16 1104.12 1104.22 1104.22 1104.31 1104.34 1104.34 1104.47 1104.50 1104.51 1104.57 1104.51 1104.81 1104.81 1104.81 1104.81 1104.81 1104.81 1104.81 1104.81 1104.81 1104.81	899999899999999999999999999999999999999	0 0 N	9	1105.28 1105.31 1105.37 1105.44 1105.47 1105.47 1105.51 1105.59 1105.62 1105.63 1105.77 1105.77 1105.89 1105.89 1105.89 1105.89 1105.89 1105.92 1105.92 1106.11 1106.17 1106.24 1106.27 1106.32 1106.35 1106.35 1106.35	mg 23209642245211001199220110211123791222076	N/S	E/U	1106.57 1106.63 1106.63 1106.63 1106.71 1106.71 1106.82 1106.82 1106.82 1106.92 1106.92 1107.03 1107.03 1107.11 1107.13 1107.35 1107.35 1107.35 1107.55 1107.55 1107.55	458987777652109036801201108855568557	N/5	€
1105.23	29			1106.52	13			1107.79	19 18		

Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location	(ft)
EST	mg/l	N/S	E/U	EST	mg/l	N/S		EST	mg/l	N/S	E/W
1107.87 1107.93 1107.93 1107.96 1107.96 1108.04 1108.08 1108.17 1108.17 1108.34 1108.31 1108.34 1108.47 1108.47 1108.57 1108.57 1108.63 1108.63 1108.76 1108.76 1108.98 1108.98	30 26 27 31 38 37 37 34 28	600 N	9	1109.07 1109.15 1109.18 1109.23 1109.29 1109.36 1109.36 1109.46 1109.56 1109.66 1109.66 1109.74 1109.66 1109.74 1109.79 1109.88 1109.88 1109.88 1109.89 1109.89 1109.95 1109.95 1109.95 1109.95	219887774443555555643336333333333333333333333	800 N	ð	1110.36 1110.44 1110.48 1110.53 1110.61 1110.64 1110.68 1110.72 1110.72 1110.86 1110.89 1110.93 1110.93 1110.93 1110.93 1110.93 1111.09	6286322210000220001100110011000110001100011	1990 N	8

## Suspended Sediment Data 9/26/78 Track 3 Depth is 50 feet

Time EST	Sed.Conc mg/l	location N/S	(fも) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W
1749.11	7	1000 N	0	1750.28	5			1751.59	10		
1749.14	4			1750.30	5 16	800 N	0	1751.61	16		
1749.17	8			1750.34	9			1751.65	- 7		
1749.19	11			1750.37	7			1751.68	7		
1749.23	5			1750.39	10			1751.72	8		
1749.26	10			1750.43	8			1751.75	16		
1749.29	.8			1750.45	11			1751.78	10		
1749.32	12			1750.49	10			1751.82	?	6 <b>0</b> 0 N	0
1749.35	11			1750.52	9			1751.84	9		
1749.38	7			1750.57	Ď			1751.88	. 8		
1749.41 1749.44	12 8			1750.60 1750.64	, 5			1751.90	11		
1749.47	13			1750.68	9 6 12 8 6 5 6			1751.94	11		
1749.50	13			1750.71	2			1751.97	10 8		
1749.53	16			1750.74	2			1752.01 1752.04	10		
1749.57	13			1750.79	ř			1752.08	4		
1749.60	14			1750.83	ă			1752.11	3		
1749.63	8			1750.86	ģ			1752.14	7		
1749.65	9			1750.88	2 5			1752.19	10		
1749.68	10			1750.91	11			1752.23	10 5 5 7		
1749.71	10 5 5 5 8			1750.93	17			1752.25	Š		
1749.74	5			1750.97	10			1752.28	7		
1749.78	5			1751.00	10			1752.31	11		
1749.80	. 8			1751.04	7			1752.34	7		
1749.83	11			1751.08	10			1752.37	13		
1749.86	10 5			1751.13	14			1752.40	5		
1749.89 1749.93	21			1751.16	12			1752.44	. 5		
1749.96	21			1751.19	9			1752.49	13 5 5 16		
1749.98	11			1751.22 1751.25	9 9			1752.52			
1750.01	iż			1751.29	8			1752.54 1752.58	19		
1750.05	16			1751.34	10			1752.60	7		
1750.08	9			1751.39	17			1752.63	11		
1750.10	13			1751.43	Ġ			1752.66	Ŕ		
1750.13	. 11			1751.46	10			1752.70	8 5 7		
1750.18	6			1751.49	11			1752.73			
1750.21	6 7			1751.52	10			1752.76	6		
1750.24	7			1751.55	6			1752.78	9		

· .

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/W
1752.83 1752.89 1752.99 1752.99 1753.05 1753.08 1753.11 1753.13 1753.20 1753.23 1753.23 1753.23 1753.29 1753.33 1753.35 1753.35 1753.59 1753.59 1753.64 1753.68 1753.68 1753.79 1753.79	56390068477809791276789011894844	N/S	e / <b>u</b>	1754.04 1754.08 1754.10 1754.13 1754.20 1754.23 1754.33 1754.33 1754.43 1754.43 1754.49 1754.55 1754.55 1754.61 1754.65 1754.73 1754.73 1754.74 1754.84 1754.84 1754.84 1754.89 1754.91	900088208678953119366879596063170	N/S	E/W			
1753.91 1753.94 1753.98	· 4 7 6			1755.06 1755.09 1755.13	8 5 8 10	200 N	0	1756.26 1756.30 1756.35	9 10 11	

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft)	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W
1756.40 1756.48 1756.59 1756.59 1756.65 1756.65 1756.68 1756.75 1756.80 1756.80 1756.80 1756.80 1756.91 1756.91 1757.00 1757.03 1757.20 1757.20 1757.20 1757.20	20 107 107 107 107 107 107 107 107 107 10	0	8	1757.59 1757.59 1757.60 1757.60 1757.669 1757.669 1757.884 1757.884 1757.99 1757.99 1757.99 1758.08 1758.12 1758.13 1758.13 1758.13 1758.33 1758.33 1758.48 1758.33 1758.48 1758.55 1758.67	98871530037800848011128830087813314946	-200 S	Ø	1758.88 1758.88 1758.88 1758.99 1758.99 1758.91 1758.91 1759.09 1759.16 1759.12 1759.13 1759.33 1759.31 1759.51 1759.51 1759.51 1759.61 1759.61 1759.74 1759.74	6687247609521988024999027079057177055	N/5	E/W
3.5	<del>-</del> ·			1758.72	4			1759.90	•		

					Depth 13	Se icer					
Time EST	Sed.Conc mg/l	Location N/S	(ft) E/H	T.188 EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc ag/l	Location N/5	(ft) E/U
1759.93 1759.95 1759.95 1759.97 1759.98 1890.00 1890.06 1890.12 1890.13 1890.13 1890.13 1890.25 1890.25 1890.37 1890.40 1890.43 1890.43 1890.43 1890.43 1890.43 1890.43 1890.57 1890.66 1890.66 1890.67 1890.68 1890.75 1890.83 1890.83	55299296576809127008441250465568892787	- <b>600</b> S	ð	1801.03 1801.06 1801.10 1801.11 1801.13 1801.16 1801.25 1801.25 1801.31 1801.35 1801.41 1801.45 1801.45 1801.56	99118317446847804729849	-8 <b>00</b> S	9				

## Suspended Sediment Data 9/28/78 Track 2 Depth is 25 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location (ft) N/S E/U
1210.80 1210.91 1211.03 1211.04 1211.14 1211.32 1211.54	221 221 221 221 220 220 220	-400 S	<b>.</b>	1216.80 1216.87 1216.94 1217.05 1217.29 1217.52 1217.66 1217.89	21 22 23 23 22 21 22 22 23 22 23 23 23 23 23 23 23 23 23	400 N	ð			
1211.77 1212.04 1212.29 1212.42 1212.56 1212.87 1213.00 1213.17 1213.31	20 221 220 221 221 221 221 221	-200 5	0	1218.04 1218.17 1218.30 1218.48 1218.69 1218.86 1218.94 1219.10 1219.33 1219.49	223 223 223 224 225 224 224 224	600 N	9			
1213.51 1213.67 1213.86 1214.09 1214.33 1214.56 1214.55 1215.45	21 21 20 20 21	9	0	1219.61 1219.76 1219.97	24 24 24 24	800 N	0			
1215.68 1215.91 1216.14 1216.23 1216.34 1216.40 1216.58 1216.68 1216.68 1216.75	21 21 22 22 22 22 22 22 22 22 23 23 23 23 23	200 N	8							

## Suspended Sediment Data 9/28/78 Track 3 Depth is 50 feet

Time	Sed.Conc	Locatio	n (fi	Time	Sed.Conc	Location	({{t}})	Time	Sed.Conc	Location	(ft)
EST	mg/l	N/S	E/L	EST	mg/l	N/S	E/W	EST	mg/l	N/S	E/W
1226.60 1226.70 1226.85 1227.03 1227.35 1227.60 1227.87 1228.03 1228.57 1228.79 1228.79	24 23 21 19 19 18 18 17 17 16 16	800 N	-200								

3

## Suspended Sediment Data 9/28/78 Track 4 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W
1231.13	18	600 N	200 E						-		
1231.47	18 17										
1231.76	17										
1231.98 1232.19	17										
1232.19	19										
1232.40 1232.62	19 19										
1232.81	19										
1233.05	19 18 18										
1233.28	18										
1233.52	19										
1233.28 1233.52 1233.86	19 19										
1234.10	19										
1234.38	19										
1234.58	19 18 20 20										
1234.80	20										
1234.97 1235.17 1235.50	5 <b>0</b> -										
1235.50	19										
1235.79	19										
1236.10	19										
1236.40 1236.55 1236.70	20										
1236.55	22										
1236.70	23										
1236.87 1236.96	53										
1237.05	<b>63</b>										
1237.15	23										
1237.20	21										
1237.2 <del>0</del> 1237.25	18										
1237.36	18										
1237.42	19										
1237.57	199 190 223 233 233 223 282 188 198 199										
1237.75	19										
1238.14	19										
1238.31	19	600 N -	200 N								

## Suspended Sediment Data 9/28/78 Track 5 Depth is 50 feet

Time	Sed.Conc	Location	t (ft)	Time	Sed.Conc	Location	(ft)	Time	Sed.Conc	Location	(ft)
EST	mg/l	N/S	E/W	EST	mg/l	N/S		EST	mg/l	N/S	E/W
1303.10 1303.13 1303.20 1303.20 1303.30 1303.30 1303.36 1303.55 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61 1303.61	330191112190222992929199679766688669978978	400 N	-290 W	1304.50 1304.65 1304.65 1304.69 1304.86 1304.95 1304.95 1305.06 1305.08	28 27 28 27 28 27 26 28 27 26 27 28 27 28 27 28 27 28 27 28 27 28 28 28 28 28 28 28 28 28 28 28 28 28	400 N	200 E				

## Suspended Sediment Data 9/28/78 Track 6 Depth is 50 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft		Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/W
1306.66	28	200 N	300		29			1310.08	30	
1306.68	27			1308.68	30			1310.08	28	
1306.71	28			1308.71	27			1310.14 1310.17	29 29	
1306.81 1306.88	27			1308.79	28			1310.17	29	
1306.88	28			1308.86	28			1310.19	59	
1306.92	29 29			1308.93 1308.95	29 29 30			1310.23	58	
1306.93	30			1308.96	58 36			1310.25	29 29 29 29 30	
1306.96				1309.01	58			1310.29 1310.33	29	
1307.02	20			1309.08	28			1310.37	29	
1307.03	28 29			1309.13	58 58 58 58			1310.41	33	
1307.07	<u> 28</u>			1309.19	29			1310.43	35	
1307.11	28			1309.25	28			1310.48	35 39 42 42	
1307.16	27			1309.31	58			1310.50	42	
1307.19	36			1309.36	29			1310.50 1310.51	42	
1307.28	58 59			1309.37	32			1310.54	45	
1307.39	58			1309.41	28 32 38 28 28			1310.57	46	
1307.48	28			1309.47	58			1310.60	46	
1307.55	29 28 27 27 28			1309.49	30			1310.62	46	
1307.68 1307.75	<u>ح8</u>			1309.49	31			1310.63	45	
1307.83	27			1309.53	59			1310.64	44	
1307.84	50			1309.58	28 29			1310.66	44	
1307.88	27			1309.62 1309.64	20 20			1310.67	43	
1308.04	ອີຊ			1309.68	29 28 28 29 29 28 29 27			1310.69 1310.71 1310.71	41 36	
1308.17	28 27 29 27			1309.69	28			1310.71	36 34	
1398.27	29			1309.72	28			1310.73	31	
1308.29	27			1309.74	29			1310.74	31 31 29 29	
1308.34	27			1309.74 1309.77	27			1310.77	29	
1308.36	30			1309.78	31			1310.77	29	
1308.36	58			1309.82	27			1310.80	39	
1308.39				1309.84	29 27			1310.81	42	
1308.42				1309.86	27			1310.83	47	
1308.48 1308.53				1309.93	28 29			1310.84	51	
1308.53	58 53			1309.93 1309.98	58 58			1310.87	55	
1308.64	58			1309.98	58 53			1310.88	58	
1308.65				1310.00	29			1310.90 131 <b>0.</b> 92	57 56	
				-3.0100	L-3			1314.36	30	

Time EST	Sed.Conc mg/l	Location N/S	(It) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U
1310.94	<b>56</b> 55			1311.79				1312.69	44		
1310.95	55			1311.80				1312.72	43		
1310.97	54			1311.84	30			1312.73	44		
1310.98	53 55			1311.86	30			1312.79	44		
1310.99 1311.00	56			1311.87 1311.89	31			1312.83	42		
1311.04	50			1311.99	30 30			1312.84 1312.87	41		
1311.05	52 52			1311.93	30			1312.87	41		
1311.08	57			1311.95	30			1312.89	42 42		
1311.11	ĕė			1311.98	31			1312.92	39		
1311.14	65			1311.98 1312.00	31			1312.92	41		
1311.17	64			1312.01	31			1312.97 1312.99	37		
1311.19	69			1312.04	30			1313.02	36		
1311.22 1311.24	69			1312.07	32			1313.04	34		
1311.24	62			1312.09	30 32 35 36			1313.07	34		
1311.24	59			1312.11	36			1313.09 1313.12 1313.15	34 33		
1311.27	51			1312.14	39			1313.12	32		
1311.30	46			1312.17	46			1313.15	31 32 31 32 32		
1311.32 1311.33	44			1312.22 1312.22 1312.24	43			1313.18	32		
1311.33	44			1312.22	41			1313.21	31		
1311.37	45 45			1312.25	38			1313.25 1313.28	35		
1311.41	42			1312.28	35			1313.28	32		
1311.43	3 <b>9</b>			1315.50	28 28 28			1313.31 1313.33	31 32 32 33 33		
1311.46	36			1312.30 1312.33 1312.35	29			1313.33	3E		
1311.46	35			1312.35	28			1313.33	32		
1311.49	31			1312.38	28			1313.35 1313.39 1313.42	33		
1311.50 1311.53	35 31 30 29 29 29 30			1312.43	28			1313.43	33		
1311.53	29			1312.46 1312.49 1312.50	30			1313.45	34		
1311.55	29			1312.49	31			1313.49	35		
1311.58	59			1312.50	31			1313.53	34 35 36		
1311.59 1311.63	30			1312.53	35			1313.56	37		
1311.65	30			1312.55 1312.56	34			1313.59	36 35 37 38		
1311.69	29 29			1312.56	38			1313.62 1313.65	35		
1311.72	30			1312.69	41 41			1313.65	37		
1311.73				1312.62	44			1313.75	38 39		
1311.76				1312.66	44			1313.80	41		

Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/5	(ft) E/U	Time EST	Sed.Conc mg/l	Location (ft) N/S E/W
EST  1313.84 1313.88 1313.90 1313.93 1313.95 1314.04 1314.05 1314.17 1314.22 1314.25 1314.30 1314.30 1314.31	mg/l 39 49 38 377 39 444 446 446 446 446 446 446 446 446 44			EST  1315.03 1315.06 1315.12 1315.17 1315.21 1315.27 1315.31 1315.35 1315.36 1315.36 1315.49 1315.49 1315.55 1315.57 1315.59				Time EST 1315.96 1315.98 1316.00		
1314.50 1314.60 1314.67 1314.71 1314.72 1314.77 1314.79 1314.82 1314.82 1314.87 1314.92	50 49 44 47 48 48 48 46 45 47 47 47 50			1315.65 1315.65 1315.70 1315.70 1315.70 1315.70 1315.77 1315.79 1315.80 1315.84 1315.87 1315.81 1315.91	64 63 68 55 55 55 55 54 53 54 45 42					

## Suspended Sediment Data 9/28/78 Track 7 Depth is 25 feet

Time EST	Sed.Conc mg/l	Location N/S	(ft) E∕U	Time EST	Sed.Conc mg/l	Location N/S	n (ft) E/W	Time EST	Sed.Conc mg/l	Location (ft) N/S E/U
1320.80	24	200 N -	300 U	1322.03	26 29				_	
1320.84	24			1322.07	26					
1320.87	25 24 24 24 23 27			1322.07 1322.12	24					
1320.91	24			1322.15	24					
1320.97	24			1322.17	24					
1320.99	24			1322.22	24					
1321.03	23			1322.25	23					
1321.05	27			1322.29 1322.32	24 23 21					
1321.08	24 22 21 21			1322.32	21					
1321.12	22			1322.34	23					
1321.16	21			1322.37	21					
1321.19	21			1322.40	21					
1321.22	21			1322.44 1322.47 1322.51 1322.56	21					
1321.25	21			1322.47	22					
1321.28	21			1322.51	21 22 22 23 21					
1321.32	21			1322.56	25					
1321.34	21 21			1322.59	21					
1321.37	21			1322.63	21					
1321.40	21			1322.66 1322.68 1322.71	20 20 20					
1321.45	21			1322.68	20					
1321.48	21			1322.71	20					
1321.51	21			1322.74	20					
1321.54 1321.58 1321.61	21 21			1322.77	21					
1321.58	21			1322.82	21 21					
1321.61	21			1322.86	21					
1321.65	21			1322.89	20					
1321.68	21			1322.86 1322.89 1323.10	20 21					
1321.72	Ž1			1323.28	20					
1321.76 1321.78	21			1323.34	20					
1321.78	22			1323.46	20					
1321.82	25			1323.54	21					
1321.84	21 21 22 25 25 25 25 25 25 25 25 25 25 25 25			1323.46 1323.54 1323.68	20					
1321.87	25			1323.84	19					
1321.90	25			1323.92 1323.97 1324.05	19					
1321.93	24			1323.97	21					
1321.96	25			1324.05	21					
1321.98	26			1324.11	19	200 N	300 E			
1322.01	28									

## Suspended Sediment Data 9/28/78 Track 8 Depth is 25 feet

					p						
Time EST	Sed.Conc mg/l	Location N/S	\ (ft) E/U	Time EST	Sed.Conc ag/l	Location N/5	(ft)	Time EST	Sed.Conc mg/l	Location (ft)	)
1327.	57 18	600 N	300 E								
1327.	78 19 90 18 97 19 25 19 57 18	000 11									
1327.9	90 18										
1328.0	97 19										
1328.	25 19										
1328.	57 18										
1328.	91 2 <b>0</b>										
1329.											
1329.	47 20										
1329.	78 20										
1330.											
1330.	50 21										
1330.	97 21										
1331.	36 61										
. 1331.° . 1332.°	(8 E1										
1332.											
1332.	70 20										
1332.	70 20 95 20 37 20										
1333.	37 26										
1333.	64 20										
1334.	95 19										
1334.	41 19				•						
1334.0	69 19										
1334.	99 18										
1335.	21 19										
1335.	<b>40</b> 18	6 <b>09</b> N -	-300 H								

#### Suspended Sediment Data 9/28/78 Track 9 Depth is 50 feet

					55ptn 15						
Time EST	Sed.Conc mg/l	Location N/S	(ft) E/U	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W	Time EST	Sed.Conc mg/l	Location N/S	(ft) E/W
1736.50	12	200 N	9								
1736.84	า้อั	200 11	•								
1737.26	12 12										
1737.68	īī										
1738.81	iż	0	9								
1738.96	ič	•	v								
1739.12	16 19										
1739.15	16										
1739.18	17										
1739.24	îģ										
1739.27	18 15 16 15										
1739.32	16										
1739.36	16										
1739.39	iš										
1739.55	13	-200 S	0								
1739.61	iš	LUU J	v								
1739.67	îĔ										
1739.69	15 14										
1739.85	iš										
1739.99	17										
1740.08	15										
1740.29	13										
1740.47	15 13 12										
1740.62	ii										
1740.75	ii	-400 5	0								
29.15	-1	- 700 3	v								

Near Bottom Currents in the Lower James and Elizabeth Rivers

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Near Bottom Currents in the Lower James and Elizabeth Rivers

A knowledge of currents in the Lower James and Elizabeth Rivers has been of interest for the longest time to the commercial and military shipping interests. This may be illustrated by the events which led to the historic naval engagement between the USS Monitor and the CSS Virginia in Hampton Roads.

A systematic survey of the currents in this region, undertaken as part of a comprehensive regional survey by the US Coast and Geodetic Survey, was reported by Haight, et al. (1930). While comprehensive in areal extent, this survey, responsive to the needs of port operation, dealt primarily with surface currents in the region of interest for the present study. The 1930 results are further compromised, for the present study, by the substantial alteration of the dredged channels since that time.

After the channel was dredged, the Coast and Geodetic Survey again measured the currents, this time at several depths, in 1951. Between then and now, the construction of the Craney Island Disposal Area again changed the current patterns in the study area. These changes are noted by Neilson and Boulé (1975).

The significance (and difficulty) of current measurements in the study region is illustrated by noting that, in 1951, the first technical report produced by the newly formed Chesapeake Bay Institute (Pritchard and Burt, 1951) was titled

"An inexpensive and rapid technique for obtaining current profiles in estuarine waters". This report, an indication of the agenda of the new institution, introduced a biplane current drag, subsequently called the Pritchard drag. In operation, the drag is manually deployed over the side of an anchored vessel, the current being related to the angle from the vertical caused by the current pulling the drag to the side while a weight tends to return it to the vertical. It is of interest, from the perspective of nearly thirty years, that the qualities of low cost and rapidity of operation were emphasized in the title while those of accuracy and precision were not so emphasized.

A study, named Operation Oyster Spat, was quickly initiated using the new device, and the data from station J-17, located in the main channel just to the south of Burwell Bay, have become famous as the prototype mean flow pattern for partially mixed estuaries. Another station from Operation Oyster Spat was located upriver of J-17, at Deep Water Shoal, the upper limit of oyster production in the James.

A subsequent study, Operation James River (Shidler and MacIntyre), was performed 13 years later by VIMS and other cooperating organizations. This study was conducted after the Craney Island Disposal Area had been built, and the currents in the lower part of the study area were shown (Neilson and Boulé, 1975) to have been shifted to the north by the construction. In contrast to the earlier study by Chesapeake

Bay Institute, Operation James River concentrated on obtaining a wide spatial coverage of the lower James River with short time series rather than long series at a few locations.

A further study using current meters in the James River part of the study area was undertaken by the U.S. Army Corps of Engineers in the support of the calibration for the Chesapeake Bay Model. For this study (Ruzecki and Markle, 1974) current meters were placed at several depths at four river transects in the study area: at the mouth of the James, at the upper limit of the Newport News Shipyard, near the downstream part of Burwell Bay and off Hog Point. Ten stations were occupied in these transects, the ones at the mouth for a period of 19 days and the others for periods of about four days each. In total thirty-three current meters were deployed at these stations. Some of the data have been analyzed (Lewis, 1975) at the downriver transects to obtain tidal constituents.

In June of 1972, the entire Chesapeake Bay watershed, including the James River, was inundated with rains from tropical storm "Agnes". As part of a massive study to examine the effects of this flooding, current meters were again set in the study area, occupying the transect off of the Shipyard (Jacobson and Fang, 1977) for a period of eight days.

From the standpoint of maintenance dredging, the Elizabeth River is much more important relative to the James River than its areal extent would suggest, for the majority of the Elizabeth has been dredged to a substantial depth. One result

of the channel depth and the short length of the Elizabeth is that the gravitational flow, which results in estuarine circulation in longer estuaries with greater fresh water inflow, becomes a rapid adjustment of the stratification in the Elizabeth to that of the James and to rainfall events (Neilson, 1975). Thus, circulation in the Elizabeth River is expected to be primarily tidal, augmented with events of two layer circulation consisting generally as an intrusion of salty water from the James upriver in the Elizabeth along the bottom. As the two layer circulation occurs in distinct events it may or may not be evident in any particular set of current records obtained in the Elizabeth River.

Several sets of current data have been obtained in the Elizabeth River over the years. A set of four stations was occupied in 1974 for a period of two and a half days, the stations located in the main stem and each of the three branches of the river. The 12 current meters used in this study were deployed at depth increments of about six feet with the uppermost instrument at a depth of six feet (Cerco and Kuo, unpublished ms.). The U.S. Navy has obtained several sets of current meter records in the part of the Elizabeth adjacent to the Norfolk Navy Base. One of these sets (S. Jenkins, Scripps Institute of Oceanography) resides at the Scripps Institution of Oceanography. Another (Ruzecki and Ayres, 1974) had current meters located near the bottom on both sides of the ship channel of the Elizabeth River close to its junction with the James

River for a period of about 20 hours under conditions of low river flow and spring tides. A third set of current meters was deployed in conjunction with the present investigation near the Craney Island landfill site for a period of 28 days with meters at depths of 3, 6, 12 and 15 meters during September and early October of 1978. As these last data have not yet been finally analyzed, they are not included in the interpretation.

One of the experimental constraints with current meter measurements is that strings of current meters cannot be placed in shipping channels, because they will be destroyed by the shipping traffic. The string of meters placed in the Elizabeth for the present study was placed at the edge of the shipping channel, and it was still damaged by shipping. result of this constraint, there are few direct measurements of currents in the middle of shipping channels. The writer knows of only one current transect obtained in the Newport News Channel. That transect has never been published, as it was ancillary to a larger experiment, and the current meters were not ever calibrated. The data do show, however, that the current in the transect reached a local maximum speed (during both flood and ebb) within the dredged channel just below the level of the surrounding river bottom. instances, the current speed at this maximum was about the same as that at the surface.

Another method of measuring currents, droqued buoys, has also been employed in the study region with some success. This method does not produce long time series, but it can be applied in the channel areas where current meters are in jeopardy. It is also compatible with a simultaneous description of currents over a wide area, such as the entrance channel of the Elizabeth River or the breadth of Hampton Roads. Using droqued buoys, surface current data have been obtained within the Elizabeth River and Hampton Roads in several projects associated with sewage effluents, (Neilson and Boulé, 1975; Welch and Neilson, 1976); bridge tunnel construction, (Fang, et al., 1972; Fang, 1979); and port facility siting efforts, (Fang, 1975). In the Elizabeth River, the surface data gathered from these various efforts have been compiled into a single Elizabeth River Circulation Atlas, (Munday et al., this report) which segregates surface current patterns by tidal phase and wind velocity classes. Another use of drogued buoys was made in the Elizabeth River directly in support of the present effort. A cross-sectional velocity estimate was constructed from droqued buoy data in a region crossing and including the main ship channel of the Elizabeth River. This estimate is of significance beyond this project because it is the first synoptic cross sectional current velocity determination which has been made entirely using drogued buoys located by remote sensing in a concurrently occupied shipping channel. It has been reported as such by Munday, et al (1980) in its context

as a new technique which is applicable to current determination in busy port areas.

A number of current studies have been performed in the region of interest. Even with these studies, little direct evidence exists for formulating estimates of currents near the bottom of dredged channels, the focus of interest for the present study. For this reason, the formulation of the estimate for current speeds at the bottom of dredged channels in the study area will be based partly on indirect measurements and inferences. The remainder of this report is concerned with these estimates for the Elizabeth River, the Newport News Channel, and the Rocklanding Shoals Channel, the major dredged channels in the study area.

## Elizabeth River Current Calculation

The Elizabeth River is complex in its geometry, but it also is relatively short. The National Ocean Survey Tide and Tidal Current Tables show time differences between tidal height and tidal currents as they propagate down the 24 kilometer length of the deep channel. The time of high tide, according to these tables, is within 15 minutes of being simultaneous at all stations, while tidal currents reach slack water about 30 minutes or less after slack water at the river mouth, near Craney Island. In addition, the typical tidal ranges at all stations are within 10% of those at Sewell's Point, at the mouth of the Elizabeth River. As all of these time differences are small with respect to the 12.42 hour (745 minute) semidiurnal tidal period, an estimate of the tidal currents can

be made using tidal prism calculations, which are based on the assumption that the water surface in the Elizabeth River is at all times a level surface, implying that slack currents occur simultaneously with extreme tidal heights and that extreme tidal heights are simultaneous and equal throughout the basin. Because of this assumption, the current estimates will be made only in the enclosed part of the Elizabeth River, that portion south of the outer levee of the Craney Island Disposal Area (36°55'27"N).

Under this assumption, the volume of water which passes through any cross-section of the river equals the product of the surface area upriver from that section and the change in water level. If the water levels considered are successive tidal height extremes, the volume is called the intertidal volume. Ιf the intertidal volume of water above a chosen cross section is assumed to be supplied by water moving through the section during the rising and falling tide, a cross-sectional average flow speed can be calculated for the tidal phase (rising or falling tide). The peak speed averaged over the cross section during a tidal cycle is  $\pi/2$  times this average flow speed under the assumption that the speed describes a half-sinusoid between successive times of slack water (or height extremes). volumetric calculation is available which permits calculation of cross-sectional average flow speeds (and peak speeds) from a consideration of surface areas and cross-sectional areas in

a short estuary of complex geometry. This calculation has been used to estimate currents in the Elizabeth River.

If a distance scale (x) is defined extending from the head of each tributary and the main course of the river, an incremental surface area dA(x) can be defined so that the total area upriver of a given point,  $x_0$ , is  $A(x_0) = \int_0^{x_0} dA(x) + \sum_{i=1}^{x} T_i$ , where  $T_i$  is the total surface area of the i<sup>th</sup> tributary entering the river above  $x_0$ . If  $A(x_0)$  is relatively independent of water level, corresponding to nearly vertical banks, the total volume of water entering the river above a cross-section at x is  $A(x_0) \Delta H$ , where  $\Delta H$  is the change in water level. With the river cross-sectional area at  $x_0$  denoted as  $C(x_0)$  and the time difference between the two water levels denoted as  $\Delta t$ , the flow velocity averaged over the cross section and the time interval becomes  $\overline{v}(x_0) = \frac{A(x_0)}{C(x_0)} \frac{\Delta H}{\Delta t}$ . In our flow calculations, the quantity  $\frac{A(x)}{C(x)}$  is evaluated for a set of chosen cross sections, and  $\frac{\Delta H}{\Delta t}$  is evaluated for each of the intervals between tide height extremes during the year 1975, an arbitrary year presumed to be typical, the values being grouped and presented as a cumulative frequency curve.

The Elizabeth and its tributaries were subdivided into 26 segments according to the scheme used by Cerco and Kuo (unpublished ms.), and the mean low water areas were measured for each segment. The measurements were made from National Ocean Survey charts 12245 and 12253, which together cover the entire tidal extent of the Elizabeth and its tributaries at a

scale of 1:20,000. Because the basin does not possess extensive marsh areas, indeed is substantially bordered by vertical bulkheads, the areas measured were applied to the entire tidal range for current computations. The areas are shown in table 1, and the segmentation scheme is shown in figure 1. To calculate tidal heights, tidal predictions from Sewell's Point were gathered for the year 1975, and values of  $\Delta H/\Delta t$  were calculated for each tidal cycle, segregated into rising and falling tides. As the two cumulative frequency curves are nearly identical for Sewell's Point, we present only a single curve in figure 2. Selected percentile values are presented in table 2. mean value is  $3.48 \times 10^{-3}$  cm/sec, while the median is  $3.40 \times 10^{-3}$  $10^{-3}$  cm/sec. Mean current speeds for each cross-section are also shown in table 1. The calculated mean cross-sectional values are shown, with other information, on figure 3 as a set of line segments connecting calculated points.

Verification of the data can be done with comparison to other work. The areal measurements are compared with previous work by Cronin (1972). The mean current speeds are verified by comparison with a drogued buoy cross-sectional current determination done specifically for the present effort.

In comparing areal measurements of the rivers, allowance must be made for the difference in river mouth locations between Cronin (1972) and Cerco and Kuo (unpublished ms.). If this is done by using Appendix A in Cronin (1972), and the value for the Lafayette River is added to that of the Elizabeth

Table 1. Measured Segment Areas and Mean Speed Calculation for the Elizabeth River

Segment	Area (x10 <sup>5</sup> m <sup>2</sup> )	Cumulative Area (x10 <sup>5</sup> m <sup>2</sup> )	Downriver Bounding Cross-Section Area (x 10 <sup>3</sup> m <sup>2</sup> )	Multiplier (x 10 <sup>2</sup> )	Mean Current Speed (cm/sec)
2	5.84	5.84	0.53	11.01	3.8
3	5.73	11.57	0.43	26.91	9.4
4	5.91	17.48	0.57	30.67	10.7
3 4 5 6 7	14.02	31.50	0.69	45.65	15.9
6	8.80	40.30	1.79	22.51	7.8
7	8.52	48.82	3.25	15.02	5.2
8 9	8.18	57.00	3.08	18.51	6.5
9	10.82	67.82	2.52	26.91	9.4
10	9.99	77.81	2.78	27.99	9.8
11	4.96	82.77	4.78	17.32	6.0
12	11.24	141.13*	6.28	22.47	7.8
13	29.03	170.16	11.78	14.44	5.0
14	42.30	271.18*	9.13	29.70	10.4
15	46.54	317.72	10.20	31.15	10.9
16	39.19	415.42*	11.52	36.06	12.6
17	46.88	462.30	14.56	31.75	11.0
E2	19.64	19.64	1.46	13.45	4.7
E3	16.38	36.02	2.00	18.01	6.3
E4	11.10	47.12	3.52	13.39	4.7
W2	15.93	15.93	0.84	18.96	6.6
W3	23.06	38.99	1.36	28.67	10.0
W4	19.73	58.72	2.09	28.10	9.8
Ll	17.73	17.73	0.99	17.91	6.2
L2	18.33	36.06	0.69	52.26	18.2
L3	22.45	58.51	0.86	68.03	23.7

<sup>\*</sup>Includes tributary contribution

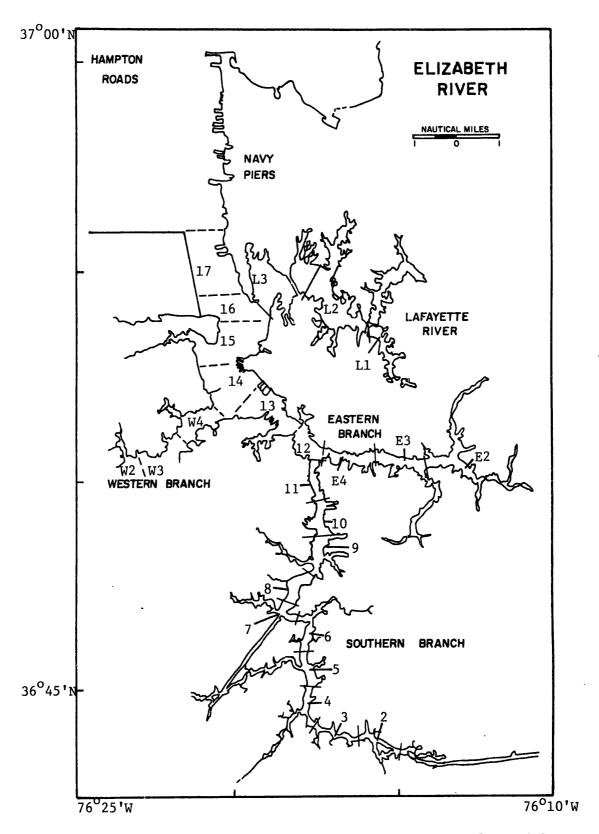


Figure 1. Segmentation of the Elizabeth River basin for tidal prism calculations (after Cerco and Kuo, unpublished ms.).

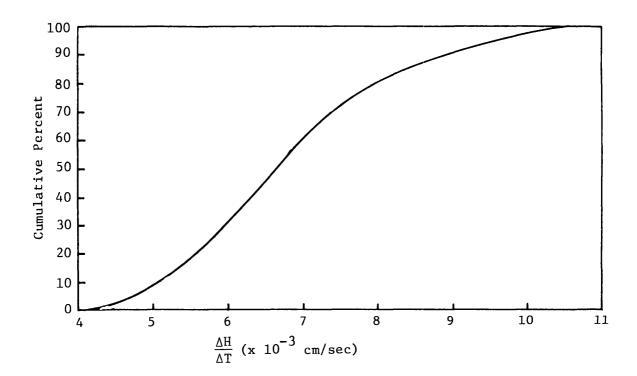
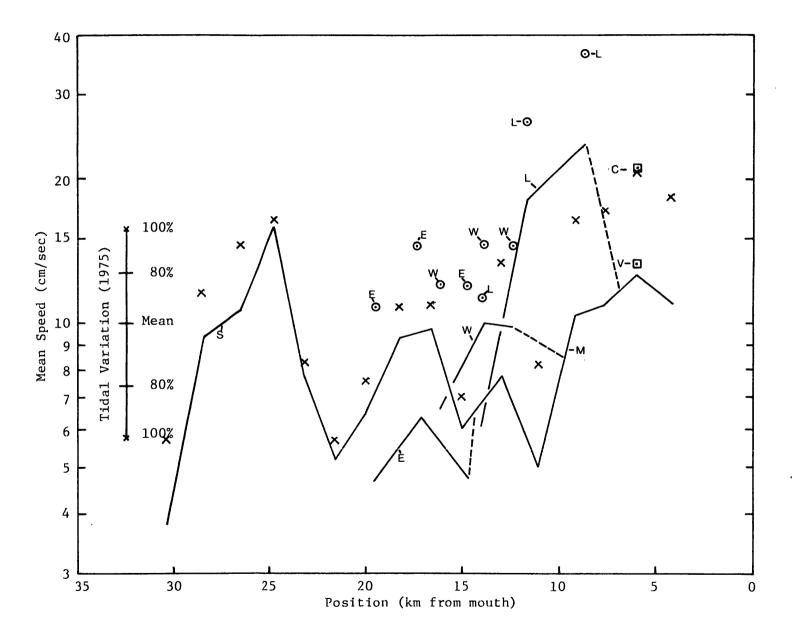


Figure 2. Cumulative rates of average predicted height change over a half tidal cycle at Sewell's Point, Hampton Roads, Virginia.

Table 2. Percentage Points for Predicted Average Height Change Over a Half Tidal Cycle at Sewell's Point, Hampton Roads, Virginia. Sample Period is 1975.

Percentage of Occurrences Less	Than	Level
.1		4.0
.5		4.3
1		4.5
1 2 5		4.6
		4.9
10		5.2
15		5.4
20		5.6
25		5.8
30		6.0
. 35		6.2
40		6.4
45		6.6
50		6.7
55		6.8
60		7.0
65		7.2
70		7.4
75		7.7
80		8.0
85		8.5
90		8.9
95		9.5
98		9.8
99		10.2
99.9		10.8



River, the total area for the river from our measurements becomes  $566.08 \times 10^5 \text{m}^2$  while that from Cronin (1972) is  $518.8 \times 10^5 \text{m}^2$ . The resulting difference amounts to 8% of our measured value. The estimated accuracy of the present area measurements is 1%, so a real discrepancy exists between the two sets of measurements.

A further comparison was made between the transport predicted by the tidal prism measurements and that measured (Munday, et al., 1980) for that purpose on September 19, 1978. The verification measurements were made near the outer boundary of section 16 as defined by Cerco and Kuo just north of Tanner The resulting interpolated velocity section (fig. 4) was planimetered for areas between each 5 cm/sec isotach neglecting the deep area towards the right of the section, which is part of a berthing area surrounded by piers, and plausibly has little transport. The areas measured are bounded by the dashed line with the dotted extension, the solid line where there is no dashed line, and the free surface. This measured cross-sectional mean speed was 22.5 cm/sec. To compare with mean speeds shown in figure 3, this value was multiplied by the ratio of mean  $\Delta H/\Delta t$  to that calculated for the time of the measurements using Sewell's Point tide station observations. It was again corrected for the time within the tidal cycle (estimated as 105 minutes before high slack water) of the measurements under the assumption of a sinusoidal height variation with time. The resulting mean speed value, 13.3 cm/sec, is shown in figure 3 and is comparable to the value of

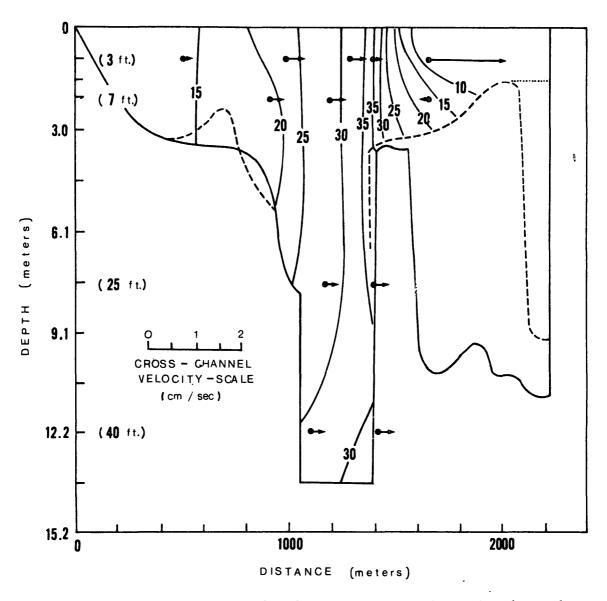


Figure 4. Synoptic flood tidal velocity cross section using drogued buoys and photogrammetry on September 19, 1978. Section is located within segment 17 looking towards the river mouth. Isotachs have units of cm/sec.

12.6 cm/sec obtained from tidal prism calculations. The two average cross section currents agree to within 6% for this verification. This agreement is well within the limits of experimental accuracy.

A final comparison is shown in figure 3 between mean current speeds from the tidal prism calculations and speeds calculated from amplitude values derived from the current meter measurements of Cerco and Kuo. These latter are shown as x's on figure 3 for the main stem and southern branch of the Elizabeth River, and as circled points for the other tributaries. Comparing the two sets of values, agreement is relatively close (<15%) in the middle part of the main and southern branch segment, but it is reduced towards the mouth and in the smaller tributaries, the Cerco and Kuo values being systematically higher than the tidal prism calculations by from 20 to 100%. Because these values were obtained by current meters located in or near the central channel, the hypothesis was formulated that the current meter data were obtained in a rapidly flowing part of the river and that the average speed was smaller than the measured speed in places where the channel occupied a relatively small part of the cross section. To test this hypothesis, the mean speed from the prism measurements for section 16 was multiplied by the peak-to-mean speed ratio from the cross section in figure 4,  $(37.5 \text{ cm-sec}^{-1}/22.5 \text{ cm-sec}^{-1})$ = 1.67). The resulting value, 20.8 cm/sec, was within 2% of the mean value of 20.4 obtained from Cerco and Kuo's results.

This supports the hypothesis that the current meter values were associated with high velocity cores in the cross-sectional flow. If the flow pattern in figure 4 is typical, it is the high speed core value which is most appropriate to the nearbottom part of the channels, where maintenance dredging is needed. This finding is similar to that obtained from the current profile from the Newport News Channel disclosed earlier.

For the Elizabeth River, then, the speed values associated with the Cerco and Kuo current meter results, denoted by X's in figure 3, are our best estimate for values of currents in deep channels for which maintenance dredging is required. These values are mean values, and the variability due to varying astronomical tides is given by the range bars in figure 3.

Newport News Ship Channel Current Calculation

beth River, while applicable to small enclosed basins with little freshwater flow, is not suitable for calculations in the main stem channels of the James River. The major reasons are that the tidal propagation in the main stem of the James has to a large extent the character of a propagating wave, and so the tidal prism estimating technique must be modified. Also, the James has current associated with river flow and an estuarine circulation which is not accounted for in the tidal prism method. On the other hand, the Newport News Ship channel has a relatively

uniform width and project depth along its length, so currents can be plausibly supposed more nearly uniform along its length than in a confined port area. Accordingly, it seems reasonable to apply data from a small number of current meters to the entire length of the Newport News Ship Channel, while such generalization is not supported in the Elizabeth. The basis for the estimate in Newport News Ship Channel is current meter data.

Current meters are sensitive to the vector sum of currents from all causes. If we have available time as an independent variable, a current record can be decomposed into a mean value, representing river flow, estuarine circulation and the mean of weather events during the period of record, an oscillatory tidal signal, representing the major current component in the region of interest, and a time-varying flow due to storm surges, local wind response and other weather related events as Kiley (1980) has done in the York River. Under these conditions, the best estimate of mean currents is the mean non-tidal value for the record. Also, the best estimate of variability from non-tidal currents is the nontidal variability of the record. The tidal variability is obtainable from the predictions or an astronomical tidal forcing function, and measured tidal variability can be biased to provide an improvement over the record data itself by taking the regularity of the tides into account. Currents from short term VIMS moorings have been treated this way by Lewis (1974), and Boon and Kiley (1978) report another method using least

squares fits for longer period data. Both of these methods are useful for segregating the total time series into tidal and non-tidal parts, with determinations of astronomical tidal constituents as at least part of their result. Both of these approaches require computers to be practically implemented, although one (Boon and Kiley, 1978) can be performed with a calculator and a special set of auxiliary tables. Both of these methods also require a regularly spaced time series of current measurements as input.

Another analysis method to estimate mean currents variability has been developed for the present estimate for which a hand calculator and tidal height tables are sufficient, particularly if the estimate to be made is near a primary tidal station, such as that at Sewell's Point. For this method, the times and speeds of current maxima are obtained from the record, and corresponding values for  $\Delta H/\Delta t$  are calculated from the tide tables. The current values are then linearly regressed on the  $\Delta H/\Delta t$  values, and the mean value of peak current is obtained from the long term mean value of  $\Delta H/\Delta t$ , already developed for Sewell's Point in the Elizabeth River calculation.

In the present instance, only one of the previously noted current studies, that of the Coast and Geodetic Survey in 1969 (DeRycke, unpublished data), actually deployed current meters within the deep channel of the Newport News Ship Channel. Two of these stations were located in the channel itself, one (station 2) at the eastern end and one (station 3) at the channel

edge in mid channel. These were both occupied for about 15 days with Roberts Radio current meters, with occasional comparison readings made using drogued buoys. The times, speeds and directions for the flood and ebb current peaks obtained from this data are shown in Appendix 1 along with corresponding values of  $\Delta H/\Delta t$  (in feet/minute, most easily obtained from the Tide Tables). These data, segregated into ebb and flood directions, were then analyzed for a regression relation of the form

Speed (knots) =  $B_0 + B_1 \times \Delta H/\Delta t$  (feet/minute). The standard errors from the relation (table 3) were calculated as estimates of random variability with tidal variability being obtainable from the variation bars of figure 3. mean value of speed was obtained by evaluating the regression equation for  $\Delta H/\Delta t = 6.86 \times 10^{-3}$  ft/min, the mean value for Sewell's Point. Finally, these data are increased by a factor of 1.53, to correct a systematic bias in C&GS data reported in Fang (1979) and converted to cm/sec for consistency with the Elizabeth River estimates. The mean current estimates for use in the sediment plume model are obtained by dividing by  $\pi/2$ to produce mean values throughout the tidal phase (ebb or flood). The mean values of peak speeds during a tidal cycle are shown in table 3 as "corrected mean" and "corrected standard error", with the estimates for mean value for use in the sediment plume model listed as "Tidal Phase Mean".

Table 3. Results of Current Calculations in Newport News Ship Channel

Station	Phase	B <sub>o</sub> (knots)	$\frac{B_1}{\begin{cases} \frac{\text{knots}}{\text{ft/min x 10}^{-3}} \end{cases}}$	Standard Error (knots)	Corrected Mean (cm/sec)	Corrected Standard Error (cm/sec)	Tidal Phase Mean (cm/sec)
2	Flood	.01	.10	.14	55	11	35
2	Ebb	.09	.10	.15	61	12	39
3	Flood	.09	.11	.17	66	13	42
3	Ebb	32	.15	.13	56	10	36

In interpretation, the values from station 3 are probably more representative of the dredged channel than those from station 2. The former have their directions oriented parallel to the dredged channel while the latter are oriented in the direction of the natural entrance channel to Hampton Roads, 45° from the dredged channel.

## Rocklanding Shoal Channel

The third and final channel in the area under consideration, Rocklanding Shoal Channel, has the shape of a dog-leg on a chart. The channel is about 6 nautical miles long, with the dog-leg section comprising the southern 25% of the length. Passing the oyster grounds of Burwell Bay, it is maintained at a depth of 21½ feet below mean low water. Rocklanding Shoal Channel shares the tidal flow of the

James with another natural channel in Burwell Bay having a controlling depth of 11 feet. According to Nichols (1972), more tidal flow passes through Rocklanding Shoal Channel during flood tide than during ebb, classifying it as a flood channel. Along its length, Rocklanding Shoal Channel passes by numerous indentations and side channels with nearly the project depth, in contrast to the other channels described in this study, which are well defined cuts through shallow reaches.

In estimating the currents in Rocklanding Shoals
Channel, current meter data obtained during Operation James
River (Shidler and MacIntyre, 1967) are used. Current stations
with measurements obtained each half hour for a period of more
than three days were obtained at three locations within the
channel during this study. The locations are near the northern
and southern ends of the primary section and in the center of
the dog-leg. Currents at the two stations in the main part of
the channel were measured with a Roberts Radio current meter,
with a Hydro Products meter used for surface currents. At the
dog-leg station, a current pole was used for surface currents,
and a Pritchard drag was used for subsurface currents with
direction being determined with the ship's compass or a handheld Weems magnetic compass.

The currents at these stations each have a distinct character, so it is likely that no single value of current can accurately describe the entire channel. Because the available stations span the length of the channel, it is plausible that they represent the extreme conditions and that appropriate values

for the intermediate points can be obtained through linear interpolation from the available data.

In the dog-leg section, the direction for ebb currents has a bimodal distribution, the bottom current frequently following the channel at 90°T and the upper currents following the trend of the river at 130°T, but the pattern of occurrence is not regular. At the southern end of the major leg, the record shows ebb currents slightly dominating over flood currents. Perhaps more important, the flood currents have little relation to the corresponding  $\Delta H/\Delta t$ 's, the correlation coefficient being only .16 with 8 samples. In contrast, ebb currents, after deletion of a weather-associated outlier, have a correlation of .70 with  $\Delta H/\Delta t$ . It may be that the division of flood currents between Rocklanding Shoals Channel and the alternate channel through Burwell Bay is highly variable and responsive to other factors, such as transverse wind stress. From the available data, the ebb currents in the southern part of the channel tend, with marginal significance, to predominate over flood currents. At the northern end of the channel, the opposite condition is found with flood currents predominating over ebb currents substantially. Both flood and ebb currents are correlated (at the 90% significance level) with  $\Delta H/\Delta t$ 's at the northern end. Thus, the northern end of Rocklanding Shoals Channel is a definite flood channel, and the southern end is a slight ebb channel. This is consistent with the data of Nichols (1972) who characterized the channel as a whole as

a flood channel from data taken slightly north of its center.

The change in measured predominance may be due to the sharp

bend which must be taken by entering water to pass by the

current stations at both ends of the channel on the appropriate

tidal phase.

For five of the six possibilities, estimates can be made for the average current speeds to be found in the channel for average  $\Delta H/\Delta t$ . These are shown in table 4. For the sixth case, the estimate is simply of the available observations, with the standard deviation of the observations reported instead of the standard error of the regression. These values are shown in parentheses to emphasize the difference in derivation between them and the rest of the values.

In general, an increase in current speeds is found in the bottom of the dredged channels as one progresses up the James River within the study area. This increase is partly due to a decrease in cross-section area progressing upstream along with a smaller decrease in tidal flux. This interpretation is a contrast to that of Nichols (1972), who indicates that bottom currents at Rocklanding Shoal are substantially smaller than those near Newport News. The difference may be related to the difference between field data used in the present estimate and hydraulic model data used in the estimate of Nichols (1972).

Table 4

Results of Current Calculations in Rocklanding Shoals Channel

Station	Phase	B <sub>O</sub> (cm/sec)	B <sub>1</sub> cm/sec ft/minx10 <sup>-3</sup>	Standard Error (cm/sec)	Mean (cm/sec)	Tidal Phase Mean (cm/sec)	Corrected Tidal Phase Mean (cm/sec)
Dog-Leg	Flood	-44.76	16.10	12	66	42	42
Dog-Leg	Ebb	-90.18	23.75	8	72	46	46
South End	Flood	-	-	(10)	(33)	(21)	(32)
South End	Ebb	-35.28	11.72	8	45	29	44
North End	Flood	23.09	4.55	7	54	35	53
North End	Ebb	-16.20	8.14	5	40	25	39

## REFERENCES

- Boon, J.D. III and K. P. Kiley, 1978, Harmonic analysis and tidal prediction by the method of least squares, Special Report No. 186 in Applied Marine Science and Ocean Engineering, Virginia Institute of Marine Science, 49 pp.
- Cerco, Carl F. and A. Y. Kuo, unpublished ms, Real-time water quality model of the Elizabeth River system, Special Report No. 215 in Applied Marine Science and Ocean Engineering, Virginia Institute of Marine Science.
- Cronin, William B., 1971, Volumetric, areal, and tidal statistics of the Chesapeake Bay estuary and its tributaries, Special Report 20, ref. 71-2, Chesapeake Bay Institute, The Johns Hopkins University, 135 pp.
- DeRycke, R. J., 1969, Current meter logs from USC & GSS Ferrel (ASV-92).
- Fang, C. S., B. J. Neilson, A. Y. Kuo, R. J. Byrne and C. S. Welch, 1972, Physical and geological studies of the proposed bridge-tunnel crossing of Hampton Roads near Craney Island, Special Report in Applied Marine Science and Ocean Engineering No. 24, Virginia Institute of Marine Science, 42 pp.
- Fang, C. S., project engineer, 1975, A surface circulation study in middle Elizabeth River, A Report to NUS Corporation, 66 pp. + appendices.
- Fang, C. S., principal investigator, 1979, James River hydraulic model study with respect to the proposed third bridge-tunnel causeway in Hampton Roads, Special Report in Applied Marine Science and Ocean Engineering No. 212, 159 pp.
- Haight, J. J., et al., 1930, Tides and Currents in Chesapeake Bay and Tributaries, USCGS Special Publication 162, 143 pp.
- Jacobson, J. P. and C. S. Fang, 1977, Flood wave-tide interaction on the James River during the Agnes Flood, pp. 104-117

  in The Effects of Tropical Storm Agnes on the Chesapeake
  Bay Estuarine System, Chesapeake Research Consortium
  Publication No. 54.

- Kiley, Kevin P., 1980, The Relationship between wind and current in the York River estuary, Virginia, April 1973, unpublished M.S. Thesis, School of Marine Science, College of William and Mary, 195 pp.
- Lewis, J. K., 1975, The analysis of short term tidal data, M.S. thesis presented to the School of Marine Science, College of William and Mary, 104 pp.
  - Munday, John C., Jr., C. S. Welch and H. H. Gordon, Estuarine circulation from dye-buoy photogrammetry, 1980, pp. 417-428 in Proceedings of the Ports' 80 Specialty Conference, Norfolk, Virginia, American Society of Civil Engineers.
  - Munday, J. C., H. H. Gordon and C. J. Alston, 1980, Elizabeth River Circulation Atlas, in A study of dredging effects in Hampton Roads, Virginia, VIMS, 1981.
  - Neilson, Bruce J., 1975, A water quality study of the Elizabeth River: The effects of the Army Base and Lambert Point STP effluents, Special Report No. 75 in Applied Marine Science and Ocean Engineering, Virginia Institute of Marine Science, 133 pp.
  - Neilson, B. J. and M. Boule, 1975, An analysis of currents and circulation in Hampton Roads, Virginia, Vol. 2 in Studies for a Proposed Nansemond River Sewage Treatment Plant, A Report to McGaughy, Marshall & McMillan-Hazen and Sawyer: A Joint Venture, 98 pp.
  - Nichols, M. M., 1972, Sediments of the James River Estuary, Virginia, The Geological Society of America Memoir/33, 43 pp.
  - Pritchard, D. W. and W. V. Burt, 1951, An inexpensive and rapid technique for obtaining current profiles in estuarine waters, Chesapeake Bay Institute, The Johns Hopkins University, 13 pp.
  - Ruzecki, E. P. and R. Ayres, 1974, Suspended sediments near Pier 12, Norfolk Navy Base on 26 June and 15 September, 1973, Data Report No. 11, Virginia Institute of Marine Science, 9 pp + appendix.
  - Ruzecki, E. P. and T. Markle, 1974, A report on the prototype data collected in the James River, Special Report Number 84 in Applied Marine Science and Ocean Engineering, Virginia Institute of Marine Science, 34 pp.
  - Shidler, J. K. and W. G. MacIntyre, 1967, Hydrographic data collection for "Operation James River-1964", Data Report No. 5, Virginia Institute of Marine Science, 455 pp.

Welch, Christopher S. and Bruce J. Neilson, 1976, "Fine scale circulation near 'Foxtrot' in Hampton Roads, Virginia. An addendum to "Oceanographic, Water Quality & Modeling Studies for the Outfall from a Proposed Nansemond Waste Water Treatment Plant, A Report to McGaughy, Marshall & MacMillan-Hazen & Sawyer: A Joint Venture, Virginia Institute of Marine Science, Gloucester Point, Va., 23062, 36 pp.

Appendix 1. Times and Speeds of Maximum Currents in Newport News Channel During U.S. Coast and Geodetic Survey Observations, 1969.

Time Meridian: 75°W Observer: R. J. DeRycke Station: 2 Latitude: 36°57'28"N Longitude: 76°21'22"W

USC&GSS Ferrel (ASV-92)

Depth: 40'

1/13/69	Date	Time (EST)	Speed (Kt)	Direction ( <sup>O</sup> T Towards)	ΔΗ/ΔΤ (x10 <sup>-3</sup> ft/min)
1/21/69     2315     +0.6     220     +7.9       1/22/69     0425     -1.0     035     -7.5       1/22/69     1125     +0.8     220     +7.7       1/22/69     1635     -1.1     035     -7.4       1/22/69     2335     +0.6     215     +7.1       1/23/69     0520     -0.8     050     -6.7       1/23/69     1145     +0.9     215     +6.4       1/23/69     1720     -0.9     040     -6.5	1/13/69 1/14/69 1/14/69 1/14/69 1/15/69 1/15/69 1/15/69 1/15/69 1/16/69 1/16/69 1/16/69 1/16/69 1/17/69 1/17/69 1/18/69 1/18/69 1/18/69 1/18/69 1/18/69 1/19/69 1/19/69 1/20/69 1/20/69 1/20/69 1/20/69 1/21/69 1/21/69	(EST)  2055 0300 1000 1550 2245 0355 1015 1705 2220 0505 1215 1820 2355 0635 1255 1845 0055 0740 1335 1935 0155 0815 1425 2055 0235 0930 1535 2210 0330 1055	(Kt) -0.4 +0.7 -0.8 +0.6 -0.7 +0.8 -0.9 +0.6 -0.7 +0.8 -0.9 +0.7 -0.9 +0.7 -0.7 +1.1 -0.5 -0.8 +1.0 -1.0 +1.0 -1.0 +1.0	OT Towards)  055 205 035 215 025 220 030 220 050 220 030 235 040 210 030 225 050 220 040 230 050 220 050 240 045 240 0550 210 035 215	(x10 <sup>-3</sup> ft/min) -5.7 +7.0 -6.7 +5.5 -6.0 +9.2 -7.4 +5.9 -6.7 +8.3 -8.2 +7.1 -7.8 +9.1 -8.9 +7.6 -7.9 +9.4 -9.1 +8.1 -8.5 +9.6 -9.3 +8.2 -8.5 +9.6 -9.3 +8.2 -8.5 +9.8
- 177/1764 11070 - 41 11	1/21/69 1/22/69 1/22/69 1/22/69 1/23/69 1/23/69	2315 0425 1125 1635 2335 0520 1145	+0.6 -1.0 +0.8 -1.1 +0.6 -0.8 +0.9	220 035 220 035 215 050 215	+7.9 -7.5 +7.7 -7.4 +7.1 -6.7 +6.4

Appendix 1 (Cont'd) Station 2

Date	Time (EST)	Speed (Kt)	Direction ( <sup>O</sup> T Towards)	$\Delta H/\Delta T$ (x10-3ft/min)
1/24/69 1/24/69 1/24/69 1/25/69 1/25/69 1/25/69 1/25/69 1/26/69 1/26/69 1/26/69 1/27/69 1/27/69 1/27/69 1/27/69 1/27/69 1/27/69 1/28/69 1/28/69 1/28/69 1/28/69 1/29/69 1/29/69 1/29/69 1/29/69 1/30/69	0605 1240 1855 0005 0655 1305 1920 0110 1000 1435 2115 0205 0745 1455 2130 0250 1055 1655 2155 0455 1150 1645 2225 0405	-0.9 +0.7 -0.8 +0.8 -0.6 +0.4 -0.7 +0.5 -0.3 +0.5 +0.5 -0.5 +0.5 -0.5 +0.6 +0.4 +0.6 -0.5 +0.6 +0.4 +0.6 +0.4 +0.6 +0.5 +0.5 +0.5 +0.5 +0.5 +0.5 +0.5 +0.5	040 230 045 240 060 260 050 245 040 210 060 250 045 220 040 220 030 <sup>1</sup> 255 060 245 060 215 035 230	-6.1 +5.4 -5.6 +6.2 -5.6 +4.5 -4.8 +5.6 -4.9 +4.0 -4.4 +5.2 -4.9 +4.0 -4.4 +5.2 -4.9 +4.0 -4.4 +5.2 -4.9 +4.0 -4.4 +5.2
End of	Data			

<sup>&</sup>lt;sup>1</sup>Speed from drogued buoy. Roberts Radio current meter readings are erratic and low.

Appendix 1 (Cont'd)

Station: 3 Latitude: 36<sup>o</sup>57.3'N Longitude: 76<sup>o</sup>22.9'W Time Meridian: 75°W

Observer: R. J. DeRycke

USC&GSS Ferrel (ASV-92) Depth: 40' Roberts Radio Current Meter

Date	Time (EST)	Speed (Kt)	Direction ( <sup>O</sup> T Towards)	$\Delta H/\Delta T$ (x10 <sup>-3</sup> ft/min)
1/14/69	1600	+0.7	250	+5.5
1/14/69	2130	-0.6	090	-6.0
1/15/69	0435	+1.1	260	+9.2
1/15/69	1115	-0.7	070	-7.4
1/15/69	1705	+0.8	110 <sup>2</sup>	+5.9
1/15/69	2245	-0.8	295 <sup>2</sup>	-6.7
1/16/69	0520	+1.1	120 <sup>2</sup>	+8.3
1/16/69	1245	-1.0	80	-8.2
1/16/69	1745	+0.9	260	+7.1
1/17/69	0010	-0.9	075	<b>-7.</b> 8
1/17/69	0615	+1.4	275	+9.1
1/17/69	1305	-1.1	080	-8.9
1/17/69	1845	+0.9	260	+7.6
1/18/69	0045	-0.9	080	-7.9
1/18/69	0710	+1.3	270	+9.4
1/18/69	1400	-1.0	075	-9.1
1/18/69	1925	+0.9	270	+8.1
1/19/69	0140	-0.9	075	-8.5
1/19/69	0750	+1.2	265	+9.6
1/19/69	1415	-1.0	080	-9.3
1/19/69	2030	+0.7	270	+8.2
1/20/69	0255	-0.7	080	-8.5
1/20/69	0905	+1.1	270	+9.3
1/20/69	1600	-0.9	065	-9.0
1/20/69	2130	+1.0	295	+8.2
1/21/69	0330	-1.0	065	-8.2
1/21/69	1020	+0.9	290	+8.5
1/21/69	1645	-0.9	80	-8.2
1/21/69	2240	+0.7	270	+7.9
1/22/69	0410	-0.8	070	<b>-7.</b> 5
1/22/69	1045	+0.9	280	+7.7
1/22/69	1620	-0.9	080	<b>-7.4</b>
1/22/69	2315	+0.7	275	+7.1
1/23/69	0540	-0.8	070	-6.7
1/23/69	1100	+0.8	270 1	+6.4
1/23/69	1710	-0.9	070	-6.5

<sup>&</sup>lt;sup>2</sup>Readings are in wrong direction-suspect instrument malfunction.

<sup>&</sup>lt;sup>1</sup>Raw data indication switches from 040 to 270 with little change in speed. Instrument malfunction is plausible.

Appendix 1 (Cont'd) Station 3

Date	Time (EST)	Speed (Kt)	Direction ( <sup>O</sup> T Towards)	$\Delta H/\Delta T$ (x10-3ft/min)
1/23/69 1/24/69 1/24/69 1/25/69 1/25/69 1/25/69 1/25/69 1/26/69	2400 0615 1230 1740 0035 0605 1255 1935 0110 0815	+0.7 -0.8 +0.9 -0.7 +1.0 -0.6 +0.6 -0.4 +0.8 -0.2	060 065 240 270 <sup>2</sup> 270 270 <sup>2</sup> 085 <sup>2</sup> 080 080 <sup>2</sup> 085	+6.8 -6.1 +5.4 -5.6 +6.2 -5.6 +4.5 -4.8 +5.6 -4.9
1/26/69	1335	+0.2	085 <sup>2</sup>	+4.0
1/27/69 1/27/69 1/28/69 1/28/69 1/28/69 1/28/69 1/29/69 1/29/69 End of	1505 2130 0305 1020 1625 2200 0430 0935 Data	+0.5 -0.2 +0.9 -0.5 +0.5 -0.2 +1.0 -0.3	260 090 265 080 270 90 255	+4.0 -4.4 +5.2 -4.9 +4.0 -4.5 +5.8

 $<sup>^{2}\</sup>mbox{Readings}$  are in wrong direction-suspect instrument malfunction.

## ELIZABETH RIVER SURFACE CIRCULATION ATLAS

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### Description

The Elizabeth River Surface Circulation Atlas is a compendium of maps which detail the surface circulation throughout the main branch of the Elizabeth River, in the port of Hampton Roads, Virginia. Data for the Atlas maps were obtained directly from field experiments using Remote Sensing and dye-emitting low-windage surface drogues. The maps show surface Lagrangian trajectories under various combinations of wind and tide. The Atlas is not intended to duplicate NOAA tidal current tables, but rather to supplement the tables with empirical trajectory data at increased spatial resolution. Knowledge of surface currents under different tide and wind conditions enables a user to predict the movement of floating debris, such as oil spills, within the Elizabeth River Basin.

The Atlas is based on the fact that motion of surface water is a product of tidal flow and local winds, and is repeatable under similar conditions. The user obtains readily-available local wind and predicted tidal data, and finds within the Atlas the maps referring to the same conditions. With the trajectories on the maps, the user may move along a trajectory forward in time to find possible future positions, or backward to identify possible earlier positions.

The Atlas was designed to be used by planners and managers charged with decision-making and regulation in the Hampton Roads port region. Within this region, the Elizabeth River Basin was chosen for development of a circulation atlas, because of the Basin's large volume of ship traffic, industrial and waste treatment plants, oil and coal handling facilities, and military and civilian port activities. Immediate applications include: prediction of oil slick movement, to permit containment of a spill before serious environmental damage occurs; 'hindsight' prediction, to identify a possible source for a spill; and sewage and industrial outfall siting, with consideration for all the various wind and tide combinations.

The Atlas is arranged in leaves to allow future revisions in response to specific user needs. Future generations of the Atlas will include data from new field studies, filling in data gaps in the Condition Matrix.

One possible modification would be the addition of a grid coordinate system superimposed on the Atlas maps for orientation. As the data base becomes more complete, circulation information could be referenced to individual grid squares for tide and wind combinations, extending the usefulness of the Atlas to all locations in the Basin. A second possibility is to include circulation anomalies such as foam lines and convergence zones on the maps. These, of course, significantly modify the surface circulation by trapping and concentrating surface material under certain tidal phases. A third possibility is the addition of maps showing subsurface trajectories. Such data can be obtained using Remote Sensing techniques developed by Munday, Welch, and Gordon (1980, Ports 80 Conference, ASCE, p. 417-428).

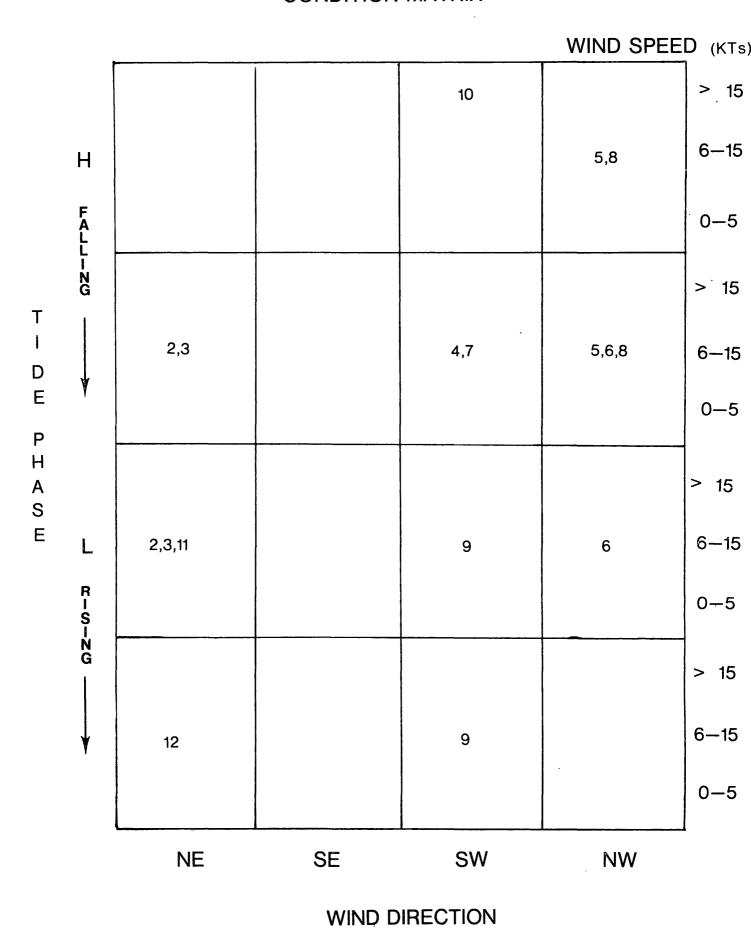
Revisions will be contingent upon user experience with the Atlas and upon future needs. Due to the flexibility of the Atlas design, accommodations to user needs could be undertaken with a minimum of expense, effort, and time. New current data can be obtained and incorporated easily because the Atlas is prepared using semi-automated photogrammetric and computer plotting techniques.

### Instructions

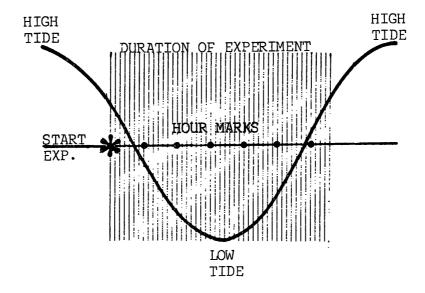
The surface circulation maps are keyed to wind data from the National Weather Service Office at Norfolk Regional Airport, and to NOAA Tide Tables for predicted high and low water at Sewells Point (Hampton Roads). The following steps are taken to locate the proper map:

- 1. Using the NOAA Tide Tables, find the times of predicted low and high tide at Sewells Point (Hampton Roads) which bracket the time of interest,
- 2. Call the National Weather Service Office in Norfolk (853-0553) and request the current and previous (2 to 3 hours) wind velocities,
- 3. Using the Condition Matrix, locate one of the sixteen bins appropriate for the tide phase and wind direction from Steps 1 and 2. Within the bin locate the wind speed rectangle corresponding to the actual speed from Step 2, and
- 4. The number(s) indicate the map number(s) which contain the specific circulation data of interest.

On each map are surface drogue positions plotted every 15 minutes, with the initial release position depicted by a \* symbol. On the lower right corner is a tide curve (high tide above the horizontal line, low tide below) showing the span of the experiment within a tide cycle. Dots along the horizontal line indicate hours after drogue release. Wind speed and direction are illustrated on each map with an arrow referenced to the north arrow (0 to 5 knots, short arrow; 6 to 15 knots, medium arrow; greater than 15 knots, wind arrow same length as north arrow).



### Sample Tide Curve



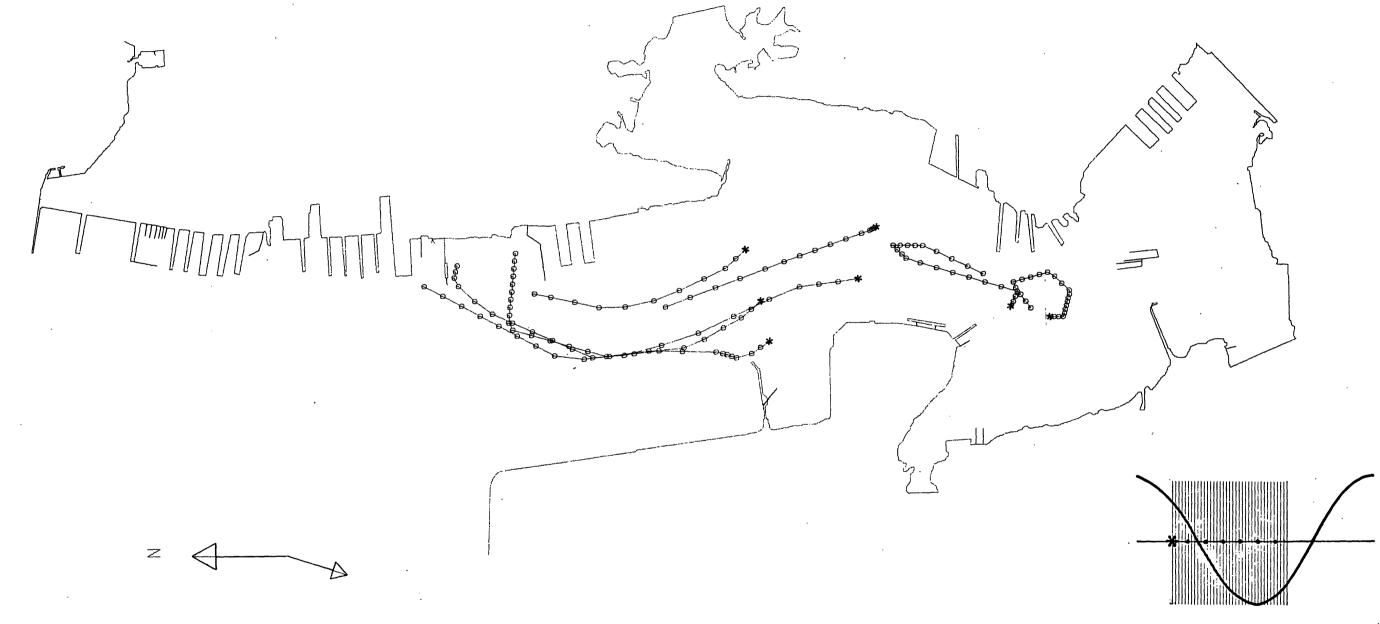
## Example (Hypothetical)

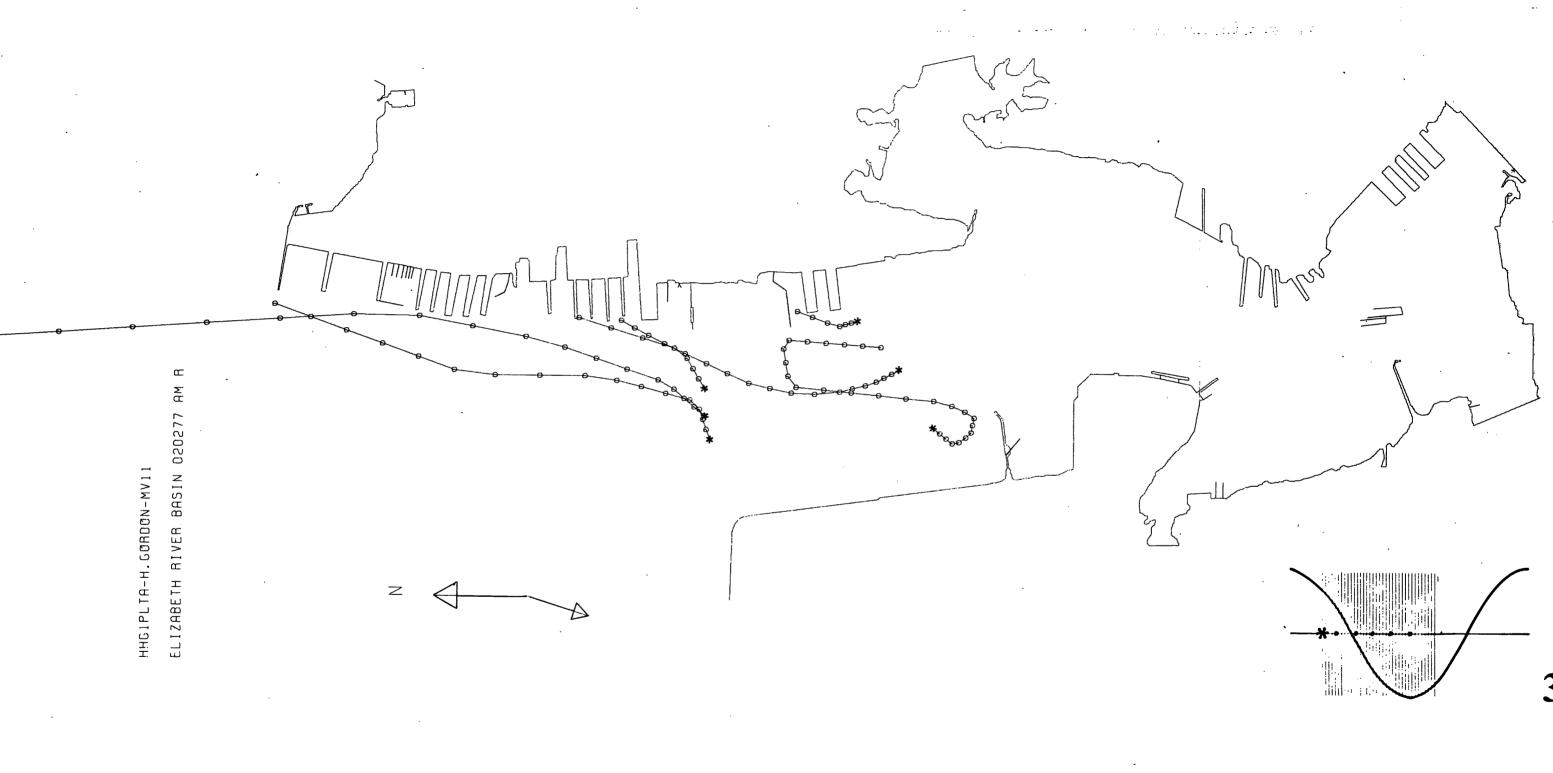
Suppose one wishes to know surface circulation west of Tanner Point in the Elizabeth River at 1200 on a particular day. By consulting the NOAA Tide Tables, time of high tide is found to be 0930 and low tide 1500. A call to the Norfolk Weather Bureau shows winds to be 200° at 10 gusting to 15 knots. Checking the Condition Matrix for a tide phase between high (H) and low (L), wind direction SW, and speed 6 to 15 knots reveals maps number 4 and 7 are appropriate. A brief review of the wind and tide information on both maps tends to favor map 4 which begins earlier in the tide cycle and has winds nearer 200°. Drogue tracks show a well-defined ebb flow.

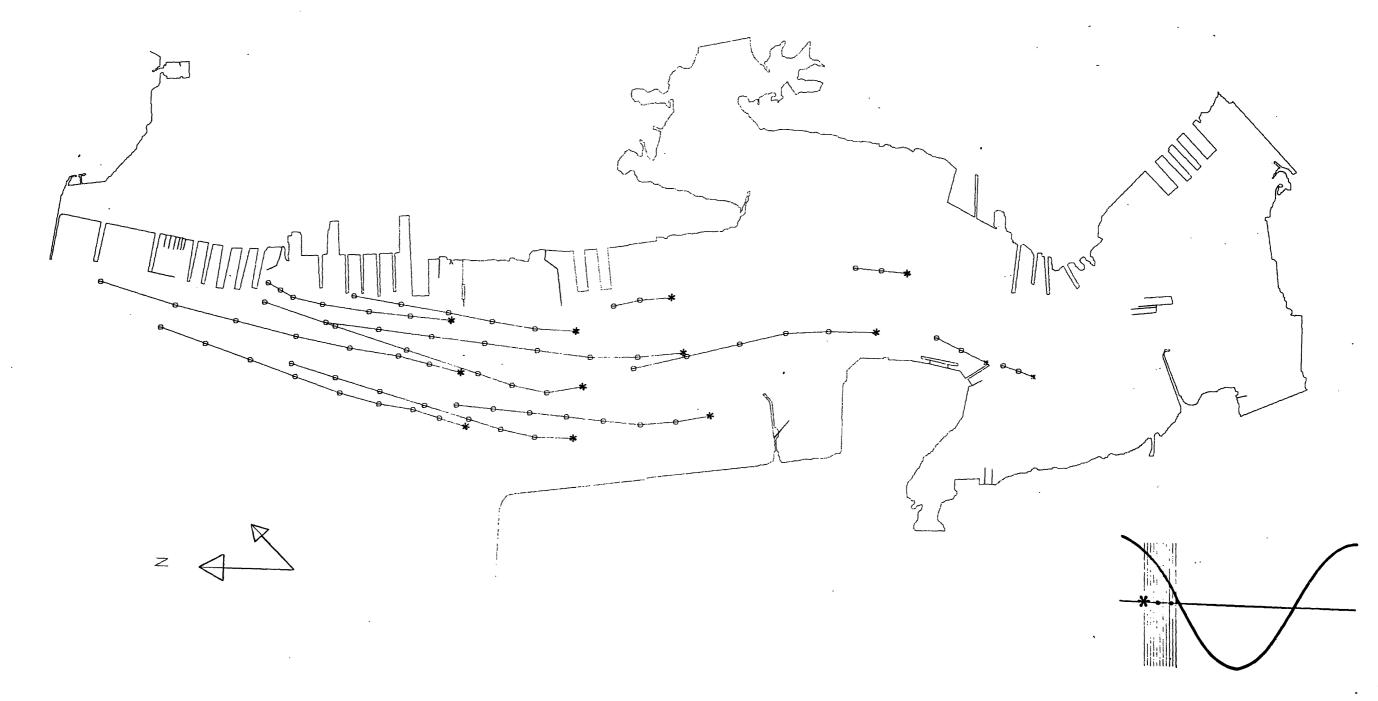
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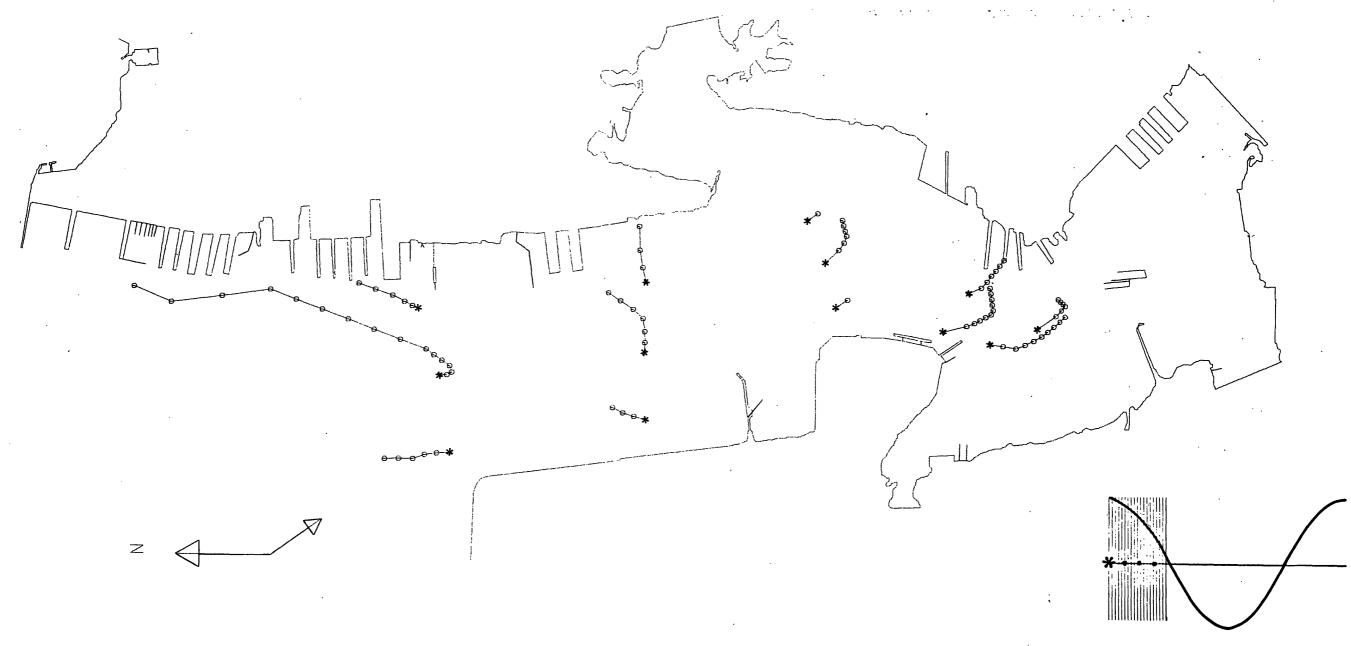
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ELIZABETH RIVER BASIN 42376



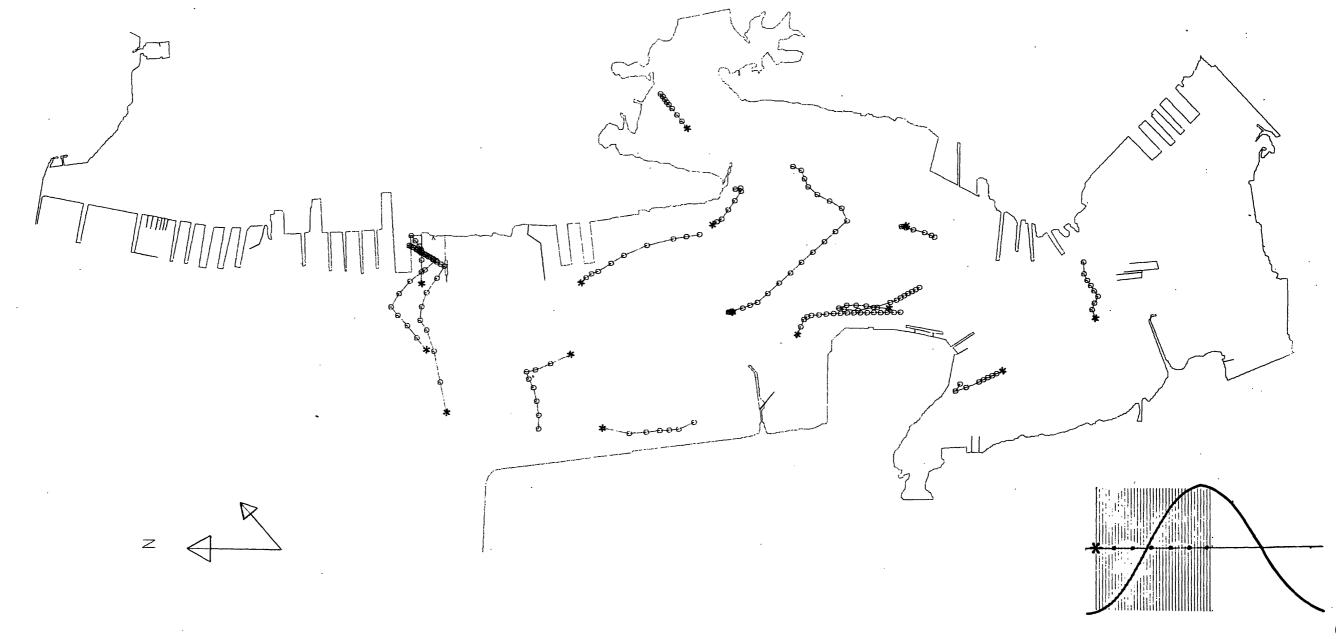


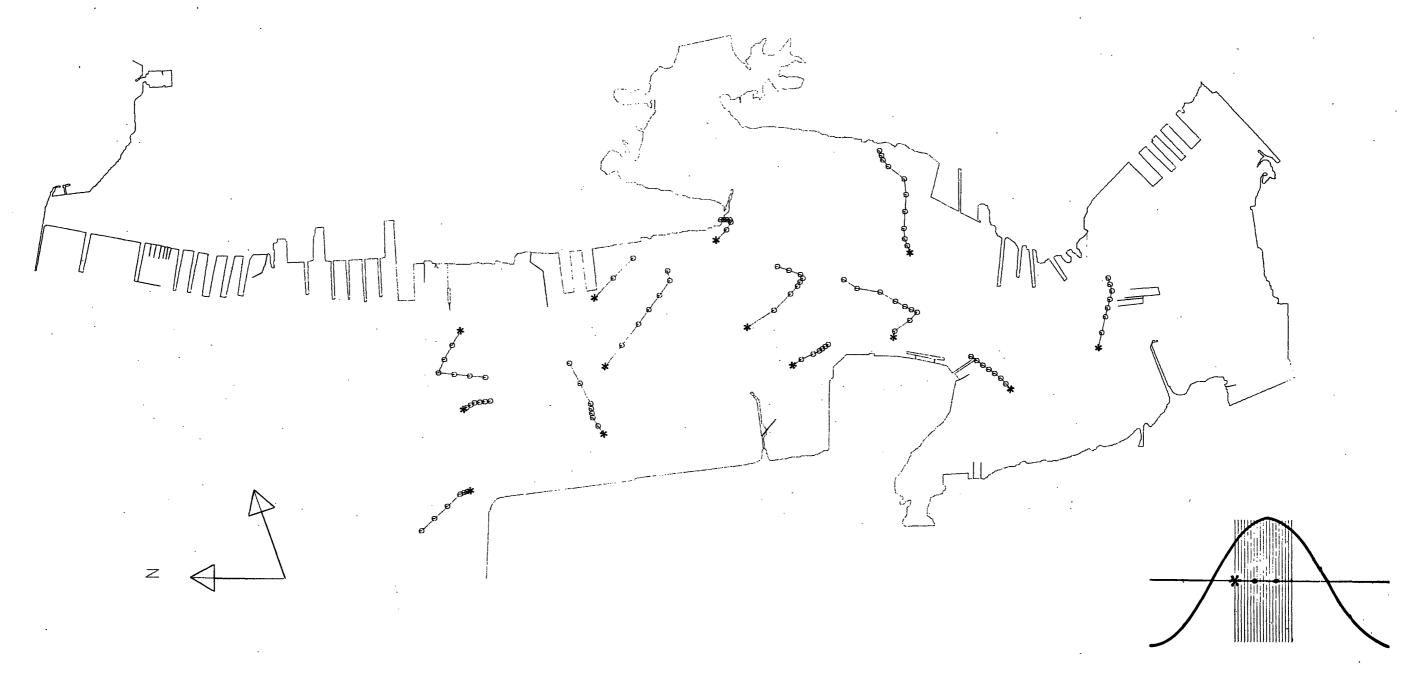


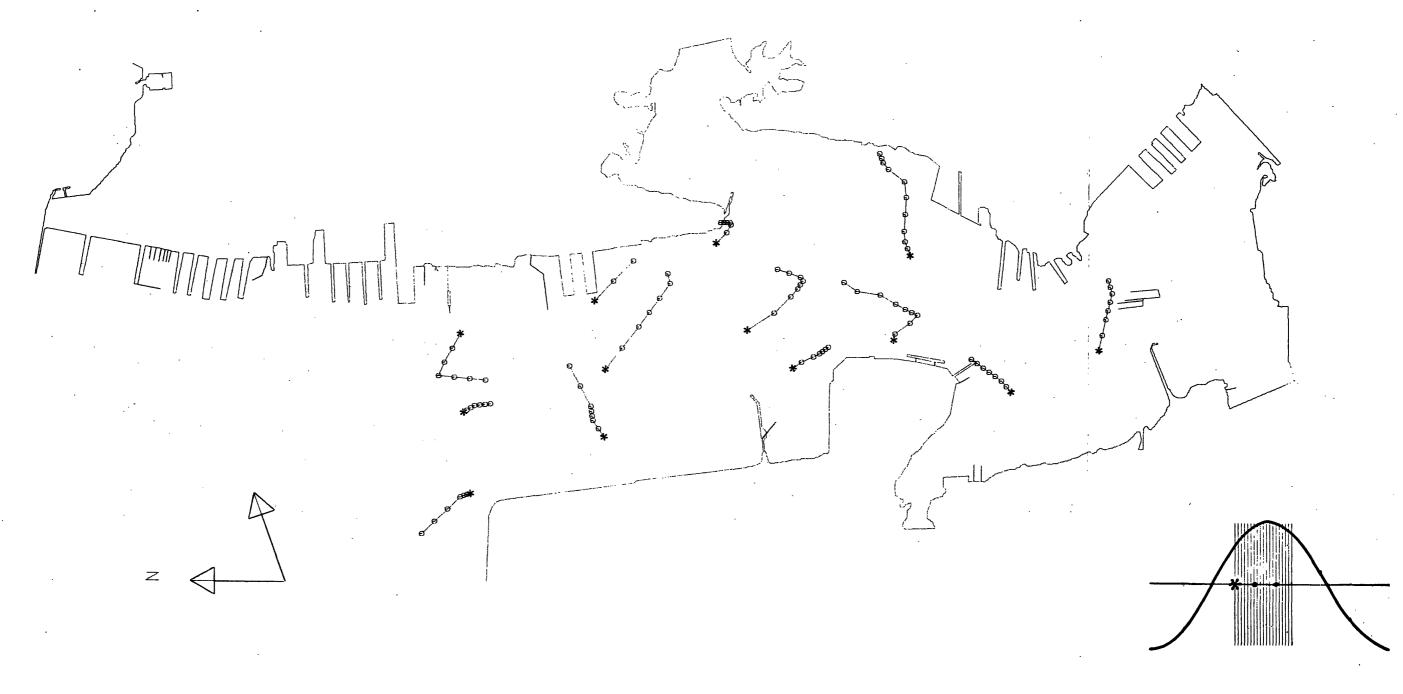


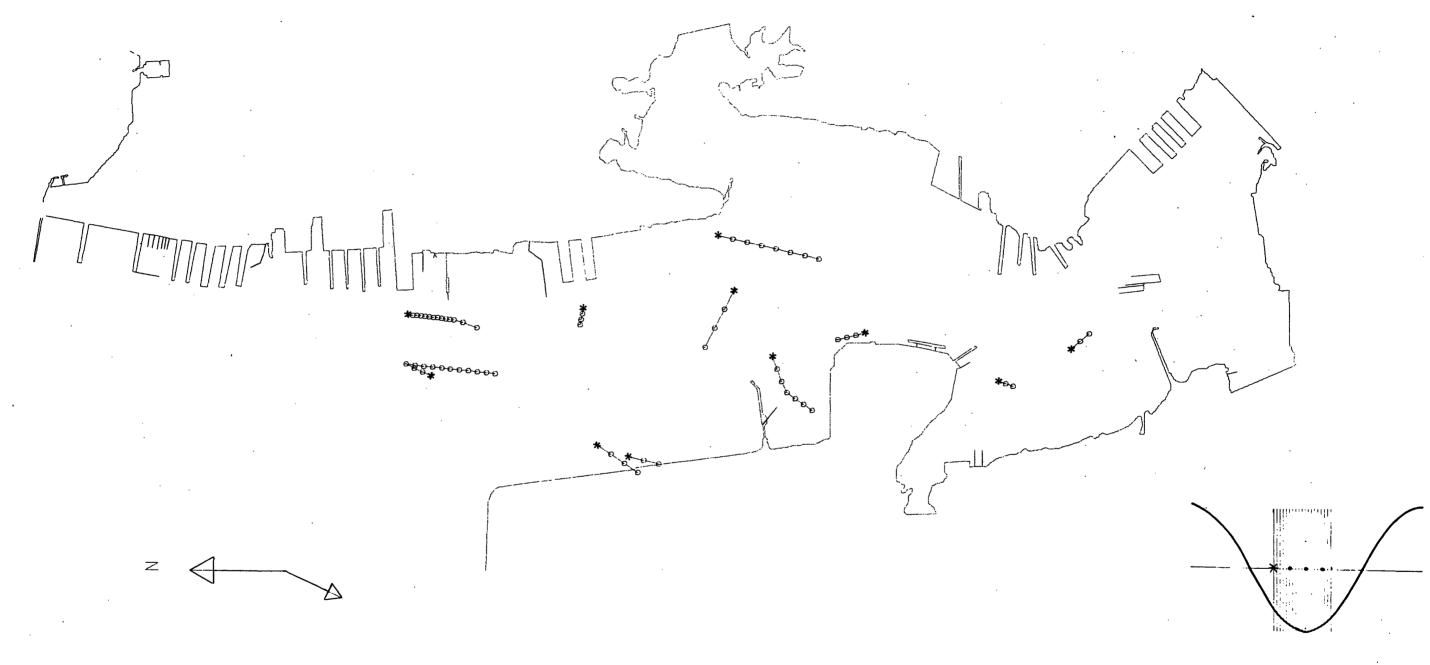
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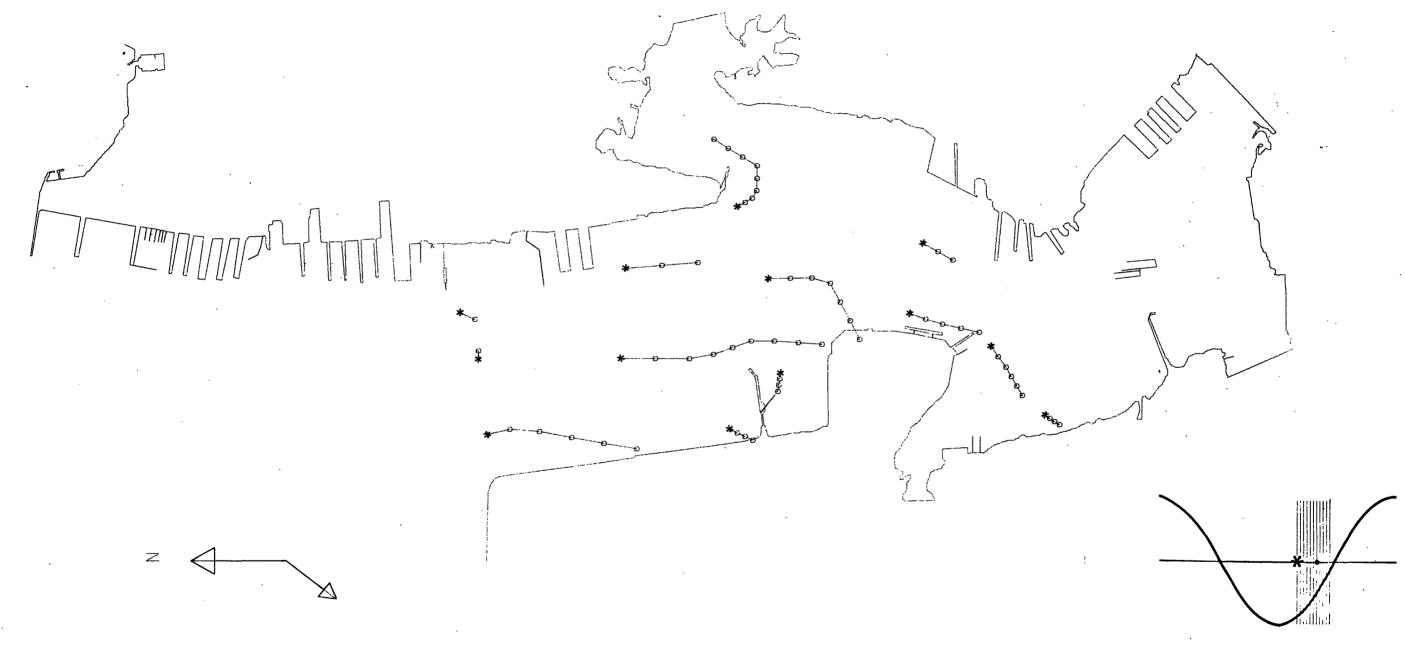
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DREDGING EFFECTS

# THE EFFECTS OF DREDGING IMPACTS ON WATER QUALITY AND ESTUARINE ORGANISMS: A LITERATURE REVIEW

bу

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### Introduction

The primary purpose of this section is to evaluate the effects of total suspended solids (TSS) levels normally generated by hydraulic cutterhead and clamshell dredges where a confined disposal site is utilized. The emphasis of this review will be on the sublethal and lethal effects of increased TSS concentrations on various estuarine organisms. There will also be a limited treatment of the effects of dredging on other water quality parameters whenever it is applicable to the types of dredging activities being considered in this report.

This report will be divided into two parts. The first will discuss the impacts of non-open water disposal hydraulic cutterhead and clamshell dredging on water quality. The second will present available data from the literature on the effects of TSS on specific estuarine organisms.

The literature on the effects of dredging, spoil disposal and suspended sediments on water quality and aquatic organisms has been very ably reviewed and summarized by a number of workers. For a more detailed analysis than is presented here Bouma (1976), Morton (1977), Stern and Stickle (1978), Allen and Hardy (1980), Saila (1980), and the Corps of Engineers Dredged Material Research Program Snythesis Report Series are suggested.

### Water Quality Aspects

The most obvious impact of dredging on water quality is the increase in suspended solids (turbidity) created by the disturbance of the bottom sediments. Despite the extensive research on dredging impacts very little has concentrated on the dredge cutterhead or clamshell as a source of suspended solids. Most of the information available deals with levels of TSS generated at the pipeline or barge disposal site where levels in grams to tens of grams per liter have been observed (Chesapeake Biological Laboratory, 1970 and May, 1973). Documentation of the levels of suspended solids created

by the dredge itself are very few. The San Francisco Bay Maintenance Dredging EIS, 1975 cited from Williamson and Nelson (1977) reported near field levels of TSS from removal operations of 43-70 mg/l for a pipeline, 12-282 mg/l for a clamshell and 74-871 mg/l for a hopper dredge. After reviewing the available literature Barnard (1978) made the following comments on the general ranges of suspended solids created by different types of dredges. Clamshell dredges usually produce a plume of suspended solids 300 m downstream on the surface and 500 m downstream near the bottom. They produce a maximum TSS concentration of approximately 500 mg/l while the average water column concentration will be about 100 mg/l. Cutterhead dredges normally produce a suspended solid plume near the bottom of a few 100's mg/l for a few hundred meters downcurrent. Hopper dredging without overflow will generate suspended solids in the range of a few grams/liter adjacent to the dragheads.

Wakeman et al (no date) cited from San Francisco COE (1975) reported a reduction of light transmission of approximately 4% below background levels adjacent to a cutterhead dredge. They also reported highly variable turbidity values for a clamshell dredge. These values ranged up to 26% reduction in light transmission below background levels.

Boon and Byrne (1975) in a monitoring report on a dredging operation on Hampton Bar reported typical surface plume TSS concentration of 20-40 mg/l during maximum current conditions. Concentrations within 400 yds of the hydraulic dredge were 50 mg/l and higher. A visible plume approximately  $400 \times 4000$  yds was produced during flood tide. Background TSS levels were 5-15 mg/l.

Boon and Thomas (1975) in a report on dredging operations associated with the construction of the second Hampton Roads Bridge Tunnel reported TSS concentrations of 15-30 mg/l in the surface plume of a hydraulic dredge

at distances of less than 1000 ft. Background levels were 3-9 mg/l. They also recorded natural bottom TSS levels of 120 mg/l over the existing tunnel during maximum tidal current velocity.

The issue of dissolved oxygen (D.O.) reduction as a result of dredging is also clouded by the fact that most reports refer to D.O. reductions during the open-water disposal of the dredged material. Even in this instance the reduction of D.O. has generally been relatively small except in bottom water density flows and of a relatively short duration (CBL, 1970; COE, 1976; Barnard, 1978). Near-bottom D.O. levels may be less than 2 mg/l near the discharge pipe during open-water disposal (Barnard, 1978).

However, Brown and Clark (1968) did report D.O. reductions from 16% to 83% below the expected minimum in the Arthur Kill between Staten Island, N.Y. and New Jersey during dredging operations. The usual method of dredging was clamshell and hopper barge which was dumped at sea. They described the bottom sediments as containing "accumulations of waste discharges that are deposited continuously. The bottom, which is characterized by a black, soft, oily silt, emanates odors of chemicals, oils, and hydrogen sulfide."

May (1973) reported substantial D.O. reduction at the discharge pipe and in bottom density flows out to 1200 feet from the discharge during open water disposal.

Wakeman et al (no date) cited from San Francisco COE (1975) reported a D.O. reduction of less than 1 ppm, uniform with depth, adjacent to a cutterhead dredge. The reductions around the clamshell dredging were again variable with average reduction being approximately 2 ppm. Some increases in D.O. were also noted, probably caused by the agitation of the water column by the bucket. The background surface D.O. was 8-9 mg/1.

Observations by the JBF Scientific Corp. in San Francisco COE (1975) showed an aeration of surface waters by a clamshell dredge and a D.O.

increase in bottom waters of approximately 3 ppm. They postulated that an upwelling was created by using the 18 cu. yd. bucket, drawing highly oxygenated water into the plume.

The literature reviewed for this report did not contain any information on the release of nutrients, heavy metals and pesticides by dredging per se.

All mention of this effect was either associated with open-water disposal or the information related to both dredging and disposal operations with no distinction in the data being made.

The material reviewed on open-water disposal operations did report that releases over background of manganese, ammonium nitrogen, orthophosphate; and reactive silica can occur for short periods of time (Barnard, 1978). Burks and Engler (1978) reported that releases of short duration of chlorinated pesticides, PCB's and ammonia can occur when their levels in the sediment are elevated. They also reported that heavy metals can be released under very specific conditions of pH and oxidation-reduction potential. These conditions are usually not found during typical open-water disposal operations, however.

The nature and extent of any nutrient and/or pollutant release and its resultant impact is dependent upon a number of site specific characteristics including: concentration in the sediment, amount of organic and fine grained material in the sediment, pH, oxidation-reduction potential and duration of release.

Kaplan et al (1974) reported significant increases in particulate phosphates, silicates and chlorophyll a immediately after a hydraulic dredging operation in a small enclosed coastal embayment which also received the effluent from the disposal area. There was no appreciable difference in levels of nitrates, nitrites and dissolved organic and inorganic phosphates before and after dredging.

Although not strictly an impact on water quality the increased rate of sedimentation in the vicinity of dredging operations can have an adverse effect on the area. Here again most of the impacts described in the literature refer to open-water disposal operations.

Wilson (1950) cited from Bouma (1976) studied the effects of shell dredging along the Texas Gulf coast. He reported that "suspended silt and resulting sedimentation extended in significant concentrations approximately 300 yards from the dredge" and that oysters placed in baskets were covered with silt within 300 yards of the dredge if they were at the same depth as the adjacent bottom but were not covered if they were placed higher than the surrounding bottom.

Mackin (1961) made several theoretical observations on the sedimentation possible from cutterhead and clamshell dredged utilizing open-water disposal on adjacent oyster leases. These hypotheses were based on average turbidities in ppm (not mg/1 TSS) in the sediment plume, current velocities, open-water disposal immediately adjacent to the dredge and the distance to nearby oyster leases. The amounts of sedimentation theoretically expected ranged from 0.2 inches on a seven acre lease 1500 feet from a cutterhead dredge with average plume values of 500 ppm turbidity to 0.5 inches on a 1000 foot long area immediately adjacent to the disposal area with an average plume value of 200 ppm turbidity from a clamshell dredge. He stated that the maximum distance the spoil was transported from the discharge pipe of a hydraulic dredge was 1300 feet.

Ingle (1952) in a study of the effects of dredging on fish and shell-fish reported that it appeared that all potentially deleterious particles had settled to the bottom within 300-400 yards of an active dredge with overboard disposal. Average sedimentation rates at 75 yards from a dredge were .228 inches/hr. just off the bottom and .108 inches/hr at mid-depth.

Hellier and Kornicker (1962) measured the sedimentation rate around an open-water spoil disposal site in Aransas Pass, Texas. Stations were established at 0.03, 0.5, 1.0, 1.5 and 2.0 miles in a line perpendicular from the channel. The spoil was deposited between the first two stations. Background sedimentation rates were 2-3 mm for a nine month period. One week after dredging there were seven cm of sediment on the 0.03 mi. station and 22 cm on the 0.5 mi. station. Sedimentation at the 1.0, 1.5, and 2.0 mi. stations was negligible.

Boone and Byrne (1975) in a study of a dredging project on Hampton Bar, Va. reported bottom deposition resulting from the dredging activity was primarily restricted to an area within a 200 yard radius of the dredge.

### Impacts on Estuarine Organisms

Phytoplankton. The reported effects of dredging and dredge spoil disposal on phytoplankton and primary production are many and varied depending upon the situation at each site. These range from a significant reduction in carbon uptake by phytoplankters (Sherk et al, 1976) to a substantial increase in primary production (Subba Rao, 1973) to no observable effect (Flemer, 1970) to a combined effect of reduced photosynthesis by increased light attenuation and the stimulation of photosynthesis by the introduction of nutrients (Odum and Wilson, 1962). For specific levels of impact please refer to Table 1.

Crustaceans. The possible impacts of dredging on this group of organisms include interference with feeding, clogging of gills and heavy metals and pesticide uptake. The levels of TSS normally encountered in upland disposal type dredging operations, a few hundred mg/l maximum, will probably cause some reduction in feeding efficiency and probably some interference with respiration of selected copepods (See Table 2). However,

the areal extent of the highest levels of TSS is very small, a radius of a few hundred meters maximum around the dredge. The impact, in all but the smallest of water bodies, should be minimal.

Peddicord and McFarland (1978) reported uptake by decapod crustaceans of heavy metals and polychlorinated hydrocarbons on a limited basis. These accumulations occurred after days of exposure to fluid mud concentrations (grams to tens of grams/liter) of highly contaminated sediments. Neither the TSS concentration levels nor the duration of exposure can be expected during dredging with upland disposal operations. Sullivan and Hancock (1977) reviewed the general impacts of dredging on zooplankton.

Mollusks. While the adults of this group of organisms are very susceptible to adverse impacts from dredging due to their sessile nature, it is also a group that has adapted to the most turbid portion of the water column. The pumping rate of adult bivalves can be adversely affected by levels of TSS generated by dredging, a few hundreds of mg/l (Table 3). However, they are also adapted to survive long periods with both valves closed or at reduced pumping rates to accommodate naturally occurring periods of adverse conditions.

The eggs of oysters are susceptible to a substantial reduction in their development at TSS concentrations of silt in the upper range of those expected from dredging operations (See Table 4). Oyster egg development is affected by lower concentrations of silt than are hard clam eggs. The larvae of oysters and clams, however, do not appear to be significantly affected until they are exposed to concentrations of silt in excess of normal dredging operation levels. Here again oyster larvae appear to be more susceptible than clam larvae (Table 4).

<u>Fishes</u>. With the exception of juvenile striped bass and silversides concentrations of TSS lethal to fishes are not even approached until they are exposed to levels one to two orders of magnitude above dredging levels for extended periods of time (Table 5).

The sublethal effects listed in Table 6 are also not experienced by fishes until levels of TSS above normal dredging operations are reached with exposure times that do not appear realistic for animals as motile as fish. The significance of the changes in blood chemistry listed is not completely understood but are symptomatic of an organism undergoing oxygen deprivation.

The effects of increases TSS concentrations on the eggs and larvae of fishes are listed in Table 7. The only effect on the eggs of four species by TSS levels at the extreme upper limit of those expected from a dredging operation was a one hour delay in hatching over controls. Lethal concentrations ( $LC_{50}$ ) of TSS on the fish larvae studies were far in excess of anticipated levels from dredging.

Several general observations are in order on the experiments done to ascertain the impact of suspended solids on aquatic organisms. Direct comparisons between the impacts natural sediments and those of processed materials, e.g. Kaolin, Fuller's earth, etc., cannot be routinely made because in some instances effects may have been observed at low levels with the processed materials but similar effects were not observed until much higher levels of natural sediments were reached and vice versa. The degree of contamination of natural sediments with heavy metals, hydrocarbons, pesticides and other pollutants can also play a significant role in the observed impacts on aquatic organisms.

Dissolved oxygen levels and temperature also affect the impacts of suspended solids. Organisms appeared to fair better at high dissolved

oxygen levels and low temperatures than they did at low dissolved oxygen levels and high temperatures (Peddicord et al, 1975).

The habitat in which the organisms are normally found also influences the level at which the organism is impacted by suspended solids. Those living in naturally highly turbid areas are usually better adapted than those preferring relatively clear water.

### Summary and Conclusions

In general, it may be concluded from the results of this review that the effects of dredging with confined upland spoil disposal are limited.

They include:

- a. Minor impacts on phytoplankton due to reduced light penetration which is often offset by increased nutrient availability.
- b. Limited interference with zooplankton feeding immediately adjacent to the dredge due to increased TSS.
- c. Reduction in development of oyster eggs due to increased TSS.
- d. Possible slight increase in sedimentation adjacent to the dredge which might affect adjacent shellfish beds.
- e. Based on the nutrient and pollutant release data from open-water disposal operations, very limited increases of manganese, iron, ammonium nitrogen, orthophosphate and reactive silica can be expected. Under very specific conditions the possibility also exists for the limited release of other heavy metals and pesticides during dredging operations.
- f. In some instances there is a reduction of D.O. of 1-2 mg/1 when dredging normal harbor sediments.

These impacts are primarily restricted to the immediate vicinity of the dredge, a radius of a few hundred meters. Tidal and wind generated currents will usually provide sufficient mixing and dilution to return the water to near background levels within this distance. Table 1. The effects of various suspended solids on phytoplankton

SPECIES	LIFE STAGES	CONC.	EXPOSURE	MATERIAL	EFFECT	SOURCE
Monochrysis lutheri	NA	2,250 mg/1	NS	SiO <sub>2</sub> median size = 17 µm	80% reduction in carbon uptake	Sherk, et al., 1976
M. <u>lutheri</u>	NA	250 mg/1	ns	SiO <sub>2</sub>	approx. 23% reduction in carbon uptake	11
Chlorella sp.	NA	1,000 mg/1	NS	SiO <sub>2</sub> median size = 6.2 µm	90% reduction in carbon uptake	11
Chlorella sp.	NA	250 mg/1	NS	SiO <sub>2</sub>	approx. 30% reduction in carbon uptake	"
Nannochloris sp.	NA	250 mg/1	NS	si0 <sub>2</sub>	approx. 28% reduction in carbon uptake	11
Nannochloris sp.	NA	1,000 mg/1	NS	SiO <sub>2</sub> particles <15 µm	90% reduction in carbon uptake	11

Table 2. The effects of various suspended solids concentrations on crustaceans

Table 2. The effects	Table 2. The effects of various suspended solids concentrations on crustaceans							
SPECIES	LIFE STAGE	CONC.	EXPOSURE	MATERIAL	EFFECT	SOURCE		
Eurytemora affinis	adult	500 mg/1	NS	SiO <sub>2</sub> ( 15 µm)	ave. 49.5% reduction in algal uptake	Sherk, et al., 1976		
11 11	11	500 mg/1	"	Fuller's earth	42% reduction in	"		
11 11		500 mg/1	11	natural sediment	algal uptake ave. 62.6% reduction in algal uptake	11		
Acartia tonsa	adult	100 mg/1	NS	SiO <sub>2</sub> ( 15 µm)	ave. 66.6% reduction in algal uptake	11		
11 11	"	100 mg/1	11	Fuller's earth	ave. 67.5% reduction in algal uptake	11		
11 11	11	500 mg/1	11	nat. sediment	72.9% reduction in algal uptake	11		
Crangon nigromaculata	adu1t	50,000 mg/1	200 hrs.	Kaolin	LC <sub>50</sub>	Peddicord et al., 1975		
Homarus americanus	adult	50,000 mg/1	NS	Kaolin	no mortality	Saila et al., 1968		
	"	1,600 ppm	NS	harbor sed.	no martality	cited from Stern and Strickle, 1978		
Palaemon macrodactylus	adult	77,000 mg/1	200 hrs.	Kaolin	LC <sub>20</sub>	Peddicord et al., 1975		
Cancer magister	adult	3,500 mg/1	21 days	contaminated sediment	LC <sub>10</sub>	Peddicord and McFarland, 1978		
Crangon nigricauda	4-6 cm	21,500 mg/1	21 days	11	20% mortality	251		

Table 3. The effects of various suspended solids concentrations on mollusks

	LIFE			erions on morrant		
SPECIES	STAGE	CONC.	EXPOSURE	MATERIAL	EFFECT	SOURCE
C. virginica	adult	4,000- 32,000 mg/l	extended	sediment	detrimental	Wilson, 1950
"	11	100-700 ppm	NS	mud	no apparent problems	Mackin, 1961
II .	11	100-4,000 mg/1	NS	silt	57-94% reduction in pumping	Loosanoff & Tommers,
Mytilus edulis	2.5 cm	100,000 mg/1	5 days	kaolin	10% mortality	Peddicord et al., 1975
11 11	10 cm	100,000 mg/1	11 days	kaolin	10% mortality	Peddicord et al., 1975
11 11	10 cm	96,000 mg/1	200 hrs.	kaolin	LC <sub>50</sub>	Peddicord et al., 1975
Crepidula fornicata	adult	200-600 mg/1	NS	NS	pronounced reduction infiltration rate	Johnson, 1971
Mytilus edulis	<b>2</b> 0 <b>-</b> 2.5	2,300 mg/1	21 days	contaminated sediment	LC <sub>10</sub>	Peddicord and McFarland,

Table 4. The effects of various suspended solids concentrations on the eggs and larvae of mollusks

Table 4. III		LIFE				s and larvae or morrusks	
SPECIES		STAGE	CONC.	EXPOSURE	MATERIAL	EFFECT	SOURCE
<u>Crassostrea</u>	virginica	egg	188 mg/1	ns	silt	22% reduction in number developing to straight hinge larval	Davis & Hidu, 1969
11	11	11	250 mg/1	ns	silt	stage 27% " "	11
11		"	375 mg/1	NS	s <b>i</b> lt	34% " "	11
*1	11	11	<1000 mg/1	NS	Fuller's earth	no significant re- duction in number	11
	11	11	<2000 mg/1	NS	Kaolin	developing to straight hinge larvae	
Mercenaria m	ercenaria	egg	750 mg/1	NS	silt	8% reduction in number developing to straight hinge larvae	Davis, 1960
11	**	"	1000 mg/1	NS	silt	21% " "	Davis, 1960
11	**	"	1500 mg/1	NS	silt	35% '' ''	
11	**	11	125 mg/1	NS	Kaolin	18% " "	Davis, 1960
11	11	11	11	NS	Fuller's earth	25% " "	
11	11	"	4000 mg/1	NS	SiO <sub>2</sub> <5 u	31% " "	Davis & Hidu, 1969
C. virginica		larvae	>750 mg/1	12 days	silt	significant reduction in survival	Davis & Hidu, 1969
H H		"	2000 mg/1	11	Fuller's earth	20% reduction in survival	Davis & Hidu, 19693
11 11		"	500 mg/1	NS	Si0 <sub>2</sub> <5 u	78% reduction in survival	11 11

Table 4. continued

SPECIES	LIFE STAGE	CONC.	EXPOSURE	MATERIAL	EFFECT	SOURCE
Mercenaria mercenaria	larvae	1000 mg/1 500 mg/1	NS 12 days	silt Kaolin	normal growth 50% reduction in	Davis, 1960
	<u> </u>				survival	

Table 5. Lethal effects of various suspended solids concentration on fishes

Table 5. Lethal effect		us suspended	solids concent	ration on fishes		
SPECIES	LIFE STAGE	CONC.	EXPOSURE	MATERIAL	EFFECT	SOURCE
Leiostomus xanthurus	adult	13,090 mg/1	,24 hrs.	Fuller's earth	LC <sub>10</sub>	Sherk et al., 1975
11 11	11	68,750 mg/1	11	Patuxent silt	<sup>LC</sup> 10	11
Morone americana	adult	9,970 mg/1	11	Patuxent silt	rc <sub>10</sub>	O'Conner et al., 1976
11 11	11	3,050 mg/1	11	Fuller's earth	LC <sub>10</sub>	Sherk, et al., 1975
Fundulus majalis	11	23,770 mg/1	11	Fuller's earth	LC <sub>10</sub>	11
11 11	11	97,200 mg/1	11	Patuxent silt	LC <sub>10</sub>	11
F. heteroclitus	11	24,470 mg/1	11	Fuller's earth	LC <sub>10</sub>	11
Menidia menidia	11	580 mg/1	"	Fuller's earth	LC <sub>10</sub>	11
Brevoortia tyrannus	juvenile	1,540 mg/1	11	Fuller's earth	LC <sub>10</sub>	11
Anchoa mitchilli	adult	2,310 mg/1	11	Fuller's earth	LC <sub>10</sub>	11
Morone saxatilis	5-6 cm	4,000 mg/1	21 days	uncontaminated sediment	LC <sub>10</sub>	Peddicord and McFarland, No. 1978
11 11	11	400 mg/1	2 days	contaminated sediment	LC <sub>50</sub>	11

Table 6. The sublethal effects of various suspended solids concentrations on fishes.

Table 6. The subleth		ts of various su	spended solids	concentrations	on fishes.	
SPECIES	LIFE STAGE	CONC.	EXPOSURE	MATERIAL	EFFECT	SOURCE
Morone americana	adult	650 mg/1	5 days	Fuller's earth	increased micro- matocrit, hemoglobin concentration & Red Blood cell count over control	0'Connor et al. 1977
M. americana	11	2000 mg/1	6 days	Natural sediment	significant increase in RBC, hematocrit & hemoglobin	11
	11	11	14 days	11	control & experimental similar	11
Trinectes maculatus	adult	1240 mg/1	5 days	Fuller's earth	increased hematocrit & RBC count; reduction in liver glycogen content	11
Fundulus majalis	adult	960 mg/1	5 days	Fuller's earth	increased hematocrit	"
F. heteroclitus	adult	1600 mg/1	4 days	11	11 11	"
Leiostomus xanthurus	adult	1270 mg/1	5 days	Fuller's earth	no significant difference in blood chemistry over control	"1
L. xanthurus	11	16,960 mg/1	7 days	Natural sediment		11
Opsanus tau	adult	14,600 mg/1	3 days	Natural sediment	11	256
Morone saxatilis	adult	1500 mg/1	14 days	Fuller's earth	increased hematocrit	"
M. saxatilis	11	1500-6000 mg/1	6 days	Natural mud	no significant change in blood chemistry over control	11

Table 7. The effects of various suspended solids concentrations of fish eggs and larvae.

Table 7. The effects	LIFE		†		1	1
SPECIES	STAGE	CONC.	EXPOSURE	MATERIAL	EFFECT	SOURCE
Perca falvescens	eggs	500 mg/1	NS	natural fine grained sediment	No statistically significant effect on hatching success,	Schubel & Wang, 1973
Morone americana	tt.	"	"	**	although a several hour delay in hatch-	11
Morone saxatilis	11	"	"	"	ing was frequently observed about 100	"
Alosa pseudoharengus	11	"	II .	"	mg/1	11
M. americana	larvae	2679 mg/1	48 hr.	NS	LC <sub>50</sub>	Morgan et al., 1973 cited from Sternand Stickle, 1978
M. saxatilis	larvae	3411 mg/1	"		LC <sub>50</sub>	"
M. americana	larvae	3730 mg/1				
	eggs	4000 mg/1	NS	NS	Delayed hatching one day	11

## **BIBLIOGRAPHY**

- Allen, K. O. and J. W. Hardy. 1980. Impacts of navigational dredging on fish and wildlife: a literature review. U. S. Fish and Wildlife Service, Biological Services Program. FWS/OBS-80/07. 81 pp.
- Barnard, W. D. 1978. Prediction and control of dredged material dispersion around dredging and open-water pipeline disposal operations. Technical Report DS-78-13, U.S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss. 102 pp.
- Boon, J. D. and R. J. Byrne. 1975. Hampton Bar dredging project, Newport News Shipbuilding and Dry Dock Co. borrow site, monitor of turbidity and sedimentation. Final Report, Va. Inst. Mar. Sci., Gloucester Point, Va. 46 pp.
- Boon, J. D. and G. R. Thomas. 1975. Report on environmental effects of the second Hampton Roads Bridge-Tunnel construction: sediment distributions and bottom characteristics. Contract Report, Va. Inst. of Mar. Sci. Gloucester Point, Va. 51 pp.
- Bouma, A. H. 1976. Shell dredging and its influence on Gulf coast environments. Gulf Publishing Company, Houston, Texas. 454 pp.
- Brown, C. L. and R. Clark. 1968. Observations on dredging and dissolved oxygen in a tidal waterway. Water Resources Res. 4(6):1381-1384.
- Burks, S. A. and R. M. Engler. 1978. Water quality impacts of dredged material disposal (laboratory investigations). Technical Report DS-78-4, U.S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss. 35 pp.
- Davis, H. C. 1960. Effects of turbidity-producing materials in sea water on eggs and larvae of the clam (Venus (Mercenaria) mercenaria). Biol. Bull. 118(1):48-54.
- Davis, H. C. and H. Hidu. 1969. Effects of turbidity-producing substances in sea water on eggs and larvae of three genera of bivalves mollusks. Veliger 11(4):316-323.
- Chesapeake Biological Laboratory. 1970. Gross physical and biological effects of overboard spoil disposal in upper Chesapeake Bay. Nat. Res. Inst. Spec. Rep. No. 3, Univ. of Maryland, Solomons. 66 pp.
- Flemer, D. A. 1970. "Phytoplankton", gross physical and biological effects of overboard spoil disposal in upper Chesapeake Bay, Special Report No. 3, Natural Resources Institute, Univ. Maryland, College Park, Md.
- Hellier, T. R. and L. S. Kornicker. 1962. Sedimentation from a hydraulic dredge in a bay. Publ. Inst. Mar. Sci., Univ. Texas 8:212-215.
- Hirsch, N. D., L. H. Disalvo and R. K. Peddicord. 1978. Effects of dredging and disposal on aquatic organisms. Technical Report DS-78-5, U.S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss. 41 pp.

- Ingle, R. M. 1952. Studies on the effect of dredging operations upon fish and shellfish, Tech. Ser. No. 5. State of Florida, Board of Conservation, Tallahassee, Fla. 26 pp.
- Johnson, J. K. 1971. Effect of turbidity on the rate of filtration and growth of the slipper limpet, <u>Crepidula fornicata</u> Lamarck, 1799. The Veliger 14(3):315-320.
- Kaplan, E. H., J. R. Welker and M. G. Kraus. 1974. Some effects of dredging on populations of macrobenthic organisms. Fishery Bulletin 72(2):455-480.
- Loosanoff, V. 1961. Effects of turbidity on some larval and adult bivalves. Proc. Gulf Carrib. Fish. Inst. 14:80-95.
- Loosanoff, V. L. and F. D. Tommers. 1948. Effect of suspended silt and other substances on rate of feeding of oysters. Science 107(2768): 69-70.
- Mackin, J. G. 1961. Canal dredging and silting in Louisiana bays. Publ. Inst. Mar. Sci., Univ. Texas. 7:262-314.
- May, E. B. 1973. Environmental effects of hydraulic dredging in estuaries. Alabama Marine Resources Bulletin 9:1-85.
- Morgan, R. P., V. J. Rasin and L. A. Noe. 1973. Effects of suspended sediments on the development of eggs and larvae of striped bass and white perch. Ref. No. 73-110. Univ. Md. Nat. Res. Inst., College Park, Md.
- Morton, J. W. 1977. Ecological effects of dredging and dredge spoil disposal: a literature review. U. S. Fish and Wildlife Service Technical Paper No. 94. 33 pp.
- O'Connor, J. M., D. A. Neumann, and J. A. Sherk. 1976. Lethal effects of suspended sediment on estuarine fish. Technical Paper No. 76-20, U.S. Army Engineer Coastal Engineering Research Center, Fort Belvoir, Va. 38 pp.
- Odum, H. T. and R. F. Wilson. 1962. Further studies on reaeration and metabolism of Texas bays, 1958-1960. Publ. Inst. Mar. Sci., Univ. Texas. 8:23-55.
- Peddicord, R. K., V. A. McFarland, D. P. Belfiori, and T. E. Byrd. 1975. Effects of suspended solids on San Francisco Bay organisms. U. S. Army Engineer District, San Francisco, Dredge Disposal Study. Appendix G-Physical Impact. 158 pp.
- Peddicord, R. K. and V. A. McFarland. 1978. Effects of suspended dredged material on aquatic animals. Technical Report D-78-29, U.S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss. 102 pp.

- Saila, S. B. 1980. Estuarine fishery resources and physical estuarine modifications: some suggestions for impact assessment. In: P. Hamilton and K. B. MacDonald (eds.), Estuarine and Wetland Processes. Plenum Publishing Corp., New York, N.Y.
- Saila, S. B., T. T. Polgar and B. A. Rogers. 1968. Results of studies related to dredged sediment dumping on Rhode Island Sound. Proc. Ann. Northeastern Reg. Anti-pollution Conf. 22-24 July 1968. pp. 71-80. Cited from: Saila, S. B., S. D. Pratt, and T. T. Polgar. 1972. Dredge spoil disposal in Rhode Island Sound. Univ. Rhode Island, Mar. Tech. Rep. 2. 48 pp.
- Schubel, J. R. and J. C. S. Wang. 1973. The effects of suspended sediment on the hatching success of Perca flavescens (yellow perch), Morone americana (white perch), Morone saxatilis (striped bass), and Alosa pseudoharengus (alewife) eggs. Special Report No. 30. Chesapeake Bay Institute, Johns Hopkins Univ. Baltimore, Md.
- Sherk, J. A., J. M. O'Connon and D. A. Neumann. 1975. Effects of suspended and deposited sediments on estuarine environments. In: L. E. Cronin (ed.) Estuarine Research Vol. II. ERF. Academic Press, Inc., N.Y. 587 pp.
- Sherk, J. A., J. M. O'Connor and D. A. Newmann. 1976. Effects of suspended solids on selected estuarine plankton, Misc. Report No. 76-1. U. S. Army Coastal Engineering Research Center, CE, Fort Belvoir, Va.
- Stern, E. M. and W. B. Stickle. 1978. Effects of turbidity and suspended material in aquatic environments: literature review. Technical Report D-78-21, U.S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss. 117 pp.
- Subba Rao, D. U. 1973. Effects of environmental perturbations on short-term phytoplankton production off Lawson's Bay, a tropical coastal embayment. Hydrobiologia 43:77-91.
- Sullivan, B. K. and D. R. Hancock. 1977. Zooplankton and dredging: research perspectives from a critical review. Water Resources Bulletin 13(3): 461-468.
- U. S. Army Engineer District, San Francisco. 1975. Dredge disposal study San Francisco Bay and estuary Appendix G Physical impact study. 158 pp. Effects of suspended solids on San Francisco Bay organisms. R. K. Peddicord, V. A. McFarland, D. P. Belfiori, and T. E. Byrd. Univ. of Calif. Bodega Marine Lab. Bodega Bay, Calif. 94923.
- U. S. Army Engineer District, San Francisco, CE, 1975. Dredge Disposal Study, San Francisco Bay and Estuary. Appendix M-Dredging Technology.
- U. S. Army Engineer District, San Francisco, CE, 1976. Dredge Disposal Study, San Francisco Bay and Estuary, Appendix C-Water Column. 98 pp.
- Williamson, K. J. and P. O. Nelson. 1977. Environment and Quality. In:
  Dredging in Estuaries, Technical Manual. W. L. Schroeder, Principal
  Investigator Oregon State University, Corvallis, Oregon 97331 313 pp.

- Wilson, W. B. 1950. The effects of sedimentation due to dredging operations on oysters in Copano Bay, Texas. M. S. Thesis, Agricultural and Mechanical College of Texas. 128 pp.
- Wright, T. D. 1978. Aquatic dredged material disposal impacts. Technical Report DS-78-1, U.S. Army Engineer Waterways Experiment Station, CE, Vicksburg, Miss. 57 pp.

## Summary and Conclusions

The aim of this report is to address the effects of dredging impacts on the Hampton Roads estuarine system. Its scope is limited by certain qualifications which were established at the beginning of the study. These qualifications must be considered before application of the conclusions and recommendations of this report can be deemed valid or appropriate for the dredging operation in question.

The results of this report apply only to channel maintenance dredging where accumulated silt and clay are excavated from the bottom of an existing well-defined channel. Both hydraulic cutterhead and clamshell bucket methods of dredging are considered.

The application of the results is primarily restricted to the principal study area which is Hampton Roads, the Elizabeth River and the Lower James River. Limited application of certain aspects of this study may be made to other areas by interpretation and extrapolation where very similar conditions exist.

Dredging operations utilizing a confined upland disposal area are the only types considered. The dredge cutterhead and clamshell bucket are the only point sources of suspended solids considered in this report. Any impacts associated with disposal operations, open-water or otherwise, cannot be interpreted using the conclusions of this report.

The study area is heavily utilized by marine resources despite its high degree of urbanization, industrialization and commercial shipping use. The Hampton Roads area supports large populations of hard clams. The Lower James River supports vitally important extensive seed oyster beds. The entire area is heavily utilized by a variety of finfish for spawning, nursery areas and/or feeding grounds.

The results of the field investigations and model predictions of the levels and distribution of suspended material and sedimentation indicate that:

- a. Both hydraulic and clamshell dredges generated suspended solids levels in excess of 200 mg/l.
- b. Dispersion and settling reduced the suspended solids generated by the dredges to background levels within approximately 300 meters down current to the dredge.
- c. Sedimentation rates predicted by the model decreased with increasing distance from the dredge. They ranged up to several millimeters
  125 m laterally from the dredge, at right angles to the current flow and at the same depth as the dredging.

In light of the impact threshold for marine resources utilizing the area and the suspended solids and sedimentation levels for dredging given in the literature and those observed and predicted by the model in this study, the following observations on the effects of dredging on these organisms and water quality are offered:

- a. Minor impact on phytoplankton photosynthesis due to reduced light penetration which is often offset by increased nutrient availability.
- b. Limited interference with zooplankton feeding immediately adjacent to the dredge due to increased suspended solids.
- c. Reduction in the development of oyster eggs into larvae due to increased suspended solids in excess of 200 mg/l.
- d. Pronounced reduction in the pumping rate of oysters when levels exceed 100 mg/1.
- e. Increase in sediment accumulation in areas adjacent to the dredged area. This sedimentation may be significant enough within a few hundred meters to have an adverse effect on oysters, particularly spatfall and spat survival.

- f. Lethal impacts on fishes should be minimal except for juvenile striped bass and atlantic silverside which are susceptible to levels of suspended solids on the order of 500 mg/l. White perch appear to undergo respiratory stress at approximately the same level.
- g. The eggs of several species of fish can experience a slight delay in hatching (a few hours) during exposure to suspended solids levels in excess of 100 mg/1.
- h. Generally, the releases of nutrients, heavy metals and pesticides should be small in quantity and of short duration.
- i. In some instances, there will be possibility of a reduction in Dissolved Oxygen by 1-2 mg/l near the dredge. This depends on numerous factors including the sediments being dredged, water temperature, and the dispersion capacity of the water body.

Based on the above information, the potential exists for dredging operations in close proximity to productive oyster beds and certain fish spawning areas at certain times of the year to have an appreciable impact on these resources. Other resources will be impacted but the extent and duration should be minimal.

In developing a management plan for dredging for Hampton Roads and the lower James River, it might be advisable to designate and classify areas of particular concern. The designation and classification of these important resource areas with respect to their potential for being affected by dredging at different times of the year could prove to be an effective tool for managing dredging in the Hampton Roads and lower James River area.

A suggested scheme for designated areas of particular concern would include the following classifications which could be applied during the appropriate times of year:

Restricted - The potential exists for serious adverse impacts on adjacent resources. Dredging and disposal operations should be prohibited, except, possibly, for emergency situations during the most vulnerable times of the year to protect the resources.

Conditional - Potential exists for adverse impacts on adjacent resources during certain times of the year. But due to the level of anticipated dredging and/or disposal impact, the proximity of the resources, or the marginal value of the areas to the resources, there are no absolutely critical times of the year when dredging should be prohibited. However, there may be times of the year when dredging and disposal operations should be avoided, when possible, to minimize unnecessary adverse impacts.

Open - Areas where the resources present are not especially susceptible to the adverse effects of dredging and/or disposal operations and time-of-year dredging restrictions are generally not warranted. This, however, does not preclude restrictions for exceptional situations which must be evaluated on a case-by-case basis.

The application of this classification system for designated areas to the Hampton Roads-lower James River area included in the present study would involve the following:

- 1. The designation of the area between Deep Water Shoals and a line from Newport News Point to Pig Point in the lower James River as a restricted area for dredging during the oyster spawning and setting season (July, August and September). Dredging within 500 meters of any other productive oyster bottom in the Hampton Roads study area during these months should also receive a restricted classification.
- 2. A conditional classification for the Southern Branch of the Elizabeth and its tributaries upstream of the I-64 bridge during

- the principal anadromous and resident fish spawning season (mid-March through June). This area is also heavily utilized as a nursery for postlarvae and juveniles of numerous fishes.
- 3. A conditional classification for dredging in the Southern Branch of the Elizabeth River during the warm weather months (July through September) might also be considered to help minimize the potential for creating dissolved oxygen depletion by adding the effects of dredging to already oxygen stressed conditions. However, this would be contingent upon the development of a sufficient body of data to indicate whether dredging contributes significantly to the reduction of dissolved oxygen levels.

